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¹ Acknowledgment is made to J. A. Newlin of the Forest Products Laboratory, who was instrumental in conceiving the study and planning the tests on which this bulletin is based, and under whose supervision the work has been carried out.

² Maintained by the U. S. Department of Agriculture at Madison, Wis., in cooperation with the University of Wisconsin.

PURPOSE AND RELATION TO OTHER PUBLICATIONS

A knowledge of the properties of any material is essential to its proper use. In recognition of this fact the Forest Products Laboratory in 1910 began a comprehensive series of tests to determine the mechanical, and some of the related physical properties of native woods. Several hundred thousand tests have been made yielding data in varying quantity on 164 species. This bulletin presents data from this study, together with related information on factors that affect strength properties.

The tests reported here were made on clear wood, free from defects that affect the strength. Inasmuch as the strength of wooden members in structural and industrial use is affected by numerous variables, such as species of wood, variation in quality of the clear wood and in defects among pieces of the same species, character and distribution of load and duration of stress, temperature and moisture conditions, and size and shape of the piece, it may be asked, "why make tests on clear wood?"

Information for application to such uses may obviously be obtained by testing actual structural members or finished manufactured articles under such conditions as obtain in service and with defects as found in such pieces. Some earlier investigations by the Forest Service included tests of this character. However, the results of such tests accurately represent only the combination of variables existing in each instance, are difficult to interpret with respect to the separate effects of each variable, and cannot be applied to instances in which a different combination exists. Furthermore, the combinations are so numerous that it is impossible to evaluate them all by such tests, consequently, the limited usefulness of the data was soon evident. The plan that has been largely followed by the Forest Service has been to obtain data that are more generally applicable by testing small clear specimens taken from a specific part of the tree and of a standard size and form according to standardized methods and supplementing the resulting basic data on each species by investigations in which the effects of the more important variables are as far as possible separately studied and evaluated. The supplementary investigations have related to the effects on strength induced by such variables as locality of growth, position in tree, rate of growth, knots, cross grain, pitch pockets, moisture content, size and shape of piece, duration of stress, preservative treatment, and kiln drying. These and other supplementary investigations are the basis for the discussion of factors affecting the strength of wood as presented in pages 31 to 74.

Some of the results of the tests on small clear specimens were combined into simplified comparative figures and published in 1930 in United States Department of Agriculture Technical Bulletin 158 (28).³ Because of their popularized form, data in Technical Bulletin 158 are not suitable for such engineering uses as calculating the strength or size of members, but are usable mainly for comparing species.

The information given here, on the other hand, is more technical, and may be used not only (1) for comparing species but also (2) for calculating the strength of wood members, (3) for establishing safe working stresses when used in conjunction with other information including results of tests of structural timbers, and (4) for grouping

³ Italic numbers in parentheses refer to Literature Cited, p. 74.

species into classes of approximately like properties for various purposes. The present bulletin is based on the same series of tests, but supersedes United States Department of Agriculture Bulletin 556 (37), because it covers additional species and additional tests on species previously reported. Another important difference is that the values for air-dry wood as given herein have been adjusted uniformly to a 12-percent moisture content, thus making them directly comparable as presented. In addition to the data from the standard series of tests begun in 1910 there is included herein results of all earlier tests by the Forest Service that were made in such a manner as to afford data of comparable character to that resulting from the standard series.

MEANING AND IMPORTANCE OF STRENGTH

In a broad sense "strength" implies all those properties that fit a material to resist forces. In a more restricted sense, strength is resistance to stress of a single kind, or to the stresses developed in a particular member. Definiteness requires that the name of the specific property be stated; as for instance, strength in shear, strength in compression parallel to grain, or strength as a short column. If the several strength properties had the same relation to each other in all species, a wood that excels in one property would, of course, be higher in all, and misinterpretation of "strength" would be less likely. Actually, however, a species may rank higher in one strength property than in another. Longleaf pine averages higher than white oak in maximum crushing strength parallel to the grain, but lower in hardness. Hence, it cannot be said that longleaf pine is "stronger" or "weaker" than white oak without specifying the kind of strength. In comparing species for a particular use the kind of strength properties or combination of properties essential to that use must be considered. Thus, from the comparisons just cited, longleaf pine is superior to oak for use as short posts carrying heavy endwise loads, whereas oak excels in resistance to wear and marring.

In most uses the serviceability of wood depends on one or more strength properties. Airplane-wing beams, floor joists, and wheel spokes typify uses in which strength is a major consideration. Other uses often require strength in combination with other characteristics. Telephone poles, railroad ties, and bridge stringers must not only carry loads, but must also resist decay. In addition, many uses not ordinarily associated with strength depend to some degree on strength properties. For example, finish and trim for buildings should be sufficiently hard to avoid marring; window sash must have screw-holding ability to permit secure attachment of hardware, and adequate stiffness to prevent springing when the window is opened and closed. Even matches must have strength to avoid breaking. Information on strength properties is therefore important not only in the design of airplanes, buildings, and bridges, but also as a guide to the selection of wood for a great variety of uses.

The data reported here refer to some of the properties that are important in many uses. Obviously, any such series of mechanical tests does not answer all questions concerning suitability for a given use because the use may involve strength properties that have not been evaluated and because characteristics other than strength (p. 26) are usually also important.

TESTING PROCEDURE

The material for test was identified botanically in the woods and was brought to the Forest Products Laboratory at Madison, Wis., in the green condition in log form. The procedure for selection and care of material, method of preparing test specimens, and method of testing are the result of many years of development in studying wood properties in the United States and embody some features of European practice. Methods of Testing Small Clear Specimens of Timber adopted as standard by the American Society for Testing Materials (4), and the American Standards Association is essentially the same as the procedure used. A generally similar procedure is also being followed in a number of other countries. Detailed description of the procedure used, and of the methods of computing the results are presented in the appendix, p. 78.

SCOPE OF TESTS

Many individual pieces of each species were tested in determining the average values of strength properties as presented in table 1. In all over 250,000 tests have been made. Only the average results for each species are, however, presented here. It is difficult to determine how many tests should be made on each species. The larger the number, the nearer may the average values be expected to approach the true average of the species, but also the greater is the cost. A balance must be reached between these desiderata, so that a species usually has been represented by only five trees from any one site or locality. Two or more five-tree units, however, from different localities have been tested for the more important species. The individual tests on a species vary in number from about a hundred to several thousand.

CONSIDERATIONS CONCERNING USE OF TABLE 1

The values given in table 1 are the best available valuations of the true averages. Those for the less important species, being based on fewer tests, are less reliable than those for the common species. In applying the data, too great emphasis should not be placed on small differences in averages. The importance of such differences depends largely on the use to which the wood is put. A discussion of variability and the significance of differences between averages is presented on page 17.

The results obtained in tests of clear wood depend not only on the inherent characteristics of the wood but also on such extrinsic factors as the size and form of specimens, the rate of loading, and other features of testing procedure, and in seasoned material on the moisture content. Care should accordingly be used in comparing the data with that from tests in which a different procedure may have been used and the moisture content of test material should be taken into consideration.

The values in table 1 are primarily for the comparison of species in the form of clear lumber. For comparing structural timbers in which the defects are limited with reference to their effect on strength, allowable working stresses are preferable (29, 61).

TABLE 1.—Strength and related properties of woods grown in the United States

Species (common and botanical names)	Place of growth of material tested	Moisture condition	Trees tested	Rings per inch	Summer wood	Moisture content	Specific gravity, oven dry, based on volume—		Weight per cubic foot	Shrinkage from green to oven-dry condition based on dimensions when green			Static bending						Impact bending			Compression parallel to grain		Compression perpendicular to grain; stress at proportional limit	Hardness; load required to embed a 0.444-inch ball to ½ its diameter		Shear parallel to grain; maximum shearing strength	Cleavage; load to cause splitting	Tension perpendicular to grain; maximum tensile strength
							At test	When oven dry		Volumetric	Radial	Tangential	Stress at proportional limit	Modulus of rupture	Modulus of elasticity	Work			Stress at proportional limit	Work to proportional limit	Height of drop causing complete failure (50-pound hammer)	Stress at proportional limit	Maximum crushing strength		End	Side			
																Proportional limit	Maximum load	Total											
1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30
HARDWOODS																													
Alder, red (<i>Alnus rubra</i>)	Washington	(Green— Dry—)	Number 6	Number 11	Percent —	Percent 98	0.37 .41	0.43	Pounds 46	Percent 12.6	Percent 4.4	Percent 7.3	Lb. per sq. in. 3,800	Lb. per sq. in. 6,500	1,000 lb. per sq. in. 1,170	In.-lb. per cu. in. 0.70	In.-lb. per cu. in. 8.0	In.-lb. per cu. in. 15.3	Lb. per sq. in. 8,000	In.-lb. per cu. in. 2.6	Inches 22	Lb. per sq. in. 2,620	Lb. per sq. in. 2,960	Lb. per sq. in. 310	Pounds 550	Pounds 440	Lb. per sq. in. 770	Lb. per in. of width 220	Lb. per sq. in. 390
Apple (<i>Malus pumila</i> var.)	Virginia	(Green— Dry—)	10	6	—	46	.61 .67	.74	28	17.6	5.6	10.1	6,900	9,800	1,380	1.85	8.4	10.7	11,600	4.8	30	4,530	5,820	540	980	590	1,080	270	420
Ash, Biltmore white (<i>Frazinus biltmoreana</i>)	Tennessee	(Green— Dry—)	5	17	49	42	.51 .55	.68	45	12.6	4.2	6.9	5,500	9,300	1,340	1.31	11.6	27.4	15,900	4.9	40	3,530	3,980	380	950	850	1,230	340	540
Ash, black (<i>Frazinus nigra</i>)	Michigan, Wisconsin	(Green— Dry—)	16	24	—	85	.45 .49	.53	52	15.2	5.0	7.8	2,600	6,000	1,040	.41	12.1	31.7	16,500	7.9	33	3,690	4,200	430	590	520	860	280	490
Ash, blue (<i>Frazinus quadrangulata</i>)	Kentucky	(Green— Dry—)	5	12	49	39	.53 .57	.60	46	11.7	3.9	6.5	5,700	9,600	1,240	1.47	14.7	38.2	11,100	5.0	43	3,580	4,180	990	1,140	1,030	1,540	350	580
Ash, green (<i>Frazinus pennsylvanica lanceolata</i>)	Louisiana, Missouri	(Green— Dry—)	10	17	58	48	.53 .56	.61	49	12.5	4.6	7.1	5,300	9,500	1,400	1.14	11.8	27.6	11,400	5.0	35	3,560	4,200	910	960	870	1,260	350	590
Ash, Oregon (<i>Frazinus oregona</i>)	Oregon	(Green— Dry—)	3	12	63	48	.50 .55	.58	46	13.2	4.1	8.1	4,200	7,600	1,130	.92	12.2	33.3	8,900	3.0	39	2,760	3,510	650	850	790	1,190	310	590
Ash, pumpkin (<i>Frazinus profunda</i>)	Missouri	(Green— Dry—)	3	21	46	51	.48 .52	.55	46	12.0	3.7	6.3	4,500	7,600	1,040	1.08	9.4	18.4	8,800	3.7	31	2,850	3,560	990	880	750	1,210	360	570
Ash, white (<i>Frazinus americana</i>)	Arkansas, New York, West Virginia, Vermont, Massachusetts, Wisconsin, New Mexico	(Green— Dry—)	23	12	54	42	.55 .60	.64	48	13.3	4.9	7.9	5,100	9,600	1,460	1.04	16.6	41.6	13,900	5.9	38	3,190	3,990	810	1,010	960	1,380	330	590
Aspen (<i>Populus tremuloides</i>)	Wisconsin, New Mexico	(Green— Dry—)	11	8	—	94	.35 .38	.40	42	11.5	3.5	6.7	3,200	5,100	860	.69	6.4	13.4	7,000	2.7	22	1,670	2,140	220	280	300	660	140	230
Aspen, largetooth (<i>Populus grandidentata</i>)	Wisconsin, Vermont	(Green— Dry—)	10	8	—	99	.35 .39	.41	43	11.8	3.3	7.9	2,900	5,400	1,120	.44	5.6	12.6	7,400	2.5	18	2,020	2,500	250	400	370	730	190	310
Basswood (<i>Tilia glabra</i>)	Wisconsin, Pennsylvania	(Green— Dry—)	8	19	—	105	.32 .37	.40	42	15.8	6.6	9.3	2,700	5,000	1,040	.40	5.3	10.3	6,300	2.1	16	1,690	2,220	210	290	250	600	150	280
Beech (<i>Fagus grandifolia</i>)	Indiana, Pennsylvania, Vermont	(Green— Dry—)	17	15	—	54	.56 .64	.67	54	16.3	5.1	11.0	4,300	8,600	1,380	.85	11.9	30.8	11,500	4.4	43	2,550	3,550	670	970	850	1,290	410	720
Beech, blue (<i>Carpinus caroliniana</i>)	Massachusetts	(Green— Dry—)	12	15	—	48	.58 .70	.72	53	19.1	5.7	11.4	3,200	6,800	990	.61	19.1	60.6	10,100	4.0	106	1,420	2,670	730	900	940	1,160	260	350
Birch, Alaska white (<i>Betula neoalaskana</i>)	Alaska	(Green— Dry—)	10	29	—	58	.49 .55	.59	48	16.7	6.5	9.9	3,800	7,100	1,350	.60	11.6	32.5	9,800	3.7	37	2,050	3,030	430	550	560	920	180	200
Birch, gray (<i>Betula populifolia</i>)	New Hampshire	(Green— Dry—)	5	—	—	63	.45 .51	.55	46	14.7	5.2	—	1,800	4,900	400	.47	13.9	37.8	7,400	2.6	59	1,080	1,860	250	430	480	800	200	370
Birch, paper (<i>Betula papyrifera</i>)	Wisconsin, New Hampshire	(Green— Dry—)	10	6	—	65	.48 .55	.60	38	16.2	6.3	8.6	3,000	6,400	1,170	.45	16.2	42.8	8,000	2.7	49	1,640	2,360	340	470	560	840	210	380
Birch, sweet (<i>Betula lenta</i>)	Pennsylvania, New Hampshire	(Green— Dry—)	10	27	—	53	.60 .65	.71	57	15.6	6.5	8.5	4,800	9,400	1,650	.94	15.7	41.4	10,500	3.2	48	2,680	3,740	580	1,070	970	1,240	300	430
Birch, yellow (<i>Betula lutea</i>)	Pennsylvania, Vermont, Wisconsin	(Green— Dry—)	17	16	—	67	.55 .62	.66	57	16.7	7.2	9.2	10,100	16,900	2,170	2.72	18.0	22.4	24,800	10.6	47	6,330	8,540	1,340	1,960	1,470	2,240	640	950
Blackwood (<i>Avicennia nitida</i>)	Florida	(Green— Dry—)	6	16	—	42	.83 .88	.96	74	15.6	6.2	9.7	5,500	11,100	1,550	1.16	12.3	39.0	15,900	6.8	42	3,760	4,940	1,870	1,570	1,700	1,370	280	660
Buckeye, yellow (<i>Aesculus octandra</i>)	Tennessee	(Green— Dry—)	5	15	—	141	.33 .36	.38	49	12.0	3.5	7.8	2,600	4,800	980	.41	5.4	10.5	6,500	2.1	18	1,680	2,050	210	360	290	660	180	320
Bustic (<i>Dipholis salicifolia</i>)	Florida	(Green— Dry—)	1	—	—	12	.86 .88	—	77	—	—	—	5,800	12,400	1,860	1.00	17.1	—	—	—	—	—	3,750	5,330	1,700	—	—	—	—
Butternut (<i>Juglans cinerea</i>)	Tennessee, Wisconsin	(Green— Dry—)	10	9	—	104	.36 .38	.40	46	10.2	3.3	6.1	2,900	5,400	970	.52	8.2	21.2	7,300	2.5	24	2,020	2,420	270	410	390	760	220	430
Buttonwood (<i>Conocarpus erecta</i>)	Florida	(Green— Dry—)	7	—	—	47	.69 .71	.85	64	14.6	5.4	8.5	4,600	7,400	1,190	1.00	6.2	15.6	14,100	6.8	40	3,050	4,110	1,140	1,080	1,110	1,220	370	470
Cascara (<i>Rhamnus purshiana</i>)	Oregon	(Green— Dry—)	5	17	—	61	.50 .52	.55	50	7.6	3.2	4.6	3,400	6,300	630	1.04	13.4	49.7	8,700	3.6	58	1,890	3,270	670	680	730	1,150	260	510
Catalpa, hardy (<i>Catalpa speciosa</i>)	Indiana	(Green— Dry—)	15	8	58	72	.38 .41	.42	41	7.3	2.5	4.9	2,700	5,200	840	.51	7.9	25.1	7,500	3.1	35	1,450	2,360	320	420	410	680	220	430
Cherry, black (<i>Prunus serotina</i>)	Pennsylvania	(Green— Dry—)	5	11	—	55	.47 .50	.53	45	11.5	3.7	7.1	4,200	8,000	1,310	.80	12.8	31.8	10,200	4.1	33	2,940	3,540	440	750	660	1,130	330	570
Cherry, pin (<i>Prunus pennsylvanica</i>)	Tennessee	(Green— Dry—)	5	6	—	46	.36 .39	.42	33	12.8	2.8	10.3	2,900	5,000	1,040	.47	6.2	18.3	6,600	2.1	22	1,810	2,170	260	440	390	680	170	300
Chestnut (<i>Castanea dentata</i>)	Maryland, Tennessee	(Green— Dry—)	10	11	48	122	.40 .43	.45	55	11.6	3.4	6.7	3,100	5,600	930	.59	7.0	17.0	7,900	2.8	24	2,080	2,470	380	530	420	800	240	440
Chinquapin, golden (<i>Castanopsis chrysophylla</i>)	Oregon	(Green— Dry—)	5	15	—	134	.42 .46	.48	61	13.2	4.6	7.4	4,200	7,000	1,020	1.09	9.5	20.4	8,800	3.4	31	2,080	3,020	490	730	600	1,010	230	480
Cottonwood, eastern (<i>Populus deltoides</i>)	Missouri	(Green— Dry—)	5	6	—	111	.37 .40	.43	49	14.1	3.9	9.2	2,900	5,300	1,010	.49	7.3	16.9	7,200	2.3	21	1,740	2,280	240	380	340	680	220	410
Cottonwood, northern black (<i>Populus trichocarpa hastata</i>)	Washington	(Green— Dry—)	5	6	—	132	.32 .35	.37	46	12.4	3.6	8.6	5,300	8,300	1,260	1.25	6.7	10.8	9,800	3.8	22	3,270	4,430	370	540	350	1,020	220	330
Dogwood (<i>Cornus florida</i>)	Tennessee	(Green— Dry—)	5	24	—	62	.64 .73	.80	64	19.9	7.1	11.3	4,800	8,800	1,180	1.11	21.0	49.1	7,100	3.5	58	3,640	1,030	1,410	1,410	1,410	1,520	340	740
Dogwood, Pacific (<i>Cornus nuttallii</i>)	Oregon	(Green— Dry—)	5	21	—	52	.68 .64	.70	55	17.2	6.4	9.6	4,200	8,200	1,090	.92	17.0	38.7	9,800	3.6	56	2,410	3,640	870	1,140	980	1,300	340	740
Elder, blueberry (<i>Sambucus coerulea</i>)	Co.	(Green— Dry—)	5	6	—	124	.46 .52	.57	65	15.6	4.4	9.0	3,400	6,600	900	.72	8.8	30.7	8,000	2.9	38	2,380	3,0						

1 The averages for this species include data from tests representing an unknown number of trees in addition to the number indicated.

TABLE 1.—Strength and related properties of woods grown in the United States—Continued

Species (common and botanical names)	Place of growth of material tested	Moisture condition	Trees tested	Rings per inch	Summer wood	Moisture content	Specific gravity, oven dry, based on volume—		Weight per cubic foot	Shrinkage from green to oven-dry condition based on dimensions when green			Static bending						Impact bending			Compression parallel to grain		Compression perpendicular to grain; stress at proportional limit	Hardness; load required to embed a 0.444-inch ball to ½ its diameter		Shear parallel to grain; maximum shearing strength	Cleavage; load to cause splitting	Tension perpendicular to grain; maximum tensile strength
							At test	When oven-dry		Volumetric	Radial	Tangential	Stress at proportional limit	Modulus of rupture	Modulus of elasticity	Work			Stress at proportional limit	Work to proportional limit	Height of drop causing complete failure (50-pound hammer)	Stress at proportional limit	Maximum crushing strength		End	Side			
																Proportional limit	Maximum load	Total											
1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30
HARDWOODS—continued																													
Elm, American (<i>Ulmus americana</i>)	(New Hampshire, Pennsylvania, Wisconsin.	(Green—Dry—	12	13	54	89	0.46	0.55																					
Elm, rock (<i>Ulmus racemosa</i>)	Wisconsin.	(Green—Dry—	15	29	50	48	.57	.66																					
Elm, slippery (<i>Ulmus fulva</i>)	Indiana, Wisconsin.	(Green—Dry—	6	16	54	85	.48	.57																					
Fig, golden (<i>Ficus aurea</i>)	Florida.	(Green—Dry—	1			88	.44																						
Gum, black (<i>Nyssa sylvatica</i>)	Tennessee.	(Green—Dry—	5	27		55	.46	.55																					
Gum, blue (<i>Eucalyptus globulus</i>)	California.	(Green—Dry—	5			79	.62	.80																					
Gum, red (<i>Liquidambar styraciflua</i>)	Missouri.	(Green—Dry—	15	16		81	.44	.53																					
Gum, tupelo (<i>Nyssa aquatica</i>)	Louisiana, Missouri.	(Green—Dry—	6	10		97	.46	.52																					
Gumbo limbo (<i>Bursera simaruba</i>)	Florida.	(Green—Dry—	5			99	.30	.32																					
Hackberry (<i>Celtis occidentalis</i>)	Indiana, Wisconsin.	(Green—Dry—	6	13	56	65	.49	.56																					
Haw, pear (<i>Crataegus tomentosa</i>)	Wisconsin.	(Green—Dry—	2	11		63	.62																						
Hickory, bigleaf shagbark (<i>Hicoria laciniosa</i>)	Ohio, Mississippi.	(Green—Dry—	19	19	65	61	.62																						
Hickory, bitternut (<i>Hicoria cordiformis</i>)	Ohio.	(Green—Dry—	11	11	70	66	.60																						
Hickory, mockernut (<i>Hicoria alba</i>)	(Pennsylvania, Mississippi, West Virginia.	(Green—Dry—	20	18	63	59	.64																						
Hickory, nutmeg (<i>Hicoria myristicaeformis</i>)	Mississippi.	(Green—Dry—	5	22	59	74	.56																						
Hickory, pignut (<i>Hicoria glabra</i>)	(West Virginia, Mississippi, Ohio, Pennsylvania.	(Green—Dry—	60	20	65	54	.66																						
Hickory, shagbark (<i>Hicoria ovata</i>)	(Mississippi, Ohio, West Virginia, Pennsylvania.	(Green—Dry—	24	19	66	60	.64																						
Hickory, water (<i>Hicoria aquatica</i>)	Mississippi.	(Green—Dry—	2	15	67	80	.61																						
Holly (<i>Ilex opaca</i>)	Tennessee.	(Green—Dry—	5	27		82	.50	.61																					
Honeylocust (<i>Gleditsia triacanthos</i>)	Indiana, Missouri.	(Green—Dry—	6	9	45	12	.57	.67																					
Hophornbeam (<i>Ostrya virginiana</i>)	Wisconsin.	(Green—Dry—	5	29		52	.63	.76																					
Inkwood (<i>Erothea paniculata</i>)	Florida.	(Green—Dry—	2			56	.73	.92																					
Ironwood, black (<i>Krugiodendron ferreum</i>)	do.	(Green—Dry—	4			32	1.04	1.08																					
Laurel, California (<i>Umbellularia californica</i>)	Oregon.	(Green—Dry—	5	6		70	.51	.59																					
Laurel, mountain (<i>Kalmia latifolia</i>)	Tennessee.	(Green—Dry—	5	24		62	.62	.74																					
Locust, black (<i>Robinia pseudoacacia</i>)	do.	(Green—Dry—	3	11	51	49	.66	.71																					
Madrono, Pacific (<i>Arbutus menziesii</i>)	California, Oregon.	(Green—Dry—	6	10		68	.58	.69																					
Magnolia, cucumber (<i>Magnolia acuminata</i>)	Tennessee.	(Green—Dry—	5	14		80	.44	.52																					
Magnolia, evergreen (<i>Magnolia grandiflora</i>)	Louisiana.	(Green—Dry—	2	15	105	46	.46	.53																					
Magnolia, mountain (<i>Magnolia fraseri</i>)	Tennessee.	(Green—Dry—	5	15		89	.40	.48																					
Mangrove (<i>Rhizophora mangle</i>)	Florida.	(Green—Dry—	4			39	.89	1.06																					
Maple, bigleaf (<i>Acer macrophyllum</i>)	Washington.	(Green—Dry—	5	12		72	.44	.51																					
Maple, black (<i>Acer nigrum</i>)	Indiana.	(Green—Dry—	1	17		65	.52	.62																					
Maple, red (<i>Acer rubrum</i>)	(New Hampshire, Pennsylvania, Wisconsin.	(Green—Dry—	14	13		63	.49	.55																					
Maple, silver (<i>Acer saccharinum</i>)	Wisconsin.	(Green—Dry—	5	7		66	.44	.51																					

1 The averages for this species include data from tests representing an unknown number of trees in addition to the number indicated.

TABLE 1.—Strength and related properties of woods grown in the United States—Continued

Species (common and botanical names ¹	Place of growth of material tested	Moisture condition	Trees tested	Rings per inch	Summer wood	Moisture content	Specific gravity, oven-dry, based on volume—		Weight per cubic foot	Shrinkage from green to oven-dry condition based on dimensions when green			Static bending						Impact bending			Compression parallel to grain		Compression perpendicular to grain; stress at proportional limit	Hardness; load required to embed a 0.444-inch ball to ½ its diameter		Shear parallel to grain; maximum shearing strength	Cleavage; load to cause splitting	Tension perpendicular to grain; maximum tensile strength
							At test	When oven-dry		Volumetric	Radial	Tangential	Stress at proportional limit	Modulus of rupture	Modulus of elasticity	Work			Stress at proportional limit	Work to proportional limit	Height of drop causing complete failure (50-pound hammer)	Stress at proportional limit	Maximum crushing strength		End	Side			
																Proportional limit	Maximum load	Total											
1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30
HARDWOODS—continued																													
Maple, striped (<i>Acer pennsylvanicum</i>)	Vermont	Green	37			35	0.44		Pounds	Percent	Percent	Percent	Lb. per sq. in.	Lb. per sq. in.	1,000 lb. per sq. in.	In.-lb. per cu. in.	In.-lb. per cu. in.	In.-lb. per cu. in.	Lb. per sq. in.	In.-lb. per cu. in.	Inches	Lb. per sq. in.	Lb. per sq. in.	Lb. per sq. in.	Pounds	Pounds	Lb. per sq. in.	Lb. per in. of width	Lb. per sq. in.
		Dry	32			12	.46		37	12.3	3.2	8.6	3,600	7,200	1,080	0.68	10.9	13.4	8,700	2.3	36	1,790	2,920	500	500	1,150			
Maple, sugar (<i>Acer saccharum</i>)	Indiana, Pennsylvania, Vermont, Wisconsin	Green	17	18		58	.56	0.68	56	14.9	4.9	9.5	5,100	9,400	1,550	1.03	13.3	33.6	12,200	4.8	40	2,850	4,020	800	1,070	970	1,460		
		Dry	44			12	.63		44				9,500	15,800	1,830	2.76	16.5	27.9	20,600	9.3	39	5,390	7,830	1,810	1,840	1,450	2,330		
Mastic (<i>Sideroxylon foetidissimum</i>)	Florida	Green	5			39	.89	1.03	77	11.7	6.1	7.5	7,100	10,400	1,580	1.79	8.1	19.8	18,000	8.7	52	4,950	5,880	2,680	1,670	1,770	1,670	430	1,030
		Dry	65			12	.93		65				6,600	10,200	1,780	1.39	6.2	6.6	14,100	5.1	24	3,940	6,930	2,830	2,090	1,790	1,470	370	710
Oak, black (<i>Quercus velutina</i>)	Arkansas, Wisconsin	Green	8	15	71	80	.56	.67	62	14.2	4.5	9.7	4,600	8,200	1,180	1.02	12.2	30.1	11,400	4.9	40	2,720	3,470	870	1,000	1,060	1,220		
		Dry	43			12	.61		43				7,900	13,900	1,640	2.15	13.7	24.0	14,400	6.4	41	4,750	6,520	1,150	1,350	1,210	1,910		
Oak, bur (<i>Quercus macrocarpa</i>)	Wisconsin	Green	5	12	59	70	.58	.67	62	12.7	4.4	8.8	3,600	7,200	880	.89	10.7	26.1	10,000	4.7	44	2,380	3,290	840	1,160	1,110	1,350	430	800
		Dry	45			12	.64		45				6,400	10,300	1,030	2.37	9.8	17.4	14,600	8.0	29	3,580	6,060	1,480	1,410	1,370	1,820	320	680
Oak, California black (<i>Quercus kelloggii</i>)	Oregon, California	Green	10	16	52	106	.51	.58	66	12.1	3.6	6.6	3,400	6,200	740	1.03	8.8	16.0	8,200	3.4	30	1,880	2,800	890	1,410	850	1,140	350	700
		Dry	40			12	.57		40				6,100	8,700	990	2.28	6.5	10.0	8,800	4.0	16	3,300	5,640	1,440	1,180	1,100	1,470	360	770
Oak, canyon live (<i>Quercus chrysolepis</i>)	California	Green	3	13		62	.70	.84	71	16.2	5.4	9.5	6,300	10,600	1,340	1.70	14.4	30.9	11,200	3.9	47	3,940	4,690	1,480	1,590	1,570	1,700	520	970
		Dry	54			12	.77		54				9,300	12,900	1,610	3.15	9.9	21.5	13,000	5.5	37	6,110	9,080	2,260	2,530	2,420	2,290	640	
Oak, chestnut (<i>Quercus montana</i>)	Tennessee	Green	5	23	50	72	.57	.67	61	16.7	5.5	9.7	4,600	8,000	1,370	.90	9.4	22.4	12,000	4.6	35	2,890	3,520	660	970	890	1,210	380	690
		Dry	46			12	.66		46				9,000	13,300	1,590	2.88	11.0	19.4	18,600	7.7	40	4,420	6,830	1,040	1,250	1,130	1,490	380	
Oak, laurel (<i>Quercus laurifolia</i>)	Louisiana	Green	5	11	61	84	.56	.70	65	19.0	4.0	9.9	4,500	7,900	1,390	.86	11.2	28.3	10,400	3.4	39	2,650	3,170	710	1,020	1,000	1,180	380	770
		Dry	44			12	.63		44				7,700	12,600	1,690	2.02	11.8	28.5	14,700	5.6	39	4,640	6,980	1,310	1,230	1,210	1,830	360	790
Oak, live (<i>Quercus virginiana</i>)	Florida	Green	5	8		50	.81	.98	76	14.7	6.6	9.5	8,400	11,900	1,580	2.54	12.3	26.0	17,200	8.5	57	4,170	5,430	2,520	1,670	1,880	2,210	550	1,010
		Dry	62			12	.89		62				8,700	18,400	1,980	2.19	18.9	39.1	21,390	11.2	33	5,120	8,900	3,510	3,150	2,680	2,660	520	1,040
Oak, Oregon white (<i>Quercus garryana</i>)	Oregon	Green	10	16	49	72	.64	.75	69	13.4	4.2	9.0	4,600	7,700	790	1.51	13.7	29.8	10,300	4.8	49	2,480	3,570	1,380	1,430	1,390	1,630	450	940
		Dry	50			12	.72		50				6,600	10,300	1,100	2.28	9.8	18.2	11,900	5.4	29	3,960	6,530	2,110	1,880	1,660	2,020	380	830
Oak, pin (<i>Quercus palustris</i>)	Massachusetts	Green	5	9	58	75	.58	.68	63	14.5	4.3	9.5	4,000	8,300	1,320	.71	14.0	35.2	11,900	4.2	48	3,680	4,880	880	1,000	1,070	1,290	470	800
		Dry	44			12	.63		44				8,000	14,000	1,730	2.22	14.8	30.5	12,300	3.6	45	4,620	6,820	1,260	1,600	1,510	2,080	530	1,050
Oak, post (<i>Quercus stellata</i>)	Arkansas, Louisiana	Green	10	26	54	69	.60	.74	63	16.2	5.9	9.8	5,000	8,100	1,090	1.31	11.0	25.4	10,900	4.1	44	2,840	3,480	1,060	1,160	1,130	1,280	410	790
		Dry	47			12	.67		47				7,600	13,200	1,510	2.25	13.2	40.4	17,600	8.6	46	3,700	6,400	1,760	1,350	1,360	1,840	430	750
Oak, red (<i>Quercus borealis</i>)	Arkansas, Indiana, Louisiana, New Hampshire, Tennessee	Green	33	10	63	80	.56	.66	63	13.5	4.0	8.2	4,100	8,300	1,350	.73	13.2	34.5	10,600	3.8	44	2,360	3,440	760	1,060	1,000	1,210	430	750
		Dry	44			12	.63		44				8,500	14,300	1,820	2.33	14.5	33.4	17,600	8.5	43	4,580	6,760	1,250	1,580	1,290	1,780	410	800
Oak, Rocky Mountain white (<i>Quercus utahensis</i>)	Arizona	Green	3	24		61	.62	.70	62	12.5	4.1	7.2	3,200	5,900	480	1.23	11.3	27.2	8,100	4.3	80	1,330	2,940	1,110	1,210	1,280	1,530	370	750
		Dry	51			12	.73		51				5,200	8,500	650	2.30	9.0	13.3	14,100	5.2	23		5,200	2,070	2,030	1,440			
Oak, scarlet (<i>Quercus coccinea</i>)	Massachusetts	Green	5	14	52	65	.60	.71	62	13.8	4.6	9.7	4,500	10,400	1,480	.81	15.0	41.9	11,900	4.0	54	2,840	4,090	1,030	1,170	1,200	1,410	420	700
		Dry	47			12	.67		47				9,700	17,400	1,910	2.92	20.5	43.9	16,100	6.1	53	5,550	8,330	1,380	1,690	1,400	1,890	450	870
Oak, southern red (<i>Quercus rubra</i>)	Louisiana	Green	4	20	46	90	.52	.62	62	16.3	4.5	8.7	4,200	6,900	1,140	.93	8.0	16.5	9,100	3.1	29	2,220	3,030	680	910	860	930	280	480
		Dry	41			12	.59		41				6,000	10,900	1,490	1.44	9.4	15.9	15,300	7.3	26	2,910	6,090	1,080	1,020	1,060	1,390	350	510
Oak, swamp red (<i>Quercus rubra pagodaefolia</i>)	do	Green	3	7	63	78	.61	.71	68	16.4	5.2	10.8	6,500	10,800	1,790	1.32	14.7	38.0	12,300	3.8	54	3,820	4,620	940	1,270	1,240	1,320	460	800
		Dry	47			12	.68		47				11,200	18,100	2,280	3.09	18.3	34.6	23,900	12.0	49	6,350	8,740	1,540	1,570	1,450	2,000	410	840
Oak, swamp chestnut (<i>Quercus prinus</i>)	do	Green	4	12	58	76	.60	.76	65	19.4	5.9	9.2	4,800	8,500	1,350	1.00	12.8	32.2	10,400	3.2	45	3,000	3,540	710	1,100	1,110	1,260	400	670
		Dry	47			12	.67		47				7,300	13,900	1,770	1.68	12.0	21.0	19,000	7.9	41	4,400	7,270	1,370	1,290	1,240	1,990	350	690
Oak, swamp white (<i>Quercus bicolor</i>)	Indiana	Green	1	16	71	74	.64	.79	69	17.7	5.5	10.6	5,400	9,900	1,590	1.05	14.5	34.7	13,300	4.8	50	3,580	4,360	940	1,200	1,160	1,300	480	860
		Dry	50			12	.72		50				10,200	17,700	2,050	2.88	19.2	47.9	22,300	11.2	49	5,830	8,600	1,470	1,680	1,620	2,000	480	830
Oak, water (<i>Quercus nigra</i>)	Louisiana	Green	5	10	61	81	.56	.68	63	16.4	4.2	9.3	5,600	8,900	1,550	1.14	11.1	32.5	11,600	3.8	39	3,260	3,740	770	1,050	1,010	1,240	450	820

¹ The averages for this species include data from tests representing an unknown number of trees in addition to the number indicated.

TABLE 1.—Strength and related properties of woods grown in the United States—Continued

Species (common and botanical names)	Place of growth of material tested	Moisture condition	Trees tested	Rings per inch	Summer wood	Moisture content	Specific gravity, oven dry, based on volume		Weight per cubic foot	Shrinkage from green to oven-dry condition based on dimensions when green			Static bending						Impact bending			Compression parallel to grain		Compression perpendicular to grain; stress at proportional limit	Hardness; load required to embed a 0.444-inch ball to ½ its diameter		Shear parallel to grain; maximum shearing strength	Cleavage; load to cause splitting	Tension perpendicular to grain; maximum tensile strength
							At test	When oven-dry		Volumetric	Radial	Tangential	Stress at proportional limit	Modulus of rupture	Modulus of elasticity	Work			Stress at proportional limit	Work to proportional limit	Height of drop causing complete failure (50-pound hammer)	Stress at proportional limit	Maximum crushing strength		End	Side			
																Proportional limit	Maximum load	Total											
1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30
HARDWOODS—continued																													
Silverbell (<i>Halesia carolina</i>)	Tennessee	(Green	5	20		70	0.42	0.48	44	12.6	3.8	7.6	3,500	6,500	1,160	0.62	8.8	16.1	9,100	3.3	27	2,140	2,830	430	550	470	930	280	460
		(Dry				12	.45		32				5,700	8,600	1,320	1.46	6.9	18.0	13,300	6.0	24	3,580	5,130	680	880	590	1,180	320	480
Sourwood (<i>Oxydendrum arboreum</i>)	do	(Green	5	24		69	.50	.59	53	15.2	6.3	8.9	4,400	7,700	1,320	.82	9.8	20.0	10,800	4.1	38	2,700	3,250	680	860	730	1,160	400	710
		(Dry				12	.55		38				8,300	11,600	1,540	2.44	10.9	21.7	17,200	8.6	34	4,400	6,190	1,080	1,350	940	1,500	380	520
Stopper, red (<i>Eugenia confusa</i>)	Florida	(Green	3			41	.81	.92	72	13.3	6.2	9.1	15,000	1,940			21.6	48.3				54	6,140	2,450			1,820		920
		(Dry				61	.87		61				16,200	2,040			10.6	19.3				34	9,790	2,790		2,600	1,850		
Sugarberry (<i>Celtis laevigata</i>)	Missouri	(Green	5	17	38	62	.47	.54	48	12.7	5.0	7.3	3,200	6,600	810	.78	12.0	30.7	8,200	3.2	33	1,990	2,800	580	840	740	1,050	380	660
		(Dry				12	.51		36				6,200	9,900	1,140	2.18	11.2	26.2	11,600	5.4	36	3,970	5,620	1,240	1,280	960	1,280	380	
Sumach, staghorn (<i>Rhus hirta</i>)	Wisconsin	(Green	5	9	61	45	.45		41				3,000	5,800	810	.67	10.8	42.4				36	2,680	480	670	590			
		(Dry				33	.47		33				7,800	10,200	1,190	2.84	8.4	19.8					5,940	1,010	880	680			
Sycamore (<i>Platanus occidentalis</i>)	Indiana, Tennessee	(Green	10	17		83	.46	.54	52	14.2	5.1	7.6	3,300	6,500	1,060	.60	7.5	15.9	8,800	3.3	26	2,400	2,920	450	700	610	1,000	330	630
		(Dry				34	.49		34				6,400	10,000	1,420	1.66	8.5	14.3	10,500	3.9	26	3,710	5,380	860	920	770	1,470	400	720
Walnut, black (<i>Juglans nigra</i>)	Kentucky	(Green	5	12		81	.51	.56	58	11.3	5.2	7.1	5,400	9,500	1,420	1.16	14.6	35.9	11,900	4.5	37	3,520	4,300	600	960	900	1,220	360	570
		(Dry				38	.55		38				10,500	14,600	1,680	3.70	10.7	17.9	16,400	8.2	34	5,780	7,580	1,250	1,050	1,010	1,370	320	690
Walnut, little (<i>Juglans rupestris</i>)	Arizona	(Green	1			67	.53	.61	55	10.7	4.4	4.6	3,400	8,000	910	.74	12.8	46.4	9,900	4.5	46		3,020	760					
		(Dry				40	.57		40				8,300	14,200	1,480	2.60	11.2	14.3	11,100	4.5	21		6,760						
Willow, black (<i>Salix nigra</i>)	Missouri, Wisconsin	(Green	10	5		139	.34	.41	50	13.8	2.5	7.8	1,800	3,800	560	.36	10.8	19.8	5,100	2.0	36	960	1,520	220	350	360	620	230	430
		(Dry				12	.37		26				3,900	6,200	720	1.94	7.9	11.1	7,700	3.6	20	2,020	3,420	480	550	450	1,050	290	460
Willow, western black (<i>Salix lasiandra</i>)	Oregon	(Green	5	5		105	.39	.47	50	13.8	2.9	9.0	3,100	5,600	1,020	.58	10.8	27.6	7,600	2.5	33	1,810	2,340	330	490	500	870	210	360
		(Dry				12	.44		31				5,500	8,500	1,310	1.37	9.3	23.4	11,000	4.7	31	3,120	4,560	630	850	630	1,160	290	530
Witchhazel (<i>Hamamelis virginiana</i>)	Tennessee	(Green	5	14		70	.56	.71	59	18.8			5,000	8,300	1,110	1.29	19.5	56.8	12,400	6.3	40		3,400	6,200	1,010	980	1,120		
		(Dry				12	.61		43				9,100	15,200	1,460	3.17	21.0						6,740	1,370	1,860	1,530			
SOFTWOODS																													
Cedar, Alaska (<i>Chamaecyparis nootkatensis</i>)	Alaska, Oregon	(Green	8	28		38	.42	.46	36	9.2	2.8	6.0	3,800	6,400	1,140	.77	9.2	26.2	9,100	3.2	27	2,500	3,050	430	540	440	840	170	330
		(Dry				12	.44		31				7,100	11,100	1,420	2.06	10.4	15.8	12,200	5.0	29	5,210	6,310	770	790	580	1,130	180	360
Cedar, incense (<i>Libocedrus decurrens</i>)	Oregon, California	(Green	14	17	30	108	.35	.37	45	7.6	3.3	5.2	3,900	6,200	840	.94	6.4	8.8	7,300	2.4	17	2,940	3,150	460	570	390	830	160	280
		(Dry				12	.37		45				5,900	8,000	1,040	1.67	5.4	8.2	9,600	3.9	17	4,760	5,200	730	830	470	880		270
Cedar, Port Orford (<i>Chamaecyparis lawsoniana</i>)	Oregon	(Green	14	23	34	43	.40	.44	36	10.1	4.6	6.9	4,000	6,200	1,420	.65	7.4	22.8	9,200	3.0	22	2,770	3,130	350	460	400	830	100	180
		(Dry				12	.42		29				7,700	11,300	1,730	1.97	9.1	19.5	13,500	5.0	28	5,890	6,470	760	730	560	1,080	220	400
Cedar, eastern red (<i>Juniperus virginiana</i>)	Vermont	(Green	5	12		35	.44	.49	37	7.8	3.1	4.7	3,400	7,000	650	1.08	15.0	34.7	7,000	2.7	35	2,540	3,570	860	760	650	1,010	180	330
		(Dry				12	.47		33				3,800	8,800	880	1.01	8.3		8,560	4.6	22		6,020	1,140	900	900	260		
Cedar, southern red (<i>Juniperus</i> sp.)	Florida	(Green	5	13		26	.42	.45	33	7.0	2.2	4.0	5,000	8,400	930	1.57	8.8	10.7	10,500	5.4	18	3,910	4,360	910	810	580	1,190	210	400
		(Dry				12	.44		31				7,300	9,400	1,170	1.88	5.4	6.6	10,200	4.2	17	5,190	6,570	1,000	1,010	610	750		
Cedar, western red (<i>Thuja plicata</i>)	Montana, Alaska, Washington	(Green	15	19	36	37	.31	.34	27	7.7	2.4	5.0	3,200	5,100	920	.63	5.0	10.1	6,900	2.5	17	2,470	2,750	340	430	270	710	140	230
		(Dry				12	.33		23				5,300	7,700	1,120	1.44	5.8	10.5	8,600	3.2	17	4,360	5,020	610	660	350	860	130	220
Cedar, northern white (<i>Thuja occidentalis</i>)	Wisconsin	(Green	5	23	36	55	.29	.32	23	7.0	2.1	4.7	2,600	4,200	640	.60	5.7	8.9	5,300	2.0	15	1,490	1,990	290	320	230	620	140	240
		(Dry				12	.31		22				4,900	6,500	800	1.72	4.8	6.0	7,100	2.8	12	2,630	3,960	380	450	320	850	150	240
Cedar, southern white (<i>Chamaecyparis thyoides</i>)	New Hampshire, North Carolina	(Green	10	16		35	.31	.35	26	8.4	2.8	5.2	2,500	4,700	750	.51	5.9	13.5	6,000	2.2	18	1,660	2,390	300	400	290	690	120	180
		(Dry				12	.32		23				4,800	6,800	930	1.46	4.1	5.2	7,600	3.0									

TABLE 1.—Strength and related properties of woods grown in the United States—Continued

Species (common and botanical names)	Place of growth of material tested	Moisture condition	Trees tested	Rings per inch	Summer wood	Moisture content	Specific gravity, oven dry, based on volume—		Weight per cubic foot	Shrinkage from green to oven-dry condition based on dimensions when green			Static bending						Impact bending			Compression parallel to grain		Compression perpendicular to grain; stress at proportional limit	Hardness; load required to embed a 0.444-inch ball to ½ its diameter		Shear parallel to grain; maximum shearing strength	Cleavage; load to cause splitting	Tension perpendicular to grain; maximum tensile strength			
							At test	When oven-dry		Volumetric	Radial	Tangential	Stress at proportional limit	Modulus of rupture	Modulus of elasticity	Work			Stress at proportional limit	Work to proportional limit	Height of drop causing complete failure (50-pound hammer)	Stress at proportional limit	Maximum crushing strength		Lb. per sq. in.	Lb. per sq. in.				Lb. per sq. in.	Pounds	Pounds
																Proportional limit	Maximum load	Total														
1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30			
SOFTWOODS—continued																																
Juniper, alligator (<i>Juniperus pachyphloea</i>)	Arizona	Green	3			40	0.48	0.54	42	7.8	2.7	3.6	Lb. per sq. in. 3,600	Lb. per sq. in. 6,600	1,000 lb. per sq. in. 450	In.-lb. per cu. in. 1.67	In.-lb. per cu. in. 13.4	In.-lb. per cu. in. 16.4	Lb. per sq. in. 6,800	In.-lb. per cu. in. 3.9	Inches 21	Lb. per sq. in. 2,490	Lb. per sq. in. 3,730	Lb. per sq. in. 1,030	Pounds 960	Pounds 820	Lb. per sq. in. 1,280	Lb. per sq. in. 160	Lb. per sq. in. 230			
		Dry				12	.51		36				5,400	6,700	650	2.74	6.5		5,600	2.5	12		4,120	3,800	1,700	1,290	1,160	920	160	230		
Larch, western (<i>Larix occidentalis</i>)	Montana, Washington	Green	13	32	37	58	.48	.59	48	13.2	4.2	8.1	4,600	7,500	1,350	1.01	7.1	18.2	9,400	3.7	24	3,250	7,490	1,080	1,110	760	1,360	160	310			
		Dry				12	.52		36				7,900	11,900	1,710	2.46	8.0	18.5	15,100	7.3	32	5,950	7,490	1,080	1,110	760	1,360	160	310			
Pine, jack (<i>Pinus banksiana</i>)	Wisconsin	Green	5	7	30	105	.39	.46	50	10.4	3.4	6.5	3,000	5,400	920	.55	5.9	21.0	7,800	3.3	30	2,180	2,580	380	380	370	760	180	310			
		Dry				12	.43		30				5,000	7,900	1,220	1.20	5.4	11.8	11,100	4.7	35		5,400	820	660	580	1,120	200	390			
Pine, jeffrey (<i>Pinus jeffreyi</i>)	California	Green	5	18	23	101	.37	.42	47	9.9	4.4	6.7	3,200	5,000	980	.60	4.7	14.1	7,200	2.6	21	2,050	2,370	350	320	340	690	160	260			
		Dry				12	.40		28				7,200	9,300	1,240	2.43	6.6	11.4	12,500	5.3	27	4,240	5,530	790	610	500	1,210	250	380			
Pine, limber (<i>Pinus flexilis</i>)	New Mexico	Green	2	14	24	68	.37	.42	39	8.2	2.4	5.1	3,900	5,200	800	1.08	5.2	8.3	7,100	2.6	18	1,850	2,410	320	300	310	740	170	270			
		Dry				12	.40		28				6,600	9,100	1,170	2.13	6.8	8.7	11,400	5.2	19		5,290	720	510	430	800	280	270			
Pine, loblolly (<i>Pinus taeda</i>)	Florida, Maryland, North Carolina, South Carolina, Virginia	Green	56	9	34	81	.47	.54	53	12.3	4.8	7.4	4,100	7,300	1,410	.68	8.2	24.2	8,900	3.0	30	2,550	3,490	480	420	450	850	180	260			
		Dry				12	.51		36				7,800	12,800	1,800	1.92	10.4	17.5	12,100	4.2	30	4,820	7,080	980	750	690	1,370	270	470			
Pine, lodgepole (<i>Pinus contorta</i>)	Wyoming, Colorado, Montana	Green	28	24	22	65	.38	.43	39	11.5	4.5	6.7	3,000	5,600	1,080	.49	5.6	11.9	7,200	2.3	20	2,110	2,610	310	320	330	680	150	220			
		Dry				12	.41		29				6,700	9,400	1,340	1.97	6.8	12.1	9,600	3.8	20	4,310	5,370	750	580	480	880	190	290			
Pine, longleaf (<i>Pinus palustris</i>)	Louisiana, Mississippi, Florida, South Carolina	Green	144	14	39	63	.54	.62	55	12.2	5.1	7.5	5,200	8,700	1,600	.95	8.9	32.4	10,100	3.2	35	3,430	4,300	590	550	590	1,040	210	330			
		Dry				12	.55		41				9,300	14,700	1,990	2.44	11.8	21.9	15,400	6.1	34	6,150	8,440	1,190	920	870	1,500	270	470			
Pine, mountain (<i>Pinus pungens</i>)	Tennessee	Green	5	15	29	75	.49	.55	54	10.9	3.4	6.8	4,500	7,500	1,270	.94	8.1	25.2	10,200	3.8	29	2,980	3,540	560	480	490	960	200	320			
		Dry				12	.52		36				7,900	11,600	1,550	2.30	8.7	15.8	14,200	6.4	29	4,260	6,830	1,210	730	660	1,200	290	360			
Pine, northern white (<i>Pinus strobus</i>)	Wisconsin, Minnesota, New Hampshire	Green	15	13	29	68	.34	.37	36	8.2	2.3	6.0	3,100	5,000	1,020	.54	5.2	10.8	6,700	2.2	17	2,060	2,490	290	310	310	660	140	240			
		Dry				12	.36		25				6,000	8,900	1,280	1.59	6.7	10.5	9,500	3.7	19	3,680	4,840	550	500	490	860	160	300			
Pine, Norway (<i>Pinus resinosa</i>)	Wisconsin	Green	5	22	41	54	.44	.51	42	11.5	4.6	7.2	3,700	6,400	1,380	.59	5.8	28.4	7,500	2.2	28	2,410	3,080	360	360	340	780	160	190			
		Dry				12	.48		34				9,400	12,500	1,800	2.78	10.0	16.9	15,900	6.8	25	5,330	7,340	830	670	580	1,230	290	430			
Pine, pitch (<i>Pinus rigida</i>)	Tennessee, Massachusetts	Green	10	12	28	79	.45	.52	50	10.9	4.0	7.1	3,600	6,800	1,200	.68	9.2	27.9	9,000	3.2	28	1,950	2,950	450	420	470	860	190	280			
		Dry				12	.49		34				6,900	10,500	1,430	1.62	9.2	15.4	12,600	5.8	31	3,960	5,940	1,010	700	620	1,360	260	450			
Pine, pond (<i>Pinus rigida serotina</i>)	Florida	Green	5	13	35	56	.50	.58	49	11.2	5.1	7.1	4,500	7,400	1,280	.93	7.5	26.8	9,400	3.2	33	2,940	3,660	540	460	510	940	190	280			
		Dry				12	.54		38				8,300	11,600	1,750	2.21	8.6	16.0	13,200	5.0	28	6,300	7,540	1,120	780	740	1,380	240	380			
Pine, ponderosa (<i>Pinus ponderosa</i>)	Colorado, Washington, Arizona, Montana, California	Green	126	19	30	91	.38	.42	45	9.6	3.9	6.3	3,100	5,000	970	.59	5.1	12.4	6,800	2.5	20	2,070	2,400	360	300	310	680	170	280			
		Dry				12	.40		28				6,300	9,200	1,260	1.85	6.6	10.8	9,800	4.0	17	4,060	5,270	740	550	450	1,160	220	400			
Pine, sand (<i>Pinus clausa</i>)	Florida	Green	5	7	30	36	.45	.51	38	10.0	3.9	7.3	4,100	7,500	1,020	.95	9.6	20.6	9,800	4.6	25	2,670	3,440	560	460	480	1,140	230	380			
		Dry				12	.48		34				6,700	11,600	1,410	1.83	9.6	17.4	12,400	5.4	19	3,900	6,920	1,030	950	730	1,100	290	360			
Pine, shortleaf (<i>Pinus echinata</i>)	Arkansas, Louisiana, North Carolina, New Jersey, Georgia	Green	136	12	31	81	.46	.54	52	12.3	4.4	7.7	3,900	7,300	1,390	.63	8.2	26.1	8,600	2.9	30	2,500	3,430	440	410	440	850	200	320			
		Dry				12	.51		36				7,700	12,800	1,780	1.93	11.0	16.6	13,600	5.2	33	5,090	7,070	1,000	750	690	1,310	270	470			
Pine, slash (<i>Pinus caribaea</i>)	Florida, Louisiana	Green	30	9	44	66	.56	.66	58	12.2	5.5	7.8	5,100	8,900	1,580	1.02	9.5	30.6	10,800	3.9	36	3,040	4,340	680	600	630	1,000	230	400			
		Dry				12	.61		43				9,800	15,900	2,060	2.76	12.6	20.8	15,800	5.8	36	6,280	9,100	1,390	1,080	1,010	1,730	290	570			
Pine, sugar (<i>Pinus lambertiana</i>)	California	Green	9	13	32	137	.35	.38	52	7.9	2.9	5.6	3,400	5,100	940	.70	5.4	12.0	7,400	2.6	17	2,330	2,530	350	320	310	680	180	270			
		Dry				12	.36		25				5,700	8,000	1,200	1.53	5.5	8.0	10,700	4.4	18	4,140	4,770	590	530	390	1,050	190	350			
Pine, western white (<i>Pinus monticola</i>)	Montana, Idaho	Green	15	20	33	54	.36	.42	35	11.8	2.6	5.3	3,400	5,200	1,170	.56	5.0	17.9	7,600	2.6	19	2,430	2,650	290	310	310	640	160	260			
		Dry				12	.38		27				6,200	9,500	1,510	1.47	8.8	14.1	11,900	4.2	23	4,480	5,620	540	440	370	850	160	260			
Piñon (<i>Pinus edulis</i>)	Arizona	Green	3	17	22	63	.50	.57	51	9.9	4.6	5.2	2,600	4,800	650	.61	7.6	23.0	8,200	4.0	12	1,810	2,590	480	510	600	920	200	460			
		Dry				12	.53		37				5,000	7,800	1,140	1.86	4.7	6.1	8,500	3.0	12		6,400	1,520	920	860						
Redwood (virgin) (<i>Sequoia sempervirens</i>)	California	Green	16	29		112	.38	.42	50	6.8	2.6	4.4	4,800	7,500	1,180	1.18	7.4	15.2	8,200	3.2	21	3,700	4,200	520	570	410	800	170	280			
		Dry				12	.40		28				6,900	10,000	1,340	2.04	6.9	8.8	10,900	3.6	14	4,560	6,150	860	790	480	940	150	240			
Redwood (second growth, openly grown) (<i>Sequoia sempervirens</i>)	do	Green	6	3		146	.28	.31	43	6.3	2.0	4.4	2,800	4,600	640	.65	5.1	6.3	5,900	2.5	11	2,660	3,810	550	590	340	840	160	240			
		Dry				12	.30		21				4,200	6,400	760	1.35	4.7	4.9	7,200	2.3	17	2,840	3,280	350	470	350	730	180	280			
Redwood (second growth, closely grown) (<i>Sequoia sempervirens</i>)	do	Green	8	7		112	.32	.36	42	7.4	2.4	5.0	3,600	6,100	1,000	.73	6.1	7.9	9,100	3.2	16	3,750	5,240	640	710	490	930	160	280			
		Dry				12	.34		24				5,500	8,300	1,120	1.45	5.7	10.4	6,800	1.8	24	1,540	2,570	180	430	370	660	120	100			
Spruce, black (<i>Picea mariana</i>)	New Hampshire	Green	5	15		38	.38	.43	32	11.3	4.1	6.8	2,900	5,400	1,060	.45	7.4	20.4	7,400	6.2	23	4,520	5,320	650	700	520	1,030	160	280			
		Dry				12	.40		28				5,800	10,300	1,530	1.34	10.5	21.4	13,400	1.9	14	1,680	1									

1 The averages for this species include data from tests representing an unknown number of trees in addition to the number indicated.

COMMON AND BOTANICAL NAMES OF SPECIES (COLUMN 1)

For convenience, the species listed in table 1 are grouped in two major classifications:

(1) Hardwoods, or trees with broad leaves, usually deciduous; (2) softwoods, or trees with needle or scalelike leaves, usually evergreen and most of them cone-bearing. The two groups are also known as hardwoods and conifers. The terms "hardwoods" and "softwoods" are thus indicative of botanical classification. They are not correlated with the actual hardness or softness of the wood. For example, basswood, poplar, aspen, and cottonwood are classified as hardwoods but are in reality among the softest of native woods, whereas longleaf pine, classed as a softwood, is quite hard.

Avoidance of confusion requires a standard nomenclature for species of wood many of which are known by several common names and to several of which a single common name is often applied. The United States Forest Service has adopted such a nomenclature, designating each species by a single common name, in addition to a botanical name about which confusion rarely exists. The official names are used herein and are those given in Check List of the Forest Trees of the United States, their Names and Ranges, except for a few subsequent changes. Page 92 shows the relation between this nomenclature and commercial lumber names (46, 54).

PLACE OF GROWTH OF MATERIAL TESTED (COLUMN 2)

In the second column are listed the States from which the trees furnishing the test specimens were obtained. The locality of growth has in some instances an influence on the strength of timber (p. 43). That this influence is, however, frequently overestimated is indicated by the fact that fully as great differences have been found between stands of different character grown in the same section of the country as between stands grown in widely separated regions within the normal range of growth. For this reason it is considered better to average together the test data on material from the various localities. However, there is a distinct difference in the properties of Douglas fir from the more arid Rocky Mountain region and those of the Douglas fir from the Pacific Northwest. Further, Douglas fir from the so-called "Inland Empire"⁴ region is found to be intermediate in its characteristics between that from the arid Rocky Mountain region and that from the Pacific Northwest. For these reasons separate averages are given for Douglas fir from the Pacific coast, intermediate type, and the Rocky Mountain regions.

MOISTURE CONDITION (COLUMN 3)

Both green and dry material were tested. The resulting data are entered in lines designated "green" and "dry", respectively, in column 3.

Values in the first of each pair of lines beginning with column 3 of table 1 are from tests on green material. Although the moisture content varies among the different species, all tests on green wood were made at approximately the moisture content of the living tree,

⁴ Northwestern Montana, Idaho north of the Salmon River, Washington east of the Cascade Mountains, and the northeastern tip of Oregon.

which is above the limit ⁵ below which differences in moisture content affect the strength properties.

The strength of dry or partially dry wood depends greatly on the particular stage of dryness and on the distribution of the moisture. Values pertaining to a uniformly distributed moisture content of 12 percent are listed in the second of each pair of lines beginning with column 3. These values were obtained by adjusting values obtained from tests made at various moisture contents. The moisture basis adopted (12 percent) represents an average air-dry condition attained without artificial heat by thoroughly seasoned wood over a considerable portion of the United States, including the Lake States region.

Table 1 shows that in most strength properties the dry material in the form of small, clear specimens excels the green. In large timbers, however, the increased strength of the wood fibers is usually offset by checks and other defects resulting from drying, so that as large increases in strength values as in small specimens cannot be expected.

Except where data on dry material are specifically required, or where significant differences in increase with seasoning is involved, the data on green material are preferable for comparing species, because they are based on a larger number of tests.

NUMBER OF TREES TESTED (COLUMN 4)

The number of trees from which specimens were obtained is stated in the fourth column of table 1. The average values for the more important species represent groups of trees from different localities. Five trees of a species were selected, as a rule, from a single locality.

NUMBER OF RINGS PER INCH (COLUMN 5)

The number of rings per inch measures the rate of growth in diameter or radius of the trees from which the test specimens were cut. Rings per inch were counted along a radial line on the end section of each specimen. One ring, consisting of a band of spring wood and a band of summer wood, is formed during each year. Few rings per inch indicate fast growth, and conversely.

Rate of growth of many species is quite variable, and the values listed are to be regarded mainly as averages of the material tested. Rate of growth does not have a definite relation to strength in the sense of strength being proportional, either directly in inversely, to the rate of growth (p. 44).

SUMMER WOOD (COLUMN 6)

Column 6 shows the proportion of summer wood in the material tested, as measured along a representative radial line. Summer wood is usually much denser than spring wood⁶ of the same species so that within a species the proportion of summer wood is indicative

⁵ Green wood contains "absorbed", or "imbibed", water within the cell walls and "free" water in the cell cavities. The free water from the cell cavities is the first to be evaporated in drying. The fiber-saturation point is that point at which no water exists in the cell cavities of the timber but at which the cell walls are still saturated with moisture. The fiber-saturation point varies with the species (16). The ordinary proportion of moisture—based on the weight of the dry wood—at the fiber-saturation point is about 30 percent. Most strength properties of wood begin to increase, and shrinkage begins to occur, when the fiber-saturation point is reached in seasoning.

⁶ Numerous determinations have shown that in the southern pines specific gravity of the summer wood is usually from 2 to 3 times as great as that of the spring wood.

of the specific gravity, and hence, of strength. It is difficult to measure the proportion of summer wood accurately and when the change from spring wood to summer wood is not marked or the contrast between them is not sharp, as in many species, the difficulty is even greater. For this reason the proportion of summer wood is given for only part of the species tested.

Summer wood is unusually well differentiated from spring wood in the southern yellow pines and Douglas fir. Some of the structural grading rules for these species involve, among other features, the selection of pieces showing one-third or more summer wood, such material being awarded as a premium higher working stresses (54, 61).

MOISTURE CONTENT (COLUMN 7)

Moisture content is the weight of water contained in the wood, expressed as a percentage of the weight of the oven-dry wood. Since it is thus expressed it is useful to remember that with a given moisture content in percent a block of wood of a given size contains more weight or volume of water if the wood is heavy than if it is light. Moisture content is commonly determined by weighing a sample and then drying it at 212° F. (100° C.) until the weight becomes constant. The loss of weight divided by the weight of the oven-dry wood is the proportion of moisture in the piece. "Moisture" as thus determined is subject to some inaccuracy, because the loss in weight includes that of any substances other than moisture that evaporate at 100° C. Also some constituents other than actual wood substance are not evaporated. Errors from these sources are not sufficient to affect the practical application of the data given in column 7.

The moisture content listed in table 1 for green material is the average for specimens taken from the pith to the circumference of the log. Hence it represents a combination of the moisture as found in the heartwood and in the sapwood, although not in proportion to the amount of wood represented by each. In each instance 12 percent is entered as the moisture content of "dry" material, because the data have all been adjusted to this basis.

As shown by table 1, the average moisture content of the green wood varies widely among species. Also moisture content often differs between heartwood and sapwood of the same species and in some instances varies with height in the tree. Many coniferous species have a large proportion of moisture in the sapwood and much less in the heartwood. Most hardwoods on the other hand show much more nearly the same moisture content in heartwood and sapwood (p. 29). Extreme limits observed in the moisture content of green wood range from as low as 30 to 40 percent in the heartwood of such species as black locust, white ash, Douglas fir, southern pines, and various cedars to about 200 percent in the sapwood of some coniferous species. In the heartwood of some species the moisture content is high at the base of the tree and becomes less toward the top. For example, in green redwood trees examined at the Forest Products Laboratory, the heartwood decreased in average moisture content from 160 percent at stump height to 60 percent at heights above 100 feet. In this instance the sapwood increased slightly in percentage moisture with height in tree.

SPECIFIC GRAVITY (COLUMNS 8 AND 9)

Specific gravity is the relation of the weight of a substance to that of an equal volume of water.

The volume occupied by a specified weight of wood substance changes with the shrinking and swelling caused by changes in moisture content. In table 1, three values of specific gravity are given for each species. They correspond to volumes when green, at 12-percent moisture, and oven-dry, and each is based on the weight of the wood when oven-dry. The number of pounds of wood (exclusive of moisture) in a cubic foot at either of the three moisture conditions may be found by multiplying the specific gravity figure by 62.4. To get the weight per cubic foot of the wood plus that of the associated water, multiply by the factor:

$$1 + \frac{\text{percentage moisture content}}{100}$$

Additional data on the specific gravity of a number of species are presented on page 30. For some species these data are more extensive than those of table 1.

SPECIFIC GRAVITY BASED ON VOLUME WHEN GREEN (COLUMN 8)

Values of specific gravity, based on weight when oven-dry and volume when green, are determined from weights and measurements of specimens tested when green. The weight when oven-dry is computed by dividing the weight when green by 1 plus the proportion of moisture, as found from a moisture determination on the same specimen.

The specific-gravity values based on volume when green, as listed in column 8, are averages of determinations made on each green test specimen. The number of determinations is much larger in most instances than those of specific gravity based on volume when air-dry or when oven-dry.

SPECIFIC GRAVITY BASED ON VOLUME WHEN AIR-DRY (COLUMN 8)

Specific gravity based on volume when air-dry is found in the same manner as that based on volume when green, except that the volume measurements are made on air-dry material. The values for air-dry wood listed in column 8 are adjusted to a volume basis corresponding to 12-percent moisture content.

SPECIFIC GRAVITY BASED ON VOLUME WHEN OVEN-DRY (COLUMN 9)

In determining the specific gravity based on volume when oven-dry, the volume as well as the weight is taken after the specimens are oven-dried to practically constant weight at 100° C.

Specific gravity, as listed in column 9, and shrinkage in volume, as listed in column 11, were determined on the same specimens of which there were usually 4 to 6 from a tree.

The difference between specific gravity based on volume when green and that on volume when air-dry or oven-dry, is due to shrinkage, and either specific gravity may be determined from the other if the corresponding shrinkage in volume is known. For example, specific gravity based on weight and volume when oven-dry equals specific

gravity based on weight when oven-dry and volume when green divided by

$$\left(1 - \frac{\text{percent volumetric shrinkage}}{100}\right)$$

As the determinations of specific gravity, based on volume when oven-dry, and of volumetric shrinkage were made on only a few specimens from each bolt, they are not related to specific gravity based on weight when oven-dry and volume when green in exact accordance with this equation.

WEIGHT PER CUBIC FOOT (COLUMN 10)

Changes in moisture content affect the weight of a piece of wood. When the moisture content is below the value at the fiber-saturation point (p. 48), changes in the moisture content also affect the volume of the piece. Consequently, in order to be specific in stating weight per cubic foot, various degrees of dryness must be recognized.

Green or freshly cut wood, contains, as shown in column 7, a considerable proportion of water. After being dried by exposure to the air until the weight is practically constant, wood is said to be "air-dry." If dried in an oven at 212° F. (100° C.) until all moisture is driven off, wood is "oven-dry."

The weights per cubic foot presented in table 1 are based on weights and volumes of small, clear specimens taken usually from the top 4 feet of 16-foot butt logs of typical trees. Because the wood from such portions is often heavier than that from higher in the tree, material thus selected averages slightly heavier than the wood in ordinary timbers, poles, posts, or railway ties.

WEIGHT PER CUBIC FOOT WHEN GREEN

The value for green wood as given in column 10 includes the moisture in the wood as received at the laboratory, and because protection from seasoning was afforded during transit and pending test, it represents closely the weight of the wood as it comes from the living tree. The weight when green is based on the average of heartwood and sapwood pieces as represented by test specimens taken from pith to circumference. In those species which have a higher moisture content in the sapwood, variations in the proportion of sapwood are accompanied by comparatively large variations in weight per cubic foot of green material.

The weights per cubic foot in column 10 correspond to the average moisture-content values listed in column 7. When in specific instances there are large differences in moisture content between heartwood and sapwood and the proportion of sapwood in logs or other products is known, better estimates of the weight per cubic foot when green may be obtained by correcting the value given in column 7 to a suitable moisture content. For example, the weight and moisture content of ponderosa pine are given in table 1 as 45 pounds per cubic foot and 91 percent, respectively. The average moisture content of ponderosa pine logs having 75 percent sapwood by volume is computed on page 30 as 121 percent. The estimated weight of such logs is then

$$45 \left(\frac{100 + 121}{100 + 91} \right) = 51\frac{3}{4} \text{ pounds per cubic foot.}$$

WEIGHT PER CUBIC FOOT WHEN AIR-DRY

Weight per cubic foot depends upon the amount of moisture in the wood which in turn depends on the species, the size and form of the pieces, the length of the seasoning period, and on the rapidity of seasoning as governed by the climate. The average air-dry condition reached in the northern Central States by wood that is sheltered from rain and snow and not artificially heated, is a moisture content of about 12 percent. The values for dry wood in column 10 apply to this moisture content. The moisture content of thoroughly air-dry wood may be 3 to 5 percent higher in humid regions, and in very dry climates, as much lower. It also varies slightly from day to day because of changes in temperature and atmospheric humidity. Large timbers will have a slightly higher average moisture content when thoroughly air-dry than small pieces. Species vary in the rate at which they give off moisture in drying, and also in the rate at which they take up moisture during periods of wet or damp weather.

Changes of several percent in the moisture content of dry wood cause only small changes in the weight per cubic foot, because of two actions which tend to counteract one another. The weight decreases as drying takes place because of the loss of moisture. At the same time shrinkage reduces the volume. Conversely, both weight and volume increase as moisture is absorbed.

Weight per cubic foot at a moisture content near 12 percent may be estimated from that at 12 percent by assuming that one-half percent increase or decrease in weight accompanies an increase or decrease of 1 percent in moisture content. Thus, raising the moisture content from 12 to 14 percent increases the weight per cubic foot about 1 percent and in drying from 12- down to 8-percent moisture content the weight per cubic foot is reduced about 2 percent.

SHRINKAGE (COLUMNS 11, 12, AND 13)

Shrinkage across the grain (in width and thickness) results when wood loses some of the absorbed moisture (pp. 6, 48). Conversely, swelling occurs when dry or partially dry wood is soaked or when it takes moisture from the air or other source. Shrinkage and swelling in the direction of the grain (length) of normal wood is only a small fraction of 1 percent and is too small to be of practical importance in most uses of wood.⁷ All shrinkages are expressed as percentages of the original or green dimensions.

Column 11 lists for the various species the shrinkage in volume from the green to the oven-dry condition. The values are averages from actual volume determinations on small specimens.

In columns 12 and 13 are average values of the measured radial and tangential shrinkages in drying standard specimens from the green to the oven-dry condition. Radial shrinkage is that across the annual growth rings as in the width of a quarter-sawed board. Tangential shrinkage is that approximately parallel to the annual-growth rings as in the width of a flat-sawed board.

The shrinkage of any piece of wood depends on numerous factors, some of which have not been thoroughly studied. In all species listed in table 1 the radial shrinkage is less than the tangential. Hence,

⁷ Appreciable longitudinal shrinkage is associated with "compression wood", and other abnormal wood structure (p. 72).

quarter-sawed (edge-grained) boards shrink less in width but more in thickness than flat-sawed boards. The smaller the ratio of radial to tangential shrinkage for a species, the greater is the advantage to be gained through minimizing shrinkage in width by using quarter-sawed wood. Also, the less the difference between radial and tangential shrinkage, the less ordinarily is the tendency of the wood to check in drying and to cup when its moisture content changes.

Air-dry wood takes on or gives off moisture with each change in weather or heating conditions. The fact that time is required for these moisture changes, causes a lag between atmospheric changes and their full effect on the moisture condition of the wood. The lag is greater in some species than in others, greater in heartwood than in sapwood, and is much less in small than in large pieces. It is increased by protective coatings such as paint, enamel, or varnish. Some species whose shrinkage from the green to the oven-dry condition is large cause less inconvenience in use than woods with lower total shrinkage, because their moisture content does not respond to atmospheric changes so closely. The shrinkage figures given do not take into account the readiness with which the species take on and give off moisture, and therefore should be considered as the relative shrinkage between woods after long exposure to fairly uniform atmospheric conditions or with the same change in moisture content.

The values listed in columns 11, 12, and 13 are shrinkages from the green to the oven-dry condition and thus are much greater than ordinarily occur in the seasoning of wood or with changes in moisture content subsequent to seasoning. About half the listed value represents the shrinkage from green to the average air-dry condition of 12 to 15 percent moisture. A change in moisture content of dry material by 1 percent may be expected to produce a percentage shrinkage or swelling of about one twenty-fifth of the value listed in columns 11, 12, or 13.

MECHANICAL PROPERTIES (COLUMNS 14 TO 30)

Columns 14 to 30 inclusive list the average values obtained from tests made according to the standardized procedure (pp. 4, 78). For convenience and ease of reference, each of the column headings is discussed independently in the order in which it appears in the table. The reliability of the averages and the significance of differences between species is discussed in a later section on variability. Appreciation of the significance of the values and of how they should be modified to apply to conditions of use differing from those under which the tests were made will be enhanced by study of later discussions, particularly those on form factors and effect of duration of stress. Modifications to make them applicable to material affected by various types of defects are indicated by the discussion of factors affecting strength.

STRESS AT PROPORTIONAL LIMIT, STATIC BENDING (COLUMN 14)

The proportional limit in any test is the limit of proportionality between load (or stress) and deformation (or strain). When load is increased by a given percentage without passing this limit, deformation increases by the same percentage. With an increase in load beyond the proportional-limit value, deformation increases by a

greater percentage than the load. Both these facts are illustrated by the load-deflection graph shown on page 80.

In accordance with current practice (3) in the field of testing materials this bulletin uses "proportional limit", instead of "elastic limit", as used in previous Forest Service publications, to designate the limit of proportionality between stress and strain or between load and deformation.

The determination of the proportional limit in any test is subject to uncertainty because it is somewhat dependent on the increments of load and deflection used in testing and on personal judgment in locating the point of departure from the straight-line relation in such a diagram as shown on page 80. Values of load and deformation at proportional limit for wooden members depend on the rate at which the load is increased and on the length of time it acts on the member. This is illustrated by the fact that stress and deformation at proportional limit are much greater in impact bending, in which the specimen is subjected to instantaneous shocks, than in static bending in which the load increases at a moderate rate.

Because a piece stressed within the proportional limit recovers from its deformation on removal of the load and release of the piece from stress, the proportional limit is sometimes called the elastic limit.

Tests have demonstrated that loads in bending or in compression parallel to grain that exceed the proportional-limit values as found from tests made at the standard speeds (4) will ultimately cause failure if they continue to act on a wooden member. Thus, these proportional-limit values of stress are upper limits to the stresses that can be used in the design of permanent structures. In determining safe working stresses, factors of safety must be applied to average values of stress at proportional limit in order to allow for variations below the average and to provide for the contingency that the member will be loaded more heavily than was assumed in its design. The effects of duration and repetition of stress are discussed on page 59.

Stress at proportional limit in static bending (column 14) is the stress that exists in the top and bottom fibers of a beam at the proportional limit load. It is in general applicable to clear beams of rectangular cross section, although a slight adjustment is necessary to adapt values from the standard 2- by 2-inch specimen to pieces of other sizes. In estimating the strength of beams of special forms, such as I, circular, box, or diamond-shaped cross sections, on the basis of the data derived from square specimens as presented herein, the effect of the shape and proportions of the section (p. 63) must be considered.

MODULUS OF RUPTURE, STATIC BENDING (COLUMN 15)

Modulus of rupture is the computed stress in the top and bottom fibers of a beam at the maximum load and is a measure of the ability of a beam to support a slowly applied load for a short time. The formula by which it is computed is based on assumptions that are valid only to the proportional limit, hence modulus of rupture is not a true stress. It is, however, a widely accepted term and values for various species are quite comparable.

Since the modulus of rupture is based on the maximum load, which is directly determinable, it is less influenced by personal and other factors than proportional limit values.

The modulus-of-rupture values are used to compare the bending strengths of different species, and in conjunction with the results of tests on timbers containing defects to determine safe working stresses for structural timbers.

Like stress at proportional limit, modulus of rupture as found from the standard 2- by 2-inch specimens requires some modification to adapt it to square or rectangular beams of other sizes or to make it applicable to beams of I, circular, box, or diamond-shaped cross section (p. 63).

MODULUS OF ELASTICITY, STATIC BENDING (COLUMN 16)

Modulus of elasticity is a measure of the stiffness or rigidity of a material. The deflection of a beam under load varies inversely as the modulus of elasticity; that is, the higher the modulus the less the deflection. Modulus of elasticity is useful for computing the deflections of joists, beams, and stringers under loads that do not cause stress beyond the proportional limit. It is also used in computing the load that can be carried by a long column, because for such columns the load depends on the stiffness, and not on the crushing strength of the wood parallel to the grain.

Some of the deflection that occurs in the bending of a wooden beam is due to shear distortion, the amount varying with the proportions of the piece and the placement of the load. About one-tenth of the deformation measured in tests of the standard bending specimen is due to shearing distortion. The true moduli of elasticity are consequently about 10 percent higher than the values in column 16.

WORK TO PROPORTIONAL LIMIT, STATIC BENDING (COLUMN 17)

Work to proportional limit in static bending, as the name implies, is a measure of the energy that the beam absorbs in being stressed to the proportional limit. Since work is the product of average force times the distance moved, work to proportional limit involves both the load and the deflection at the proportional limit.

Values of work to proportional limit may be used to compare the ability of different species to withstand a combination of high load and high deflection without appreciable injury. Hence, they measure the toughness of a piece to the elastic limit. It is a comparative property only and cannot be used directly like modulus of rupture in strength calculations.

WORK TO MAXIMUM LOAD, STATIC BENDING (COLUMN 18)

Work to maximum load in static bending represents the capacity of the timber to absorb shocks that cause stress beyond the proportional limit and are great enough to cause some permanent deformation and more or less injury to the timber. It is a measure of the combined strength and toughness of a material under bending stresses. Superiority in this quality makes hickory better than ash, and oak better than longleaf pine for such uses as handles and vehicle parts subjected to shock. Work to maximum load is closely related to height of drop in impact bending as a measure of shock resistance.

Work-to-maximum-load values cannot be used directly in design, but, like many others, their usefulness is limited to comparisons.

TOTAL WORK, STATIC BENDING (COLUMN 19)

Total work in static bending is a measure of the toughness under bending stresses that cause complete failure. Like work to maximum load, it is a measure of that quality which makes hickory a superior wood for handles, and other uses involving shock resistance. It is also indicative of the same quality as is measured by height of drop in impact bending.

STRESS AT PROPORTIONAL LIMIT, IMPACT BENDING (COLUMN 20)

The stress at proportional limit is the computed stress in the top and bottom fibers of the beam at the proportional limit (pp. 11, 84). The stress at proportional limit averages approximately twice as great in impact as in static bending. It is mainly of use in comparing species with respect to their elastic behavior under impact loads. Stress at proportional limit is the only stress computed from the standard-impact-bending test.

It is impossible from the measurements made in this test to find the maximum force between the hammer and the specimen or to compute a maximum stress value analogous to modulus of rupture in static bending. That such a value would, if determined, be considerably higher than modulus of rupture is demonstrated by the fact that stress at proportional limit in impact averages somewhat higher than modulus of rupture. In a few tests in which specimens were broken by a single impact and the maximum force acting on the specimen found from records of the deceleration of the hammer, the computed maximum stress was approximately 75 percent higher than modulus of rupture of similar specimens tested in static bending (58).

WORK TO PROPORTIONAL LIMIT, IMPACT BENDING (COLUMN 21)

The work to proportional limit in impact bending is a measure of the energy that the beam absorbs in being stressed to the proportional limit. It involves both the deflection and the stress at proportional limit. Work to proportional limit is used to compare the ability of a timber to absorb shock and recover promptly without injury. It represents a quality important in such products as tool handles or tennis rackets. The values apply only to the resistance to falling bodies or like conditions in which the stress is applied and removed in a fraction of a second.

HEIGHT OF DROP OF HAMMER, IMPACT BENDING (COLUMN 22)

The height of drop of the hammer in impact bending is the height from which the 50-pound hammer is finally dropped to cause complete failure of the standard test specimen. It is a comparative figure expressing the ability of wood to absorb shock that causes stresses beyond the proportional limit. It represents a quality important in such articles as handles, and picker sticks, which are stressed in service beyond the proportional limit. Wood requiring a large height of drop to produce failure usually exhibits a splintering fracture when broken, whereas a small height of drop is associated with a brittle fracture.

STRESS AT PROPORTIONAL LIMIT, COMPRESSION PARALLEL TO GRAIN (COLUMN 23)

Stress at proportional limit is the greatest stress at which the compressive load remains proportional to the shortening of the specimen (pp. 11, 86).

The stress at proportional limit is applicable to clear compression members for which the ratio of length to least dimension does not exceed 11 to 1. It is the limiting stress in compression parallel to grain which should not be exceeded in determining safe loads. The stress at proportional limit in compression parallel to grain is taken into account in arriving at safe working stresses for short columns and other compression members, determining design values for bolted joints and the like. The stress at proportional limit averages about 80 percent of the maximum crushing strength for coniferous woods, and 75 percent for hardwoods.

MAXIMUM CRUSHING STRENGTH, COMPRESSION PARALLEL TO GRAIN (COLUMN 24)

Maximum crushing strength is the maximum ability of a short piece to sustain a slowly applied end load over a short period. It is applicable to clear compression members whose ratio of length to least dimension does not exceed 11. This property is important in estimating endwise crushing strength of wood, and in developing safe working stresses for structural timbers, design of bolted joints, and the like.

Maximum crushing strength is one of the simplest properties to determine. It is usually less adversely affected by various treatments or processes applied to wood than other strength properties, and hence should not be regarded as representative of other strength properties in appraising the effect of such treatments.

STRESS AT PROPORTIONAL LIMIT, COMPRESSION PERPENDICULAR TO GRAIN (COLUMN 25)

Stress at proportional limit is the maximum across-the-grain stress of a few minutes duration that can be applied without injury through a plate 2 inches wide and covering but a portion of the timber surface. It is useful in deriving safe working stresses in compression perpendicular to grain, for computing the bearing area for beams, stringers, and joists, and in comparing species for railroad ties and other uses in which this property is important.

In compression perpendicular to grain, particularly if the load is applied to only part of the surface area as in this test, wood does not exhibit a true ultimate or maximum strength as in compression parallel to grain and static bending; but the load continues to increase until the block is badly crushed and flattened out. Hence, no ultimate or maximum strength value is obtained.

In the standard test procedure, the specimen is placed with the direction of the annual growth rings parallel to the direction of the load except when this is impossible, such as with specimens from near the pith of the tree. Thus the load is applied to the radial face, but it should be pointed out that the fiber stress at proportional limit in compression perpendicular to grain like other across-the-grain properties of wood are very appreciably affected by ring placement.

Although there appears to be no consistent difference in fiber stress at proportional limit when the rings are parallel and perpendicular respectively to the direction of the applied load, appreciably lower values obtain when the rings are at an angle of 45° . This fact is of practical importance in timber design and use.

The fiber stress at proportional limit in compression perpendicular to grain depends also on the size of plate with respect to the length of the test specimen. With the surface of the specimen but partly covered, there is a component of tension parallel to grain at the edge of the plate, in addition to the compressive stress proper. Values of proportional limit lower than those obtained with the standard test are found when the plate covers the entire surface of the test specimen, and higher values result when the width of plate is decreased. The method of test employing a plate covering but part of the surface is somewhat analogous to the bearing conditions in service where a joist or beam rests on its supports.

HARDNESS (COLUMNS 26 AND 27)

Hardness is the load required to embed a 0.444-inch ball to one-half its diameter in the wood. It represents a property important in wood subjected to wear and marring, such as flooring, furniture, railroad ties, and paving blocks. The hardness test provides data for comparing different pieces or different species of wood, but the results cannot be used for calculating the size of members, as can such properties as modulus of rupture.

Hardness tests are made on end, radial, and tangential surfaces. End hardness values are given in column 26. There is no significant difference between radial and tangential hardness, and they are averaged together as "side hardness" in column 27.

In determining side hardness the principal stress is perpendicular to the grain, but because of the depth of penetration of the ball, a considerable component of end-grain hardness is introduced in the load. Likewise the end-hardness values reflect a component of side-grain hardness. Although end hardness is usually higher than side hardness, it is evident that the two are closely related.

Although hardness is the best available index of the ability of wood to resist wear, it is not so good a criterion of suitability as would be actual comparisons from some kind of abrasion tests that would more nearly simulate service conditions. However, no abrasion test for wood has yet been standardized and systematic results are not available.

MAXIMUM SHEARING STRENGTH, SHEAR PARALLEL TO GRAIN (COLUMN 28)

Maximum shearing strength is the average stress required to shear off from the test specimen a projecting lip having a length in the direction of the grain of 2 inches. Shearing strength parallel to the grain is a measure of the ability of timber to resist slipping of one part upon another along the grain. Shearing stress is produced in most uses of timber. It is important in beams, where it is known as horizontal shear—the stress tending to cause the upper half of the beam to slide upon the lower—and in the design of various kinds of joints.

It is difficult to devise a test that involves only shearing stress. A tensile component perpendicular to the grain of the wood influences the results of tests made by the standard method, but in general, the same effect in varying degree obtains in other methods in use or proposed. In obtaining the average shear values presented, a uniform distribution of stress throughout the shearing area is assumed, although it is not certain that uniformity obtains. The maximum shearing strength also varies with the amount of offset between the shearing force and the line of support of the specimen. Comparable values are obtained by standardizing the test procedure as in this series of tests.

LOAD TO CAUSE SPLITTING, CLEAVAGE (COLUMN 29)

Cleavage is the maximum load required to cause splitting of the standard specimen. It is expressed in pounds per inch of width.

It is evident that the maximum load in cleavage depends on the width and length of the specimen. In order to insure comparable results, the standard length of 3 inches is always maintained. The cleavage strength, like some of the other properties cannot be used directly for calculating required sizes of wood members or in similar design problems, but is useful mainly for comparisons. This test differs from the action of nails in splitting wood when driven, and should not be taken as a criterion of the relative resistance of the different species to such splitting.

MAXIMUM TENSILE STRENGTH, TENSION PERPENDICULAR TO GRAIN (COLUMN 30)

The maximum tensile strength perpendicular to the grain is the average maximum stress sustained across the grain by the wood.

The tabulated values are obtained by dividing the maximum load by the tension area. It is recognized that the tensile stress is not uniformly distributed over the area. Consequently, the values probably do not represent a true tensile strength. They are, nevertheless, useful for comparing species and for estimating the resistance of timber to forces acting across the grain.

VARIABILITY

Variability is common to all materials. If one tests pieces of wire from a roll, the loads necessary to pull the wire apart will vary. Likewise, the breaking strengths of different pieces of the same kind of string or rope are not the same. Materials, however, differ considerably in the amount of variation or the spread of values.

The growing tree is subject to numerous constantly changing influences that affect the wood produced, and it is not surprising that even the clear wood is variable in strength and other properties. The factors affecting tree growth include, soil, moisture, temperature, growing space, and heredity.

Everyone who has handled and used lumber has encountered variability and observed that different pieces even of the same species, are not exactly alike. The differences most commonly recognized are in the appearance, but even greater differences in weight and in strength properties occur and may be of greater importance.

The variability of wood can be illustrated by considering as an example the data on specific gravity of Douglas fir presented in table 2.

These data show that the specific gravity of the heaviest piece included in the series was nearly twice that of the lightest, and that the number of very heavy and very light pieces is small. Most of the values are grouped closely about the average.

TABLE 2.—*Results of specific gravity determinations on 1,240 samples of Douglas fir (coast type)*

Specific gravity ¹ group limits	Pieces in group		Specific gravity ¹ group limits	Pieces in group	
	Number	Percent		Number	Percent
0.300 to 0.309.....	2	0.16	0.460 to 0.469.....	96	7.74
0.310 to 0.319.....	7	.56	0.470 to 0.479.....	74	5.97
0.320 to 0.329.....	6	.48	0.480 to 0.489.....	70	5.65
0.330 to 0.339.....	15	1.21	0.490 to 0.499.....	56	4.52
0.340 to 0.349.....	13	1.05	0.500 to 0.509.....	46	3.71
0.350 to 0.359.....	23	1.85	0.510 to 0.519.....	41	3.31
0.360 to 0.369.....	25	2.02	0.520 to 0.529.....	30	2.42
0.370 to 0.379.....	38	3.06	0.530 to 0.539.....	23	1.85
0.380 to 0.389.....	47	3.79	0.540 to 0.549.....	12	.97
0.390 to 0.399.....	64	5.16	0.550 to 0.559.....	9	.73
0.400 to 0.409.....	75	6.05	0.560 to 0.569.....	10	.81
0.410 to 0.419.....	85	6.86	0.570 to 0.579.....	4	.32
0.420 to 0.429.....	76	6.13	0.580 to 0.589.....	1	.08
0.430 to 0.439.....	99	7.98	0.590 to 0.599.....	3	.24
0.440 to 0.449.....	100	8.06			
0.450 to 0.459.....	90	7.26	Total.....	1,240	100.00

¹ Based on weight when oven-dry and volume when green. Average specific gravity equals 0.445; highest observed specific gravity, 0.549; lowest, 0.308.

The manner in which the values are grouped about an average is called a frequency distribution, from which the chances that a random piece will differ from the average by a given amount can be estimated by computation. Such calculations, for example, assuming that the specific-gravity values conform to a so-called normal distribution, leads to the expectation that one-half of the Douglas fir samples would be within 7.9 percent of the average specific gravity, or within the limits 0.41 and 0.48 inclusive, and that one-fourth would be below 0.41 and one-fourth above 0.48. The figure defining such limits, 7.9 percent in this instance, is called the probable variation. By actual count 654 of the pieces or 52.7 percent of the total number (1,240) have a specific gravity between 0.41 and 0.48, whereas 25.4 percent (315) were below 0.38 and 21.9 percent (271) were above 0.48. Thus, as might be expected, the calculated percentages do not agree exactly with the actual count. Nevertheless, the agreement is sufficiently close to show the value of the theory in estimating the variability.

The range in strength properties can be studied and used as a basis for making estimates in a like manner.

After tests have been made it is, of course, easy to determine from the results the proportion of the test pieces within any given range, but one can only estimate the reliability of the averages and the degree to which this test data applies to other pieces. One would like to know the true average for each species, a quantity which cannot actually be determined. The best that can be done is to assume that the laws of chance are operative and thus estimate the probability of variations of given magnitude from the averages found. Such is the basis of the suggestions for estimating variability by means of data presented herein.

It would be desirable to present a measure of the variability of each property of each species. However, the extensive calculations involving all properties and species have not been made; and if available, their presentation would be involved. Although it is known that all species are not equally variable, existing information indicates that they are enough alike that estimates made on the assumption that the percentage variability in any one property is the same for all species will be sufficiently accurate for approximate calculations.

The questions that most frequently arise in a consideration of the variability of wood, are of two types:

(1) What is the significance of the differences between average values for two species or what is the likelihood that the averages will be changed a specified amount by additional tests?

(2) What is the range that includes a specified proportion of material of a species, or what is the likelihood that a piece selected at random will be within a specified range?

VARIATION OF AVERAGE VALUES

The probable variations of observed averages from the true averages enables one to appraise the significance of differences between observed averages. The estimated probable variation of the observed average from the true average of a species, when based on different numbers of trees, is given in table 3. The percentage probable variations listed in table 3 being average values for a number of species, an occasional species may be considerably more or less variable than indicated.

TABLE 3.—Percentages probable variation¹ of the observed average from the true average of a species, when based on material from different numbers of trees

Trees.....number.....	1	2	3	4	5	10	15	20	30	40	50
Specific gravity based on volume when green.....	4.7	3.3	2.7	2.4	2.1	1.5	1.2	1.0	0.9	0.7	0.7
Shrinkage:											
Radial.....	11.6	8.2	6.7	5.8	5.2	3.7	3.0	2.6	2.1	1.8	1.6
Tangential.....	9.0	6.4	5.2	4.5	4.0	2.8	2.3	2.0	1.6	1.4	1.3
Volumetric.....	8.8	6.2	5.1	4.4	3.9	2.8	2.3	2.0	1.6	1.4	1.2
Static bending:											
Fiber stress at proportional limit.....	11.2	7.9	6.4	5.6	5	3.5	2.9	2.5	2.0	1.8	1.6
Modulus of rupture.....	8.9	6.3	5.2	4.5	4	2.8	2.3	2.0	1.6	1.4	1.3
Modulus of elasticity.....	11.2	7.9	6.4	5.6	5	3.5	2.9	2.5	2.0	1.8	1.6
Work to proportional limit.....	15.6	11.1	9.0	7.8	7	5.0	4.0	3.5	2.9	2.5	2.2
Work to maximum load.....	13.4	9.5	7.7	6.7	6	4.2	3.5	3.0	2.4	2.1	1.9
Impact bending:											
Fiber stress at proportional limit.....	8.9	6.3	5.2	4.5	4	2.8	2.3	2.0	1.6	1.4	1.3
Work to proportional limit.....	11.2	7.9	6.4	5.6	5	3.5	2.9	2.5	2.0	1.8	1.6
Height of drop.....	15.6	11.1	9.0	7.8	7	5.0	4.0	3.5	2.9	2.5	2.2
Compression parallel to grain:											
Fiber stress at proportional limit.....	11.2	7.9	6.4	5.6	5	3.5	2.9	2.5	2.0	1.8	1.6
Maximum crushing strength.....	8.9	6.3	5.2	4.5	4	2.8	2.3	2.0	1.6	1.4	1.3
Compression perpendicular to grain:											
Fiber stress at proportionallimit.....	13.4	9.5	7.7	6.7	6	4.2	3.5	3.0	2.4	2.1	1.9
Hardness, end.....	8.9	6.3	5.2	4.5	4	2.8	2.3	2.0	1.6	1.4	1.3
Hardness, side.....	11.2	7.9	6.4	5.6	5	3.5	2.9	2.5	2.0	1.8	1.6
Shearing strength parallel to grain.....	6.7	4.7	3.9	3.4	3	2.1	1.7	1.5	1.2	1.1	.9
Tension perpendicular to grain.....	11.2	7.9	6.4	5.6	5	3.5	2.9	2.5	2.0	1.8	1.6

¹ The percentage probable variation of the average of a species is a figure such that there is an even chance that the true average is within this percentage of the observed average in table 1.

The observed average is always the most probable value of the true average. The importance of the differences between species with

respect to averages depends on the magnitude of this difference in relation to the probable variation of the averages, as well as on how exacting the strength requirements are for the particular use under consideration.

If the averages of any property of two species of table 1 differ by an amount equal to the probable variation of the difference,⁸ there is 1 chance in 4 that the true average for the species which is lower in that property on the basis of present data equals or exceeds the true average of the other. There is also 1 chance in 4 that the true average for the higher species exceeds that of the lower one by as much as twice the observed difference. When the averages differ by amounts that are $\frac{1}{2}$, 1, 2, 3, 4, or 5 times the probable variation of their difference the chances of the true average of the lower species equaling or exceeding the true average of the higher, or of the observed difference being at least doubled are given in the following tabulation:

	<i>Multiples</i>	<i>Chance</i>
$\frac{1}{2}$		1 in $2\frac{3}{4}$.
1.....		1 in 4.
2.....		1 in 11.
3.....		1 in 46.
4.....		1 in 285.
5.....		1 in 2,850.

As an example, consider the average values for modulus of rupture of 9,300 and 9,600 pounds per square inch for Biltmore white ash and blue ash, respectively, in the green condition (table 1). These averages being based on five trees of each species the probable variation according to table 3 is 4 percent. Then 4 percent of 9,300 equals 372, and 4 percent of 9,600 equals 384, the probable variations of these averages. The probable variation of the difference between the averages is then $\sqrt{(372)^2 + (384)^2}$ or 535; the observed difference in the averages for modulus of rupture (9,600-9,300) is 300. The ratio of the observed difference to the estimated probable variation being less than 1, it may be estimated from the tabulation that the chance that the true average modulus of rupture for Biltmore white ash equals or exceeds that for blue ash is somewhat greater than 1 in 4. There is the same chance that the true average of blue ash exceeds that for Biltmore white ash by as much as 600 or twice the difference in present average figures as shown in table 1. Therefore, the difference in modulus of rupture between blue ash and Biltmore white ash is not to be regarded as significant.

As a second example, consider the figures for modulus of rupture of 9,400 and 8,300 for sweet birch and yellow birch, respectively (table 1). The figures for sweet birch are based on 10 trees, those for yellow birch on 17. From table 3 the probable variation of the species average for modulus of rupture when based on 10 trees is 2.8 percent and when based on 17 trees it is 2.2 percent. (The figure for 17 trees is taken as between that given for 15 trees and 20 trees). Following the method of the preceding example, the probable variation of the difference between the averages is found to be 320. The difference between the observed averages is 1,100, which is about three and one-half times its probable variation of 320. The tabula-

⁸ The probable variation of the difference of two average figures is the square root of the sum of the squares of the probable variations of the averages. The probable variation of the average of any property may be estimated from the figures in table 3.

tion indicates that the chance that the true average for modulus of rupture of yellow birch would equal or excel that for sweet birch is less than 1 in 46. The importance of such differences will depend on the use to be made of the wood.

VARIATION OF AN INDIVIDUAL PIECE FROM THE AVERAGE

The upper and lower limits for any property within which one-half of the material of a species would be expected to fall may be estimated from the following tabulation.

Estimated probable variation of an individual piece from average for species

<i>Property:</i>	<i>Percent</i>
Specific gravity based on volume when green.....	8
Shrinkage:	
Radial.....	11
Tangential.....	10
Volumetric.....	12
Static bending:	
Fiber stress at proportional limit.....	16
Modulus of rupture.....	12
Modulus of elasticity.....	16
Work to maximum load.....	23
Impact bending:	
Fiber stress at proportional limit.....	13
Height of drop.....	18
Compression parallel to grain:	
Fiber stress at proportional limit.....	18
Maximum crushing strength.....	13
Compression perpendicular to grain: Fiber stress at proportional limit...	21
Hardness, end.....	13
Hardness, side.....	15

As an example, consider the modulus of rupture of red alder, when green, which is found from table 1 to be 6,500 pounds per square inch. The tabulation lists the probable variation for modulus of rupture as 12 percent. Twelve percent of 6,500 is 780; which when subtracted from and added to the average gives limits of 5,720 and 7,280 pounds per square inch. The probable variation is a value associated with the range within which one-half of the material of a species will fall. Consequently, it may be estimated that in red alder approximately one-half of the material would be between 5,720 and 7,280 pounds per square inch in modulus of rupture.

Considered in another way, there is 1 chance in 4 that the modulus of rupture of an individual specimen taken at random will be below 5,720 pounds per square inch, 1 chance in 4 that it will be above 7,280 pounds per square inch, and there are 2 chances in 4 that it will be between 5,720 and 7,280 pounds per square inch. The greater the probable variation, the greater the difference that may be expected in values, and the less the certainty with which the average values can be applied to individual pieces.

It is possible by means of mathematical tables, which are available in numerous texts on the theory of probability or statistical methods, to calculate the proportion of material associated with other ranges or that may be expected to be below or above any specified limit.

SELECTION FOR PROPERTIES

The fact that a piece of wood differs in properties from another of the same species often makes one more suitable than the other for a specific use. This suggests the possibility of selecting material of a quality best suited to meet specific use requirements. Fortunately, strength is frequently correlated with weight and to a lesser degree with other physical characteristics, and these relationships sometimes afford a basis for grading and selecting for strength.

Aside from weight, the other physical characteristics most usable for selecting on the basis of the strength of the clear wood are proportion of summer wood, rate of growth, hardness, and stiffness. Either visual or mechanical methods, or both, may be employed in appraising the properties. For example, selection may be made at the sawmill so that the heavier, and consequently stronger and harder, pieces go into structural timbers, flooring, or other uses for which the higher measure of these properties particularly adapt them, while the lighter pieces may preferably be used for such purposes as trim or heat insulation; or selection may be made at the lumber yard when material of high weight or that of low weight is desired. By means of selective methods the variability of wood, usually regarded as a liability, can within certain limits be made an asset. Selection on the basis of grades that limit defects is a common practice. Selection on the basis of quality of clear wood is less common, but is frequently very desirable, and offers possibility in the improvement of marketing practice. In any instance defects must of course be considered.

OTHER MECHANICAL PROPERTIES NOT INCLUDED IN TABLE 1

In addition to the data from the tests presented in table 1, information on certain other mechanical properties, principally tension parallel to grain and torsional properties is sometimes needed. A brief discussion of these properties, and of a special toughness test that may be used as an acceptance method follows.

TENSION PARALLEL TO GRAIN

In order to get reliable data on tension along the grain, special care must be exercised in preparing test specimens, and for this and other reasons little information on this property is available. Furthermore, the true tensile strength of wood along the grain is less important in design than other properties because it is practically impossible to devise attachments that permit the tensile strength of the full cross section of a wooden member to be developed.

Available results of tension tests show that generally the ultimate tensile strength considerably exceeds the modulus of rupture. Hence the modulus of rupture may be used as an estimate of the ultimate tensile strength parallel to grain for conditions where a uniform distribution of tensile stress obtains over the net cross section of a member. Uniform stress distribution, however, does not occur in the net tension area of a bolted joint, where it has been found that for softwoods the net tension area must be 80 percent, and for hardwoods 100 percent of the total bearing area under all the bolts (50) in the joint.

Table 4 presents the average results of tests in tension parallel to the grain on several species.

TABLE 4.—Results of tests to determine the ultimate tensile strength parallel to the grain

Species	Green				Air-dry			
	Moisture content	Tests	Specific gravity ¹	Ultimate tensile strength	Moisture content	Tests	Specific gravity ¹	Ultimate tensile strength
	Percent	Number		Lb. per sq. in.	Percent	Number		Lb. per sq. in.
Ash, white	—	1	0.535	16,150	—	—	—	—
Beech	53	1	.569	12,530	—	—	—	—
Cedar:								
Port Orford	34	34	.393	11,380	—	—	—	—
Western red	40	10	.300	6,200	8.8	7	0.323	7,130
Cypress, southern	78	15	.424	8,720	—	—	—	—
Douglas fir:								
Coast	24	48	.425	12,980	11.1	8	.444	13,830
“Inland Empire”	30	9	.409	9,380	10.2	1	.474	14,880
Fir:								
Noble	29	11	.353	14,750	10.2	9	.370	13,020
California red	168	14	.373	9,040	10.1	10	.385	10,750
White	48	9	.367	8,030	10.7	6	.382	10,450
Hemlock, western	67	20	.380	9,860	10.9	14	.400	9,820
Maple, sugar	48	5	.577	15,660	—	—	—	—
Oak, pin	80	3	.578	16,260	—	—	—	—
Pine:								
Loblolly	47	2	.446	11,570	11.6	1	.484	15,050
Ponderosa	69	11	.364	8,320	—	—	—	—
Poplar, balsam	106	3	.298	7,940	10.4	2	.351	12,160
Redwood	104	29	.377	9,780	10.7	33	.401	10,920
Spruce:								
Eastern ²	34	14	.366	13,650	11.7	13	.391	13,670
Sitka	40	17	.385	8,110	9.5	10	.406	11,150

¹ Based on weight when oven-dry and volume at test.² Exact species not known.

Figure 1 illustrates the form of specimen on which table 4 is based. Despite the reduced cross section in the central portion of the length the specimens sometimes fail by shear instead of in tension. Specimens that failed other than in tension are not included in the average values of table 4.

TORSIONAL PROPERTIES

The torsional strength of wood is little needed in design and, except for Sitka spruce, has not been studied extensively. Available results, however, indicate that the shearing stress at maximum torsional load, as calculated by the usual formulas, are approximately one-third greater than the values in table 1 for shearing strength parallel to the grain (51).

The effect of duration of stress on torsional strength is pronounced, being greater on the proportional limit than on the maximum torsional strength. With slowly applied loads the proportional limit may be less than 50 percent of the maximum, whereas with quickly applied loads the proportional limit may be 75 percent of the maximum load.

The modulus of rigidity or the modulus of elasticity of wood in shear is a combination of the component moduli along radial and tangential surfaces, and is influenced among other things by the position of the growth rings. The combined moduli are known as the mean modulus of rigidity, which for Sitka spruce is about one-fifteenth the modulus of elasticity along the grain. Scattered tests on other species show a range in values of the mean modulus of rigidity be-

tween one-fourteenth and one-eighteenth the modulus of elasticity along the grain. Until definite values are available for other species, a ratio of one-seventeenth appears conservative.

A third shear modulus that does not come in play in torsion about an axis parallel to the grain is associated with stresses that tend to roll the wood fibers by each other in a direction at right angles to the grain. This shearing modulus is extremely low but is of little importance in most design.

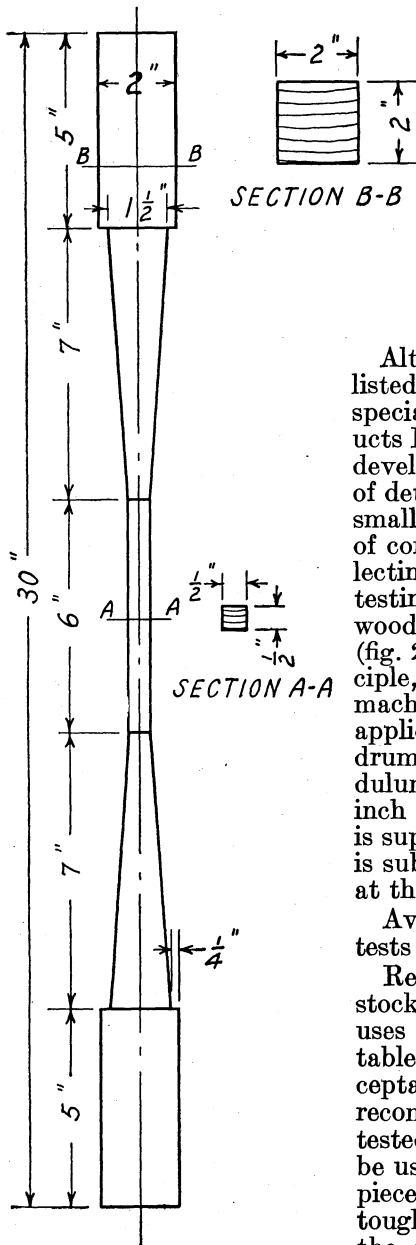
TOUGHNESS

Although a number of the properties listed in table 1 measure toughness, a special device known as the Forest Products Laboratory toughness machine was developed to provide a simple method of determining toughness from relatively small samples. The test affords a means of comparing species, and a basis for selecting stock of known properties by testing small specimens from pieces of wood intended for use. The machine (fig. 2) operates on the pendulum principle, but it differs from other pendulum machines in that the striking force is applied through a cable attached to a drum mounted on the axis of the pendulum. The specimen, which is $\frac{5}{8}$ by $\frac{5}{8}$ inch or $\frac{3}{4}$ by $\frac{3}{4}$ inch in cross section and is supported over an 8- or 10-inch span, is subjected to an impact bending force at the middle of its length (26).

Available average results of toughness tests are presented in table 5.

Recommended acceptance values for stock for aircraft and other high-class uses are presented for a few woods in table 6. In applying the test as an acceptance requirement for wood, it is recommended that four specimens be tested from the same piece as the part to be used is taken. To be acceptable, the piece (1) must either meet a minimum toughness requirement established for the species under consideration, or if within a certain tolerance below this minimum must pass in addition a minimum

FIGURE 1.—Details of tension-parallel-to-grain test specimen.



specific-gravity requirement; (2) must show a limited range in toughness values for specimens from the same piece, and (3) must pass careful visual inspection.

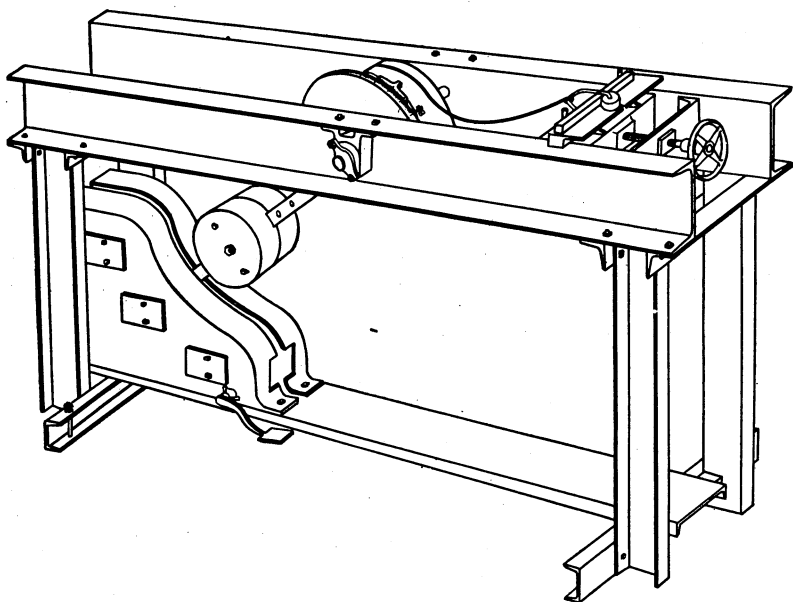


FIGURE 2.—Forest Products Laboratory toughness-testing machine.

TABLE 5.—Results of toughness tests

[Specimens $\frac{5}{8}$ by $\frac{5}{8}$ by 10 inches tested on an 8-inch span]

Species	Moisture content	Specific gravity (oven-dry based on volume at test)	Face to which load is applied			
			Radial		Tangential	
			Tests	Toughness	Tests	Toughness
	Percent		Number	In.-lb. per specimen	Number	In.-lb. per specimen
Birch:						
Alaska white.....	9.8	0.56	14	184	16	180
Yellow.....	11.9	.65	10	262	11	330
Catalpa, hardy.....	66	.40	13	180	19	181
	11.8	.41	18	104	17	124
Cedar:						
Alaska.....	10.4	.48	10	109	10	122
Western red.....	9.2	.33	21	45	21	70
Douglas fir.....	36	.43	51	82	59	112
	10	.46	36	86	36	151
	55	.31	44	36	44	52
Fir, corkbark.....	9.9	.31	28	36	30	51
Hemlock, eastern.....	12.3	.41	13	56	13	86
Hemlock, western.....	11.1	.38	31	60	34	86
Maple, sugar.....	13.8	.64	11	194	11	192
Oak, pin.....	11.5	.64	15	226	18	225
	86	.47	99	139	206	176
Pine, loblolly.....	11.9	.51	174	93	168	149
	90	.54	39	183	38	232
Pine, longleaf.....	13.3	.57	39	94	43	143
	88	.48	106	140	71	191
Pine, shortleaf.....	12.9	.50	75	77	71	120
	78	.55	72	185	73	238
Pine, slash.....	11.6	.59	67	109	63	167
	103	.39	101	58	96	106
Redwood.....	11.4	.39	104	49	99	75
Spruce, Sitka.....	9.8	.44	33	83	37	121

TABLE 6.—*Minimum acceptance requirements for aircraft woods based on tests¹ in the Forest Products Laboratory toughness machine*

Species of wood	Size of specimen	Span	Minimum average acceptable toughness		
			With specific gravity limitation		Without specific gravity limitation; minimum average toughness ³
			Minimum specific gravity ²	Minimum average toughness ³	
	<i>Inches</i>	<i>Inches</i>		<i>In.-lb. per specimen</i>	<i>In.-lb. per specimen</i>
White ash.....	5/8 by 5/8 by 10	8	0.56	150	175
Yellow birch.....	3/4 by 3/4 by 12	10	.58	225	260
Douglas fir.....	5/8 by 5/8 by 10	8	.45	95	115
White oak.....	3/4 by 3/4 by 12	10	.62	175	200
Sitka spruce.....	5/8 by 5/8 by 10	8	.36	75	90
Black walnut.....	3/4 by 3/4 by 12	10	.52	150	175

¹ Load applied to the tangential face of the specimen.² Based on weight and volume of oven-dry wood.³ These values are to be applied to the average of 4 or more test specimens, and the range in individual test values used in arriving at the average should not exceed 1 to 2½ among 4 specimens.

The procedure is simple and tests are made very rapidly. No calculation is necessary as the readings of the machine are readily converted into toughness values by the use of available tables. The procedure is further simplified by the fact that when testing dry wood the moisture condition of the specimen may be ignored, as tests have shown that toughness is affected but little by such moisture differences as may be commonly encountered.

The one essential in the application of the toughness test as an acceptance method, in addition to the necessary machine for making the tests, is a knowledge of the species with respect to minimum toughness requirements. The recommended values presented in table 6 have been established from tests made at the Forest Products Laboratory.

PROPERTIES OTHER THAN STRENGTH

RATING OF SPECIES IN SEVEN PROPERTIES

It has been mentioned that consideration of properties other than strength, weight, and shrinkage may be necessary in appraising the suitability of a wood for various uses (p. 3). Table 7 compares a number of species with respect to ease of kiln drying, ability to stay in place, workability, nail-holding ability, ease of gluing, resistance to decay, and ability to hold paint. The classifications are approximate, and only in some instances are they based on technical research. In others they are based on observation, experience, and general information. The ratings vary from 1 to 4 or 1 to 5, the lowest number indicating the best rating. For some other properties, such as acid resistance, sufficient information is not available to prepare even such a general classification of species. Information on properties other than those presented in this bulletin, insofar as available, may be obtained by writing the Forest Products Laboratory, Madison, Wis.

TABLE 7.—*Approximate comparison of 7 properties of commercial species of wood*

Key to classification of woods: Columns 2 and 4 represent a gradation of properties in the various woods from those which can be dried and worked with comparative ease (class 1) to those which present some difficulty in those respects (class 4). Column 3 represents a gradation from those woods which possess the greatest ability to stay in place under conditions of actual use (class 1) to those species which do not possess that ability to the same extent (classes 2, 3, 4, in the order named). Column 5 represents a gradation from those which possess the greatest nail-holding power but have the greatest tendency to split (which necessitates the use of smaller nails) to those having the least nail-holding ability but which are less likely to split. In column 6 the woods in class 1 are known to be used commercially in glued construction. Class 2 includes species about which little is known but which are not believed to be difficult to glue. Class 3 includes species which are known to require a little more attention in gluing than class 1 woods in order to get best results. Class 4 includes woods which are known to present real difficulties in gluing, and class 5 those species about which little is known but which it is believed would present some difficulties in view of their similarity to species of known properties. Column 7 presents comparative values for resistance to decay of heartwood when used under conditions that favor decay, class 1 being most decay-resistant. Column 8 represents a classification of softwood species with respect to ability to hold paint when used outside, class 1 species holding paint the most satisfactorily. Ability to hold paint is more important for outside than for inside use. The hardwood species are not commonly used for exterior work requiring painting and have not yet been classified]

Species	Ease of kilm drying ¹	Ability to stay in place	Work- ability	Nail- holding ability	Ease of gluing	Resist- ance to decay (heart- wood)	Ability to hold paint
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)
HARDWOODS							
Alder, red	2	3	2	-----	1	-----	-----
Ash:							
Black	3	4	3	-----	-----	-----	-----
White	2	3	4	2	3	4	-----
Aspen	2	3	2	5	2	5	-----
Basswood	2	3	2	5	1	5	-----
Beech	4	4	4	1	5	4	-----
Birch:							
Paper	2	4	3	-----	5	-----	-----
Sweet and yellow	2	4	-----	1	3	4	-----
Buckeye, yellow	-----	-----	2	-----	2	-----	-----
Butternut	2	2	-----	-----	2	-----	-----
Casara	-----	-----	4	-----	-----	-----	-----
Cherry:							
Black	4	3	3	-----	-----	-----	-----
Pin	3	3	2	-----	2	-----	-----
Chestnut	2	2	2	4	-----	1	-----
Chinquapin, golden	-----	-----	3	-----	1	-----	-----
Cottonwood:							
Black	3	4	2	5	1	5	-----
Eastern	2	4	2	5	1	5	-----
Dogwood	2	5	5	1	5	-----	-----
Elm:							
American	3	5	4	3	1	-----	-----
Rock	3	5	4	-----	-----	-----	-----
Gum:							
Black	3	5	4	-----	2	5	-----
Red	2, 4	4	4	3	1	3	-----
Hackberry	2	4	3	-----	2	-----	-----
Hickory, shagbark	4	5	5	1	4	-----	-----
Honey locust	4	2	4	1	5	2	-----
Hophornbeam	3	5	5	1	5	-----	-----
Laurel, California	5	3	4	-----	5	-----	-----
Madrone, Pacific	4	5	4	-----	5	-----	-----
Magnolia, cucumber	3	4	3	3	1	-----	-----
Maple:							
Bigleaf	3	3	3	-----	5	-----	-----
Red	3	3	4	-----	5	-----	-----
Sugar	3	4	4	1	3	4	-----
Oak:							
California black	4	3	4	-----	5	-----	-----
Red	3, 4, 5	4	4	1	3	4	-----
White	3, 4, 5	4	4	1	1	2	-----
Persimmon	4	4	5	1	4	-----	-----
Poplar, yellow	2	2	2	4	1	-----	-----
Sycamore	4	4	4	2	2	-----	-----
Walnut, black	4	2	3	-----	1	1	-----
Willow, black	2	3	2	-----	2	5	-----

¹ Softwoods are in general easier to dry than hardwoods. A softwood given the same numerical rating as a hardwood is, therefore, regarded as slightly easier to dry. These ratings are based on ease of removal of moisture without visible degrade but do not take into account susceptibility to reduction in strength in drying under high temperatures (57).

² 2 refers to sapwood and 4 to heartwood, known commercially as sap gum and red gum, respectively.

³ 4 refers to the upland type of oak and 5 to the lowland type of oak.

TABLE 7.—*Approximate comparison of 7 properties of commercial species of wood—*
Continued

Species	Ease of kiln drying	Ability to stay in place	Work- ability	Nail- holding ability	Ease of gluing	Resist- ance to decay (heart- wood)	Ability to hold paint
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)
SOFTWOODS							
Cedar:							
Alaska.....	1	1	3	-----	2	1	1
Incense.....	1	1	2	-----	2	1	1
Northern white.....	2	1	2	5	2	1	1
Port Orford.....	2	2	3	-----	2	1	1
Western red.....	⁴ 2, 3	1	2	5	2	1	1
Cypress, southern.....	3	2	3	3	2	1	1
Douglas fir.....	1	3	4	3	1	⁵ 2, 3	4
Fir:							
Alpine and balsam.....	1	-----	2	5	2	5	3
Grand, noble and white.....	1	3	2	5	2	5	3
Hemlock:							
Eastern.....	2	3	3	4	2	4	3
Western.....	2	3	3	3	1	4	3
Larch, western.....	3	3	4	3	2	3	4
Pine:							
Jack.....	1	3	3	4	2	-----	3
Lodgepole.....	1	2	2	5	2	-----	3
Northern white.....	2	1	1	4	1	-----	2
Norway.....	1	3	2	3	2	-----	3
Pitch.....	1	3	4	3	2	3	4
Ponderosa.....	1	2	2	4	1	-----	3
Southern yellow.....	1	3	4	2	1	⁵ 2, 3	4
Sugar.....	2	1	1	-----	1	-----	2
Western white.....	3	2	2	4	1	-----	2
Redwood.....	⁶ 3, 4	2	3	4	1	1	1
Spruce:							
Engelmann.....	2	2	2	5	2	4	3
Red and white.....	1	2	2	4	1	4	3
Sitka.....	1	2	2	4	1	4	3
Tamarack.....	2	3	4	-----	2	3	3

⁴ 2 refers to material from upper logs and 3 to material from butt logs which are generally susceptible to collapse.

⁵ 2 refers to dense Douglas fir and dense southern yellow pine.

⁶ 3 refers to material from upper logs and 4 to sinker stock from butt logs.

REQUIREMENTS FOR MOISTURE CONTENT OF WOOD IN BUILDINGS

The satisfactory use of lumber frequently depends upon the characteristics of the stock in its entirety, such as the size, kind, and number of defects as well as upon the properties of the clear wood, and may be further influenced by sizes available, degree of seasoning, and marketing practices. For most purposes seasoned is to be preferred to unseasoned stock, and for some uses, such as flooring, a definite degree of seasoning is essential for satisfactory results.

As an example of seasoning requirements, table 8 gives recommendations for desirable initial moisture content of lumber for various parts of dwellings (40).

While it is desirable that the average moisture content be near the value given in table 8, it is far more important that the moisture content of individual pieces of a lot be within the specified range.

TABLE 8.—*Recommended moisture-content values for various wood items at time of installation*

Use of lumber	Moisture content (percentage of weight of oven-dry wood) for—					
	Dry southwestern States		Damp southern coastal States		Remainder of the United States	
	Average	Range for individual pieces	Average	Range for individual pieces	Average	Range for individual pieces
Interior finishing woodwork and softwood flooring.....	6	4-9	11	8-13	8	5-10
Hardwood flooring.....	6	5-8	10	9-12	7	6-9
Sheathing, framing, siding, and exterior trim.....	9	7-12	12	9-14	12	9-14

MOISTURE CONTENT OF HEARTWOOD AND SAPWOOD

Average moisture-content values from green specimens consisting entirely of sapwood, or entirely of heartwood, are listed in table 9, for a number of species. These values show the variation in moisture content among species, the relative equality in moisture content of heartwood and sapwood in several hardwoods, and the large differences commonly existing in softwoods.

TABLE 9.—*Average moisture content for green heartwood and sapwood of 19 species*

Species	Trees	Average moisture content		Species	Trees	Average moisture content	
		Heartwood	Sapwood			Heartwood	Sapwood
HARDWOODS	Number	Percent	Percent	SOFTWOODS—contd.	Number	Percent	Percent
Ash, white.....	12	38	40	Hemlock:			
Beech.....	6	53	78	Eastern.....	5	58	119
Birch, yellow.....	9	68	71	Western.....	13	42	170
Elm, American.....	3	95	92	Pine:			
Gum, black.....	4	50	61	Loblolly.....	8	34	94
Maple:				Lodgepole.....	5	36	113
Silver.....	4	60	88	Longleaf.....	18	34	99
Sugar.....	6	58	67	Norway.....	4	31	135
SOFTWOODS	3	36	117	Ponderosa.....	4	40	145
Douglas fir.....	5	91	136	Shortleaf.....	8	34	108
Fir, lowland white.....	3	36	117	Spruce:			
		91	136	Engelmann.....	2	54	167
				Sitka.....	2	33	146

The moisture content of green heartwood and sapwood varies greatly among trees, and varies within the tree at different heights. The sapwood of the softwood species was consistently higher in moisture content than the heartwood, but some hardwood trees were found in which the heartwood was slightly higher than the sapwood. Because of the variation in moisture content of green wood, the values presented should not be taken as rigid averages for the species, but rather as indications of what may be expected.

The values in table 9 may be used in specific instances to estimate the average moisture content of logs. For example, if ponderosa pine logs in a shipment are observed to have 75 percent of sapwood.

by volume, the average moisture content would be estimated as $(0.75 \times 148) + (0.25 \times 40) = 121$ percent. Average moisture-content values computed in this way are likely to be more accurate in such instances and a better basis for computing weights than the average values listed for green material in column 7 of table 1 as these latter values may represent a quite different proportion of sapwood. The proportion of sapwood and heartwood in trees varies with the age of the stand and with growth conditions.

OTHER DATA ON SPECIFIC GRAVITY

In addition to the data on the specific gravity of the wood subjected to strength tests as presented in table 1, the Forest Products Laboratory has obtained for 14 common softwood species information based on sections of boards collected at sawmills in various parts of the United States (41). For a number of species the sampling from sawmills was more extensive than that used in obtaining specimens for strength tests, and the data are of interest on that account. In addition, data on heartwood and sapwood were segregated, whereas this has not been done with the data from the standard series of strength tests.

The principal data from the study of samples collected at sawmills are shown in table 10.

TABLE 10.—Comparison of specific gravity (oven-dry, based on volume when green) of mill-run samples with that of specimens used for mechanical tests

Species	Mill-run samples ¹					Specimens for mechanical tests		
	Specimens	Specific gravity heartwood and sapwood combined	Probable variation	Specific gravity heartwood	Specific gravity sapwood	Trees	Specimens	Specific gravity
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)	(9)
	Number		Percent			Number	Number	
Cypress, southern.....	377	0.38	10.0	0.39	0.36	26	479	0.42
Douglas fir:								
Washington and Oregon.....	2,764	.44	8.1	.44	.43	34	1,029	.45
"Inland Empire".....	176	.44	6.6			10	113	.41
Fir: White.....	1,187	1.33	17.1	1.33	1.33	10	278	.35
Hemlock, western.....	1,359	.39	6.8	.39	.39	18	689	.38
Larch, western.....	820	.45	7.5	.45	.43	13	214	.48
Pine:								
Longleaf.....	15,396	1.52	110.3	1.57	1.48	34	806	.55
Northern white.....	386	.34	5.7	.35	.34	18	299	.34
Norway.....	121	.39	6.6	.39	.38	5	126	.44
Ponderosa.....	1,876	.37	8.7	.38	1.36	31	579	.38
Shortleaf.....	4,357	1.47	18.5	1.51	1.46	22	1,190	.49
Sugar.....	965	.33	6.7	.33	.32	9	191	.35
Western white.....	1,178	.36	5.9	.36	.36	14	211	.36
Redwood.....	585	.36	9.7	.36		16	564	.39
Spruce, Sitka.....	658	.36	6.9	.36	.33	25	1,392	.37

¹ The mill-run specimens were classified according to commercial species designations of the lumber and not according to botanical classification, although in most instances the two are approximately the same. The southern pines are the principal exception as there is no known method of distinguishing the several species botanically from the wood alone, and hence species are mixed in the commercial designations. The samples used for mechanical tests were taken from trees identified botanically in the woods.

² Values for shortleaf and loblolly pine combined.

It was not possible in all cases to identify these samples as to species. Consequently, the data are classified according to commercial designation of the lumber and not according to exact species. However, except for those names to which footnote 1 is appended, the designations are probably the correct species names.

Table 10 shows for comparison values of specific gravity taken from column 8 of table 1. In general, the values in columns 3 and 9 of table 10 are in reasonable agreement although with but two exceptions (western hemlock and Douglas fir from the "Inland Empire" region) those of column 9 are the same or higher. Other studies have disclosed considerable variation in Douglas fir in the "Inland Empire" region and in this instance the operation of chance in sampling might readily lead to the difference between the values in columns 3 and 9. Further reasons for differences include the effect of position of material in the tree, and the fact that the methods of determining specific gravity were not quite identical.

The specimens used for standard strength tests (column 9) were taken mainly from the top 4 feet of 16-foot butt logs, whereas the samples collected at the mill (column 3) represent mixed material in which wood from all parts of the tree may be included. Because in many species the wood near the butt of the tree is heavier than that from the upper portions of the trunk, the specific-gravity values in column 9 would in general be expected to be slightly higher than those representing mixed material. An example of this kind is afforded by western larch. The butt portions of western larch trees contain large quantities of extractives which increase the weight considerably and as much as 12 feet of the portion immediately above the stump is often discarded because the extra weight makes handling of the logs difficult. On the other hand, Sitka spruce is an example of a species whose specific gravity varied but little with height in tree.

In general, the differences between the values listed in columns 3 and 9 are not greater than are to be expected from the causes just discussed combined with the effects of chance in sampling.

Table 10 also lists some data on the specific gravity of heartwood and sapwood, and the probable variation in specific gravity of the mill samples. It may be noted that the specific gravity of heartwood is in general slightly higher than that of sapwood. One reason for this higher value is the greater quantity of extractives (p. 47) in the heartwood.

FACTORS AFFECTING THE STRENGTH OF WOOD

The numerical data presented in table 1 were, as has been shown, derived from tests of small clear specimens taken from a specific part of the tree and tested under a standardized procedure.

Most uses of wood involve pieces differing in size and shape from those tested; clear material may not be available or may be more expensive than a contemplated use justifies; conditions of use may differ radically from standard test conditions; time limitations may require kiln drying; need for permanence may point to preservative treatment; the user may have erroneous concepts of the rate of growth as a criterion of suitability or of the comparative strength of heartwood or sapwood; he may hesitate to accept material from dead trees, or from turpented trees. These and many other questions

that may arise require consideration in order to properly interpret the numerical data and adapt it to specific uses of wood. A knowledge of factors affecting strength is thus essential to the interpretation of test data and is of value in the purchase of lumber, in the preparation of specifications covering the use of timber in engineering structures, and in the selection, classification, and use of wood for manufactured products. A brief discussion of various factors affecting the strength of wood is accordingly presented.

RELATION OF PROPERTIES TO STRUCTURE

Wood is a heterogeneous material consisting essentially of fibers of cellulose cemented together by lignin. The fibers, which taper toward the ends, are about one-eighth of an inch long in softwoods, one twenty-fourth of an inch in hardwoods, with a central diameter about one hundredth of the length. They are hollow, their longer dimension running lengthwise of the tree. In the softwoods the fibers act as water conductors. In the hardwoods a limited number of fibers act similarly and there are also relatively large pores or vessels which serve the same function. Besides these vertical fibers which comprise the principal part of the wood, all woods except palms and yuccas contain horizontal strips of cells known as rays or wood rays which are oriented radially and are an important part of the tree's food transfer and storage system. Among different species the rays differ widely in their size and prevalence.

The shape, size, and arrangement of the fibers, the presence of the wood rays, and the layer effect of spring and summer wood make wood a nonisotropic material with large differences in the properties along and across the grain (19, 43). Certain of the properties across the grain may be but a small fraction of the like properties along the grain. In air-dry Sitka spruce, for instance, the modulus of elasticity across the grain, may be only one one-hundred-and-fiftieth as great as when the load is parallel to the grain (10,200 pounds per square inch for 45° angle (p. 35) as compared to 1,570,000 pounds per square inch in column 16, table 1). There is an increasing need for information which will permit a closer correlation of structure and properties. Such information is of value in accounting for and remedying and preventing certain difficulties in the use of wood, and for giving a more precise basis for timber design through a better knowledge of properties and stress distribution.

TABLE 11.—Average results of tests showing influence of position of growth rings on the mechanical properties of Sitka spruce, Douglas fir, and loblolly pine

Properties	Sitka spruce				Douglas fir				Loblolly pine, green	
	Green		Air-dried		Green		Kiln-dried		Position A ¹	Position B ¹
	Position A ¹	Position B ¹	Position A ¹	Position B ¹	Position A ¹	Position B ¹	Position A ¹	Position B ¹		
Static bending:										
Moisture.....percent ²	45.2	45.3	12.2	12.2	30.6	29.4	11.9	11.9	26.0	25.8
Specific gravity ³341	.343	.370	.372	.427	.431	.455	.459	.599	.599
Fiber stress at proportional limit.....pounds per square inch	3,160	3,150	5,800	5,900	4,510	4,700	7,800	8,120	4,820	4,540
Fiber stress at maximum load.....pounds per square inch	4,890	4,960	8,470	8,450	7,280	7,470	10,630	10,860	9,750	9,740
Modulus of elasticity.....1,000 pounds per square inch	1,104	1,124	1,370	1,374	1,475	1,480	1,723	1,713	1,398	1,398
Work to proportional limit.....inch-pounds per cubic inch	.52	.52	1.46	1.49	.81	.86	2.03	2.22	1.00	.90
Work to maximum load.....inch-pounds per cubic inch	5.2	5.6	7.5	7.5	6.3	7.6	7.4	7.5		
Work, total.....inch-pounds per cubic inch	15.8	14.4			15.0	11.9				
Impact bending, 50-pound hammer:										
Moisture.....percent ²	45.8	44.4	12.7	12.5	30.4	30.3	10.7	11.0		
Specific gravity ³343	.350	.372	.378	.422	.431	.457	.460		
Fiber stress at proportional limit.....pounds per square inch	7,870	7,860	10,150	9,900	8,870	9,450	12,550	12,370		
Modulus of elasticity.....1,000 pounds per square inch	1,277	1,274	1,618	1,662	1,480	1,729	2,140	2,110		
Work to proportional limit.....inch-pounds per cubic inch	2.7	2.7	3.7	3.4	3.0	2.9	4.2	4.1		
Height of drop causing complete failure.....inches	20	20	21.0	20.9	20.4	18.8	23.4	23.1		
Compression parallel to grain:										
Moisture.....percent ²	45.2	46.4	12.7	12.7	29.6	29.2	10.2	10.4		
Specific gravity ³339	.363	.370	.370	.428	.419	.451	.459		
Rings per inch.....	14.5	15.7	6.5	7.6	16.3	15.9	20.4	23.0	7.0	7.5
Fiber stress at maximum load.....pounds per square inch	2,220	2,210	4,490	4,670	3,810	3,730	7,230	7,250	4,680	4,650
Hardness ⁴ :										
End.....pounds	357	357	682	701	440	440	713	713		
Side.....pounds	289	283	436	462	452	446	700	657		
Compression perpendicular to grain: Fiber stress at proportional limit.....pounds per square inch	227	227	548	582	455	496	609	667		
Shear parallel to grain: Shearing strength.....pounds per square inch	713	668	184	1,202	883	833	1,209	1,266		
Cleavage: Cleavage strength.....pounds per square inch	122	94	242	189	133	136	163	163		
Tension at right angles to grain: Tensile strength.....pounds per square inch	208	130	466	357	179	165	255	307		

¹ Position A and B refer to placement of growth rings with respect to directions of application of load, as illustrated in fig. 3.² Percent moisture based on weight of oven-dry wood.³ Specific gravity based on weight when oven-dry and volume at test.⁴ Adjusted to drop for 2- by 2-inch cross section.⁵ Load required to imbed a 0.444-inch ball to $\frac{1}{2}$ its diameter.

POSITION OF GROWTH RINGS

In the sawing of lumber and timber the position of the growth rings may be made to assume different directions with respect to the surfaces of the piece. Any effect of position of growth rings on the properties thus assumes practical significance.

Table 11 presents, for three species, data on clear specimens 2 by 2 inches in cross-section tested to determine the effect of two positions of growth rings on the strength properties (fig. 3). It may be noted

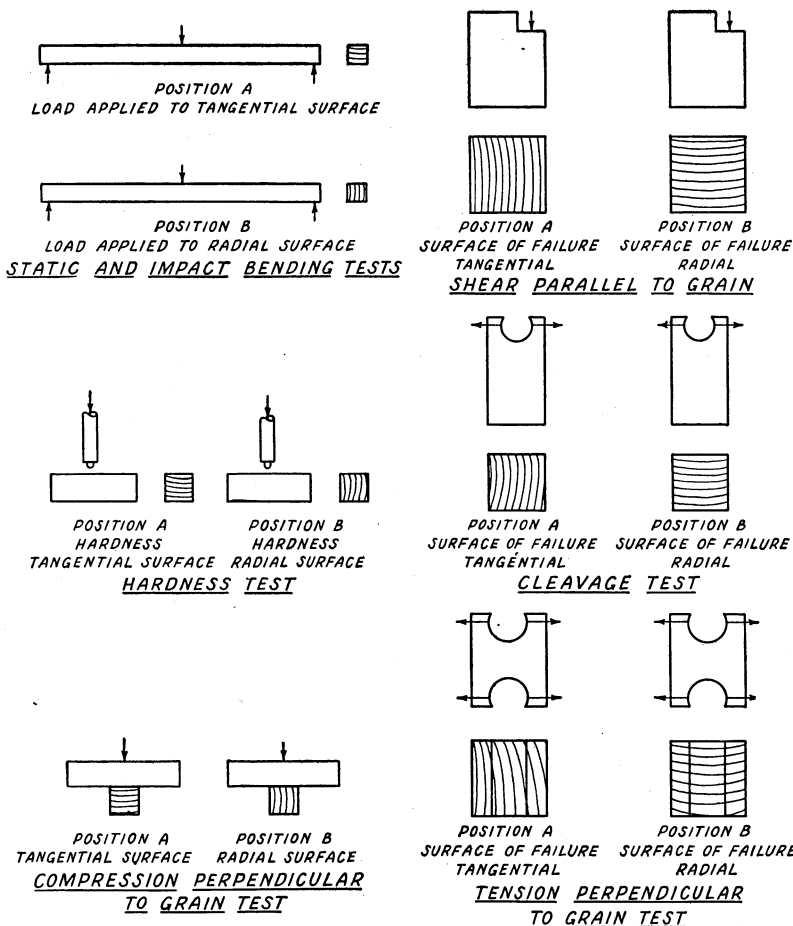


FIGURE 3.—Sketch of standard mechanical tests which afford choice in placement of growth rings with respect to direction of application of load.

that the bending tests, which were on specimens 30 inches long, show little difference in the properties listed, whether the rings as viewed on the end of a piece are vertical or horizontal. Some of the other properties listed, however, show significant differences between the two placements of rings resulting not only from the difference in structure due to the rings themselves, but also the difference orientation of the other minute structural elements of the wood with respect to the direction of stress.

The values from the tests in compression parallel to grain, which were unaffected by the placement of growth rings because the specimens were square, together with the data on specific gravity and rings per inch, show that the wood representing position A was practically identical in quality with that representing position B.

There are many further effects of stratified structure on properties, as evidenced by the growth-ring position, not brought out by results of standard tests. An outstanding example is in compression perpendicular to grain. The results of some preliminary determinations of modulus of elasticity in compression perpendicular to grain are presented in table 12.

TABLE 12.—*Modulus of elasticity in compression perpendicular to grain as influenced by direction of growth rings*

[Specimens $1\frac{1}{2}$ by $1\frac{1}{2}$ by 6 inches loaded on the $1\frac{1}{2}$ by $1\frac{1}{2}$ -inch face]

Species	Specific gravity	Moisture content	Modulus of elasticity when the growth rings with respect to the applied load are at an angle of—				
			0°	22½°	45°	67½°	90°
		Percent	Lb. per sq. in.	Lb. per sq. in.	Lb. per sq. in.	Lb. per sq. in.	Lb. per sq. in.
Redwood.....	0.34	11	78,400	28,600	17,100	27,900	106,600
Douglas fir.....	.45	37	58,200	21,400	12,200	26,800	85,400
Spruce, Sitka.....	.42	13	62,400	18,100	10,200	22,400	110,300
Hemlock, western.....	.44	88	45,400	11,600	8,300	14,100	71,500
Birch, yellow.....	.68	63	48,000	39,900	34,000	55,900	81,200
Do.....	.67	13	106,400	82,300	80,800	113,200	158,000
Oak, red.....	.56	119	66,200	57,800	59,700	77,400	110,300

It may be noted that there is a large difference in the modulus of elasticity in compression perpendicular to grain with position of rings, amounting to as much as 11 to 1 in Sitka spruce between material with the rings at 90° to the direction of the load and that with rings at 45°. Proportional limit and maximum crushing strength perpendicular to grain are also affected by ring position, although the indications are that the differences are considerably less than for modulus of elasticity.

In the Forest Products Laboratory toughness test, in which specimens one-half to three-fourths inch square and 10 to 12 inches long are used, some marked differences have been found, depending on whether the load is applied to the radial or tangential face. In some species average differences of as much as 50 percent of the lesser values were noted (table 5), the higher values resulting when the load was applied to the tangential face. These results as compared with those of table 11, indicate that size of specimen may be an important factor in the influence of position of rings.

SPRING WOOD AND SUMMER WOOD PLACEMENT EFFECT

Significant differences with ring placement may become evident in properties not appreciably affected in 2- by 2-inch pieces when specimens of smaller size are tested. This was demonstrated by static-bending tests on 1- by 1- by 16-inch specimens of southern yellow pine and Douglas fir containing large amounts of summer wood, modulus of elasticity being determined (without stressing the specimen beyond the proportional limit) by placing the specimen with the

rings horizontal and then vertical. The modulus of elasticity of specimens with summer wood layers on the two faces averaged 12 percent higher for southern yellow pine, and 16 percent higher for Douglas fir with the rings horizontal (load applied to tangential face) than with the rings vertical (load applied to radial face). On the other hand, with specimens having spring wood layers on two faces, the modulus of elasticity when the rings were horizontal (load applied to the tangential face) averaged 9 percent lower than when the rings were vertical (load applied to radial face) for southern yellow pine and 13 percent lower for Douglas fir. These differences, it should be observed, represent a spring wood and summer wood placement effect rather than a pure growth-ring placement effect. Theoretical calculations based on the assumption of widely different properties in spring wood and summer wood check these observed values closely.

SPECIES OF WOOD

Some species of wood differ greatly from others in their average specific gravity, strength, and other properties. Certain species, such as hickory and ash, excel in toughness and shock-resisting ability. Others, such as southern yellow pine and Douglas fir, are high in bending strength and stiffness for their weight. Still other species are soft, uniform in texture, and easy to work. Such differences permit a choice of species to meet the requirements of diverse and exacting uses. Comparative data on important properties are presented for 164 species of wood in table 1.

The average differences in strength properties between species ordinarily competing for the same use are often quite small. Nevertheless, there may be decided differences in structure and in behavior with respect to moisture relations, drying, and manufacturing characteristics which make it necessary to vary the handling procedure or manufacturing practice to best suit the wood under consideration. In this way as satisfactory service may be obtained from species not generally regarded so suitable for a use as from species that give a good account of themselves regardless of care or of lack of care in their handling.

SPECIFIC GRAVITY (OR DENSITY) AS RELATED TO STRENGTH

The substance of which wood is composed is actually heavier than water, its specific gravity being about 1.5 regardless of the species of wood. In spite of the fact that the actual wood substance is heavier than water, the dry wood of most species floats in water, and it is thus evident that a considerable portion of the volume of a piece of wood is occupied by cell cavities and pores. The specific gravity of a piece of dry wood is thus an excellent index of the amount of wood substance it contains and hence is an index of the strength properties.

The relations between specific gravity and other properties of wood may be considered on the basis of (1) different species and (2) different pieces of the same species.

SPECIFIC GRAVITY-STRENGTH RELATIONSHIP AMONG SPECIES

The general relation of specific gravity to strength is illustrated by two widely different woods, mastic, a very heavy species growing in Florida, and balsa, a very light species from Central America. Compression-parallel-to-grain tests on green material gave the results in

table 13, and show that mastic with average specific gravity 9 times as great as that of balsa was 9 times as high in crushing strength along the grain. Weight for weight, the crushing strength parallel to grain of these diverse species are substantially equal.

TABLE 13.—Comparison of the specific gravity and the maximum crushing strength of mastic and balsa

Species	Specific gravity, based on weight and volume of wood when oven dry	Maximum crushing strength parallel to grain	Specific strength (column 3÷column 2)
(1)	(2)	(3)	(4)
Mastic.....	1.03	<i>Lb. per sq. in.</i> 5,880	5,710
Balsa.....	.11	644	5,850

The average specific gravity-strength relations based on 163 species of hardwoods and softwoods show that some properties, such as maximum crushing strength parallel to grain, increase approximately in proportion to the increase in specific gravity, whereas others increase more rapidly. Modulus of rupture, for instance, varies from one species to another as the $1\frac{1}{4}$ power of specific gravity. Other properties are related to specific gravity by equations of still higher powers; for example, the exponent of specific gravity for relation to hardness is $2\frac{1}{4}$. It is evident, therefore, that small differences in specific gravity may result in large differences in certain strength properties. For example, one species twice as high in specific gravity as another has $4\frac{3}{4}$ times the hardness.

Approximate average relations of specific gravity to strength properties among different species are given in table 14 (38).

TABLE 14.—Specific gravity-strength relations among different species ¹

Property	Unit	Moisture condition	
		Green	Air-dry (12-percent moisture content)
Static bending:			
Fiber stress at proportional limit.....	Pounds per square inch.....	10200 <i>G</i> ^{1.25}	16700 <i>G</i> ^{1.25}
Modulus of rupture.....	do.....	17600 <i>G</i> ^{1.25}	25700 <i>G</i> ^{1.25}
Work to maximum load.....	Inch-pounds per cubic inch.....	35.6 <i>G</i> ^{1.75}	32.4 <i>G</i> ^{1.75}
Total work.....	do.....	103 <i>G</i> ²	72.7 <i>G</i> ²
Modulus of elasticity.....	1,000 pounds per square inch.....	2380 <i>G</i>	2800 <i>G</i>
Impact bending:			
Fiber stress at proportional limit.....	Pounds per square inch.....	23700 <i>G</i> ^{1.25}	31200 <i>G</i> ^{1.25}
Modulus of elasticity.....	1,000 pounds per square inch.....	2940 <i>G</i>	3380 <i>G</i>
Height of drop.....	Inches.....	114 <i>G</i> ^{1.75}	94.6 <i>G</i> ^{1.75}
Compression parallel to grain:			
Fiber stress at proportional limit.....	Pounds per square inch.....	5250 <i>G</i>	8750 <i>G</i>
Maximum crushing strength.....	do.....	6730 <i>G</i>	12300 <i>G</i>
Modulus of elasticity.....	1,000 pounds per square inch.....	2910 <i>G</i>	3380 <i>G</i>
Compression perpendicular to grain: Fiber stress at proportional limit.....	Pounds per square inch.....	3000 <i>G</i> ^{2.25}	4630 <i>G</i> ^{2.25}
Hardness:			
End.....	Pounds.....	3740 <i>G</i> ^{2.25}	4800 <i>G</i> ^{2.25}
Radial.....	do.....	3380 <i>G</i> ^{2.25}	3720 <i>G</i> ^{2.25}
Tangential.....	do.....	3460 <i>G</i> ^{2.25}	3820 <i>G</i> ^{2.25}

¹ The values listed in this table are to be read as equations, for example: Modulus of rupture for green material = 17600*G*^{1.25}, where *G* represents the specific gravity, oven-dry, based on volume at moisture condition indicated.

Some species of wood contain relatively large amounts of resins, gums, and other extractives, which add to the weight but do not contribute so much to the strength as would a like amount of wood substance (23). In addition, species vary in the structural arrangement of their fibers. For these reasons, two species which average the same in specific gravity may exhibit different strength character-

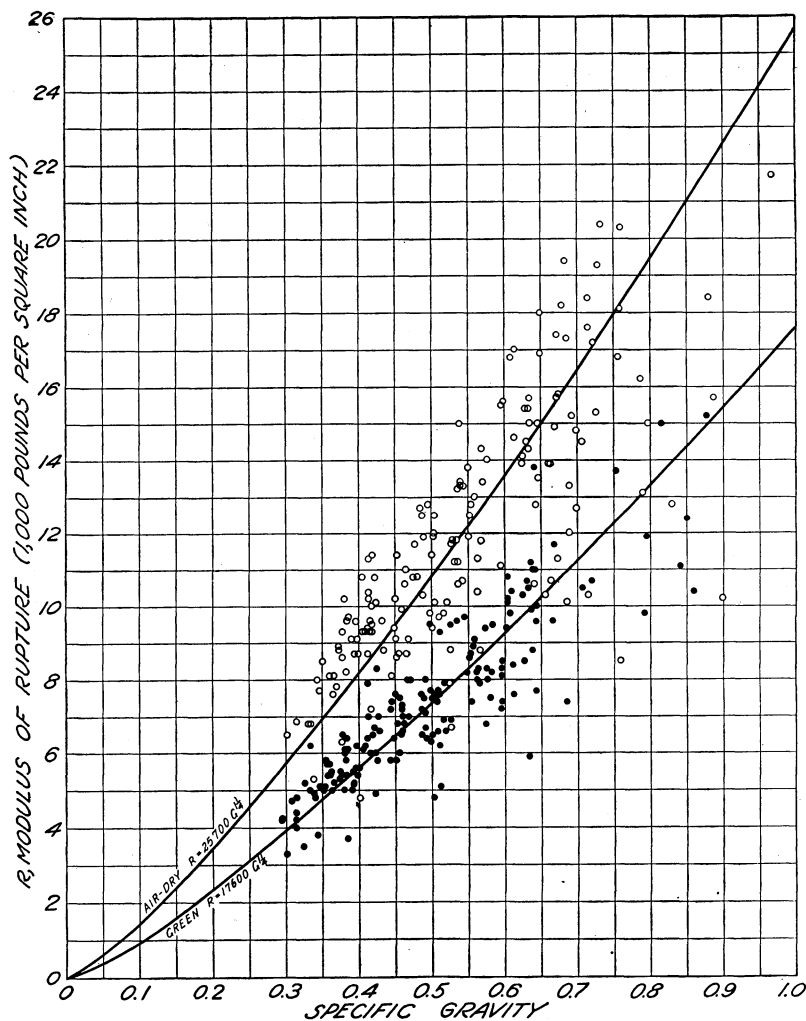


FIGURE 4.—Relation of modulus of rupture to specific gravity for green and air-dry material of various species.

istics. This fact is illustrated by the scattering of the points in figure 4. The values for Douglas fir (coast type) and red gum in table 1 illustrate an extreme example of variations from the average density-strength relations among species. Although these woods are about equal in weight per unit volume when dry, Douglas fir averages 39 percent higher in compressive strength but considerably lower than red gum in shock resistance.

It is true, likewise, that some species of wood are equal in some respects to others of higher density. Douglas fir (coast type), although its density is but three-fourths that of commercial white oak, is about equal to the oak in bending and compressive strengths, and excels it in stiffness. However, the oak averages much higher than Douglas fir in hardness and shock resistance. Hence the specific gravity relationships among species represent general trends and not uniform laws. Departure of a species from the general relationship often indicates some exceptional characteristic which makes this species particularly desirable for certain use requirements.

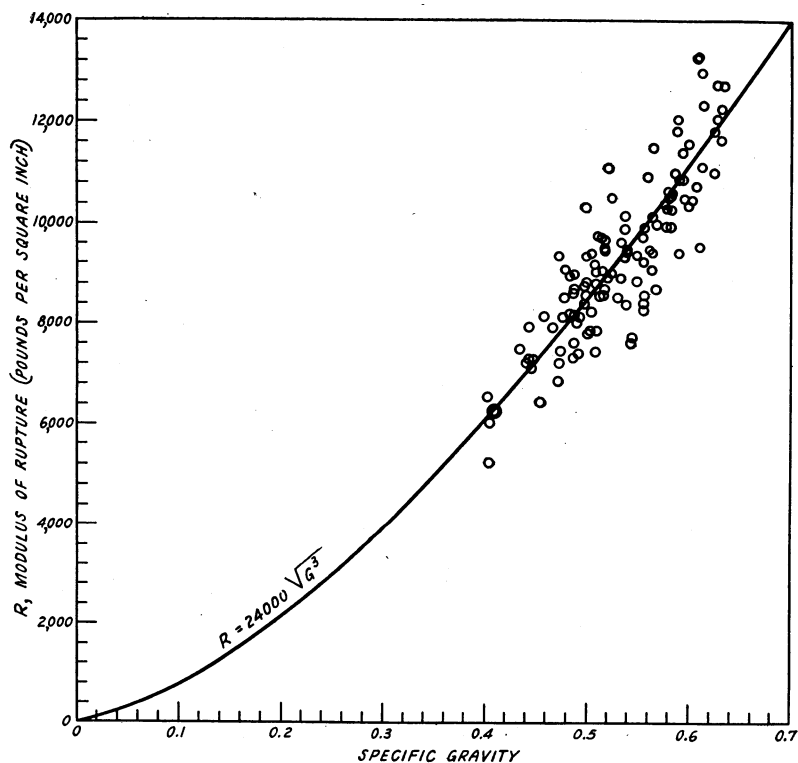


FIGURE 5.—Relation of modulus of rupture of white ash (green) to specific gravity.

SPECIFIC GRAVITY-STRENGTH RELATIONSHIP AMONG INDIVIDUAL PIECES OF A SPECIES

While a general relationship thus exists between the specific gravities and strength properties among different species, specific gravity affords a still better index of strength within a species. The heaviest pieces of any species of wood are generally 2 to 3 times as high in specific gravity as the lighter ones of the species, and are correspondingly stronger. The relationship of pieces within a species is usually represented by a power of specific gravity slightly higher than that representing average values for different species. Furthermore, departures from the average relationship are less marked. Figure 5 illustrates the relation between the specific gravity and the modulus of rupture for individual pieces of white ash.

THE TREE IN RELATION TO STRENGTH

HEIGHT IN TREE

The wood from the butt of the trees of many species is higher in specific gravity than that from higher positions. Since wood of higher specific gravity usually has the better mechanical properties regardless of position in tree, the height in tree ordinarily needs to be taken into account only in connection with other factors (fig. 6). Sometimes, however, notably in hickory and ash, material from the

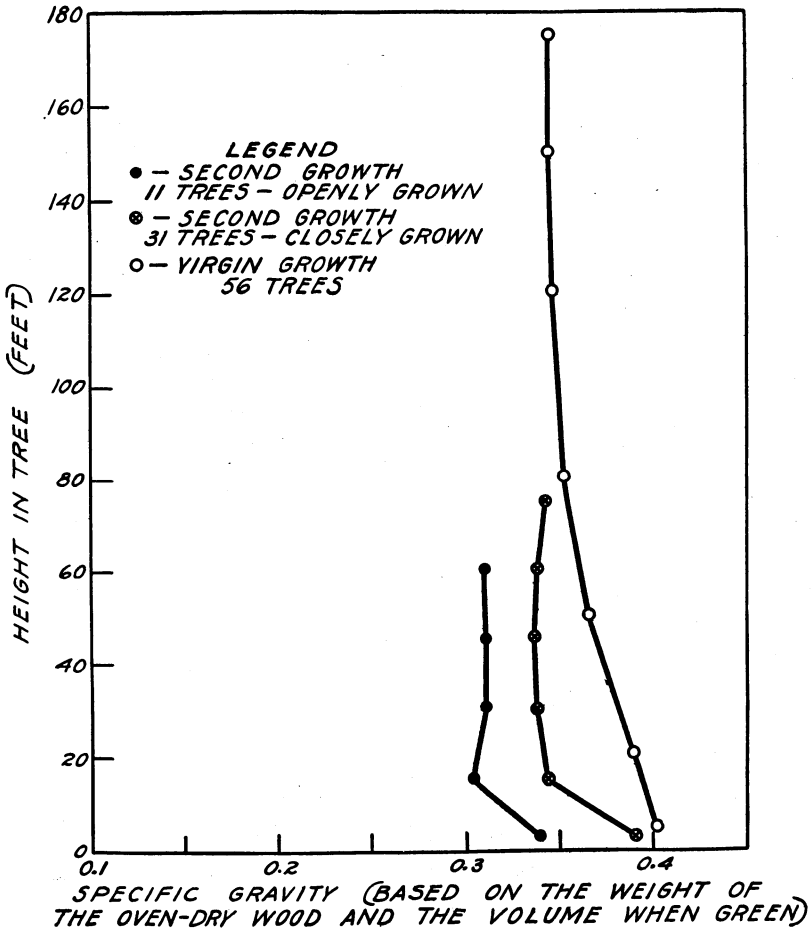


FIGURE 6.—Variation in specific gravity with height for virgin-growth and second-growth redwood.

butt shows superior toughness or shock resistance for its weight. On the other hand, wood from the swelled butts of certain swamp-grown hardwoods is usually low in specific gravity and of inferior strength properties, whereas that above the swelled butt is more nearly normal.

POSITION IN CROSS SECTION OF TREE

Position in cross section is not in itself a reliable guide to the strength of the wood. As in other instances, the wood of highest specific gravity has the best strength properties.

In coniferous species wood near the pith of the tree is often of very rapid growth and low specific gravity, whereas that in the outer part of overmature trees is of slow growth and likewise of medium to low specific gravity, the wood of highest strength most frequently being that in the intermediate zone. The many factors influencing growth, however, result in wide diversity of wood formation and preclude the drawing of rigid general rules (fig. 7).

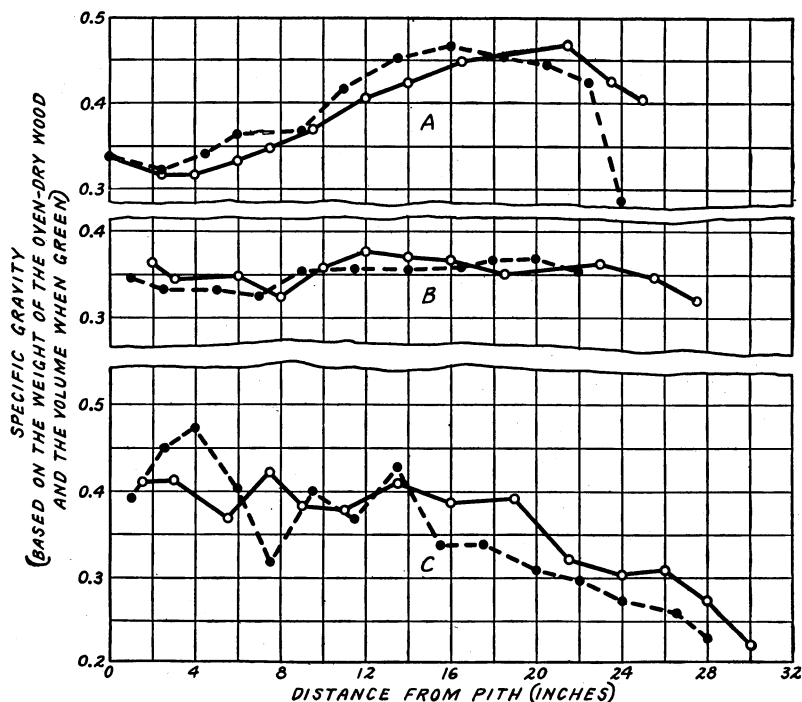


FIGURE 7.—Variation of specific gravity with distance from the pith for three different virgin-growth red-wood trees at a height of 20 to 30 feet above the ground, showing (A) increase in specific gravity with distance from pith for greater part of diameter (B) little or no change, and (C) decrease. Solid and dotted lines represent specimens taken from opposite sides of the pith.

In the hardwoods, wood of high density may be produced at any stage in the life of the tree, depending on the growth conditions at the particular time the wood is formed (39). In some hickory trees, for instance, wood of high density is found near the pith, and in others farther out in the cross section.

HEARTWOOD AND SAPWOOD

The trunk and principal branches of a tree consist of a central portion called heartwood surrounded by a layer of sapwood.

All wood is formed as sapwood and as the growth of the tree proceeds the inner portion becomes heartwood. In most species the transformation is accompanied by an infiltration of various substances that cause a change in color and in some species by the plugging up of the pores with a frothlike growth, known as "tyloses" (13).

In the many tests which have been made on the various species of wood, no effect upon the mechanical properties of most species due to change from sapwood to heartwood has been found. In general the conditions of growth that prevail when wood is first formed determine

its strength properties and whether heartwood or sapwood is the stronger depends on those conditions. Consequently, in one tree the heartwood may excel and in another of the same species the sapwood. Thus the heartwood of the southern pines and of Douglas fir is not, as has often been supposed to be the case, intrinsically stronger than the sapwood. The sapwood of hickory or ash may be either superior or inferior to the heartwood for handles (8). In some instances, however, as shown in the discussion of extractives, heartwood and sapwood do differ essentially in strength properties.

The heartwood of many species is of much darker color than the sapwood. In numerous species, on the other hand, the color difference is nonexistent or very slight. The sapwood of all species is lacking in resistance to decay and rapidly loses its strength if exposed to conditions favoring the growth of decay-producing organisms. The heartwood of some species is very resistant to decay, while that of other species is readily attacked.

Sapwood is more permeable to liquids than heartwood, and hence is desirable in wood that is to be impregnated or treated to increase its resistance to decay, fire, or insect attack.

VARIATION AMONG TREES

In addition to the variation of wood from one part to another of the same tree, there are considerable differences among trees of a species including those that grow side by side. The magnitude of these variations is illustrated by data on redwood. Of 57 virgin-growth trees examined in lots of 4 to 6 from each of 12 different localities throughout the range, the greatest observed difference in average specific gravity between individual trees from a single locality was 25 percent, based on the heaviest tree, whereas considering the entire range the greatest difference between individual trees was only 30 percent. The two trees representing the extremes found in the entire range were from the same county. These data indicate that the growth conditions affecting individual trees within a single site, and perhaps inherent differences in strains or types of trees, are of much greater importance in causing variations in specific gravity than geographical location within the normal range of growth of the species.

Probable variation of random tree from average for species

Property:	Percent
Specific gravity based on volume when green.....	4
Static bending:	
Fiber stress at proportional limit.....	9
Modulus of rupture.....	7
Modulus of elasticity.....	9
Work to maximum load.....	15
Impact bending:	
Fiber stress at proportional limit.....	8
Work to proportional limit.....	12
Height of drop.....	13
Compression parallel to grain:	
Fiber stress at proportional limit.....	12
Crushing strength.....	7
Compression perpendicular to grain: Fiber stress at proportional limit.....	14
Hardness:	
End.....	10
Side.....	9
Shearing strength parallel to grain.....	7
Tension perpendicular to grain.....	12

The preceding tabulation presents an estimate of the probable variation of a random tree from the average for a species, for a number of physical and mechanical properties. The values are general figures derived from a number of species.

LOCALITY OF GROWTH

In considering the causes of variations in properties of wood, it may first be noted that many factors affect the growth of trees. Such features of environment as soil, soil moisture, climatic conditions, and competition for light and food, vary widely within small areas, and are subject to further variation from one period to another during the life of the tree. Their effect is seemingly of greater importance than geographical location within the normal range of a species. This is indicated by the finding of significant differences in strength properties between samples from adjacent areas, among trees grown within a few yards of each other and between the inner and outer portions of the same tree and the observation that samples from widely separated regions may be very similar (29). This is illustrated by the discussion of redwood on page 42.

A further example is noted in Sitka spruce. Samples from two localities in Oregon show an average difference of 12 percent in specific gravity and 20 percent or more in modulus of rupture. In contrast, samples from near Ketchikan, Alaska, tested in a green condition, average the same in specific gravity as samples from near Portland, Oreg., and the difference in modulus of rupture was only a few percent. These and similar observations lead to the general conclusion that, in the absence of specific data concerning a given lot of material, average data for the species is a more reliable estimate of the strength properties of that lot than data on samples from adjacent localities or from sites that appear to be the same. However, there may be differences apparent in the grade and quality of wood from different stands, especially old-growth and second-growth stands in which prevalence of defects, seasoning characteristics, and the like, are sufficient in importance to justify marketing preferences.

The whole problem of the effect of region, site, and conditions of stand on wood properties is an exceedingly complicated one, and sufficient data are not available nor has sufficient study been made to attempt a final appraisal.

A few instances of significant differences in the properties of a species grown in different regions have been noted. For example, Douglas fir grows to larger size in the moist region of the Pacific Northwest than in the drier Rocky Mountain States, and the wood from the former region averages somewhat higher in specific gravity and strength properties than the latter. On the other hand, weight for weight, the wood from the two regions has the same strength, and pieces of Douglas fir from the Rocky Mountain region may be selected which are higher in properties than unselected Douglas fir from the Pacific Northwest.

Another significant effect of growth conditions on properties is that resulting from inundation. Some of the hardwoods, notably ash and tupelo gum (44) grown in the overflow bottom lands of the lower Mississippi basin develop swelled butts, the wood in which although of rapid growth and relatively good appearance, is low in specific gravity and poor in mechanical properties compared to average mate-

rial of the species. The characteristics of the wood from these swelled butts are so unlike those of the normal wood of the species that it cannot be satisfactorily employed for the same uses. Wood above this butt swell usually is normal in properties. Hence one utilization problem is the proper classification of such stock according to its properties and potential uses.

RATE OF GROWTH

Rate of growth as indicated by the width of the annual rings is of some assistance in appraising the physical and mechanical properties of wood, but it cannot be regarded as an efficient criterion for selection. Density or specific gravity, as explained on page 36, is a much more reliable criterion of strength. In any species, wood of excellent mechanical properties may vary considerably in rate of growth, but such material will quite consistently be of good density.

Among the ring-porous hardwoods, such as hickory, ash, and the oaks, the production of wood with low specific gravity is caused by some unfavorable condition which interferes with the normal growth of the tree. As a rule, wood of fairly rapid growth put on at any

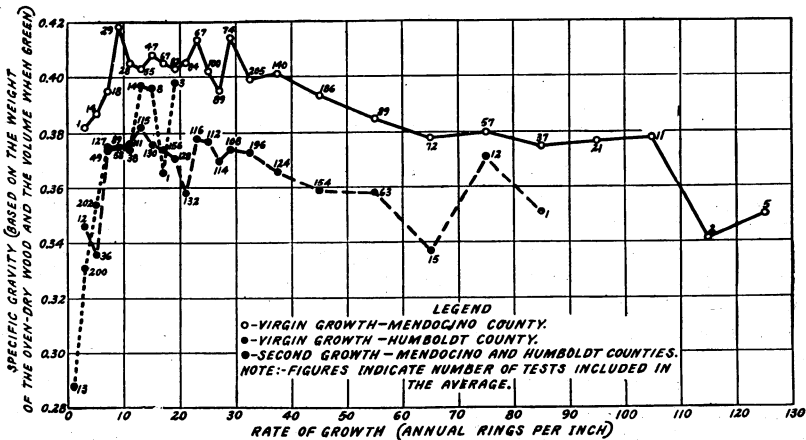


FIGURE 8.—Relation between specific gravity and rate of growth of the heartwood of redwood.

period of the life of the tree, is likely to be excellent in weight and strength. Wood of slow but uniform growth near the center of a tree may also be of high density, but wood of slow growth near the outside of the same tree is sure to be poorer if an interval of faster growth has intervened, or if the outer growth is slower than that about the center (39). Hence, in the ring-porous hardwoods fast growth (few rings per inch) is generally indicative of good strength properties, although slow growth does not necessarily indicate weak material. An exception is found in the rapid growth material from swelled butts of swamp-grown trees (p. 40).

Of the diffuse-porous hardwoods studied, sugar maple trees produced dense wood during early age whether their growth was rapid or slow. In some of the yellow poplar trees examined, wood of more rapid growth near the center was lighter in weight than that from the rest of the cross section, while other trees growing on rich alluvial soil

did not exhibit this difference. Accelerated growth following a period of slow growth resulted in an increase in the specific gravity of the wood, and hence in strength.

Softwood species show a wide range in density and strength at each rate of growth, but usually the strongest material is associated with a normal growth rate. Exceedingly rapid or exceptionally slow growth is most likely to be attended by low density and low mechanical properties. The lighter weight, slow-growth material shrinks and swells less with moisture changes than the heavier material, and usually stays in place better because of its greater freedom from internal stresses, so that it is to be preferred for many uses not primarily involving strength.

Figure 8 illustrates the relations between rate of growth (rings per inch) and specific gravity for redwood (24), and figure 9, the relation between rate of growth and modulus of rupture and work to maximum load for hickory.

TIMBER FROM LIVE AND FROM DEAD TREES

Sound wood from trees killed by insects, fungi, wind, or fire is, unless unduly checked, as good for any structural purpose as that from trees that were alive when cut (20).

If a tree stands on the stump after its death the sapwood is likely to become decayed or to be severely attacked by wood-boring insects, and in time the heartwood will be similarly affected. Such deterioration occurs also in logs that have not been properly cared for subsequent to being cut from live trees. Because of variations in climatic and local weather conditions and in other factors that affect the rate of deterioration, the length of the period during which timber may stand dead on the stump or may lie in the forest without serious deterioration varies. Tests on wood from trees of one species that had stood as long as 15 years after fire-killing demonstrated that this wood was sound and as strong as wood from live trees. Also logs of some of the more durable species have had thoroughly sound heartwood after lying on the ground in the forest for several decades. On the other hand, decay may cause great loss of strength within a very brief time, both in trees standing dead on the stump and in logs that have been cut from live trees and allowed to lie on the ground. Consequently, the important consideration is not whether the trees from which timber products are cut are alive or dead, but whether the products themselves are free from decay or other defects that would render them unsuitable for use. In considering the utility of timber from a dead tree it is helpful to remember that the heartwood of a living tree is entirely dead, and in the sapwood only a fraction of the cells are alive.

Decay that is not sufficiently advanced to be readily detected may still affect seriously the strength of a piece of wood. For this reason and also because decay is present in timber from dead trees more frequently than in that cut from freshly felled live trees, timber from dead trees needs more careful inspection. Specifications for some timber products, notably poles and piling, often require that only live trees be used. This requirement is difficult to enforce unless inspection is made in the forest, because wood cut from dead trees before weathering, seasoning, discoloration, decay, insect attack, or similar change has occurred cannot ordinarily be distinguished from wood

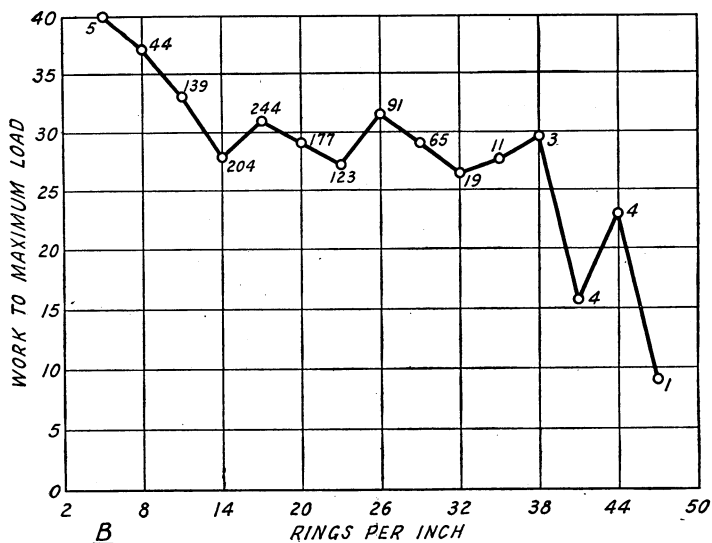
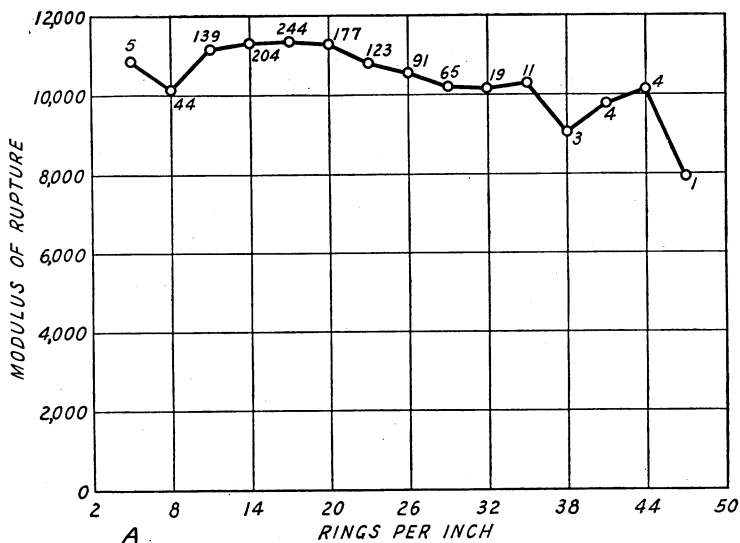


FIGURE 9.—Relation between the rate of growth and modulus of rupture (A) and also work to maximum load for green hickory (B). Figures indicate number of tests included in the average.

taken from live trees. Many specifications omit the live-tree requirement, depending entirely on inspection to determine the suitability of timber for use.

EFFECT OF RESIN AND OF TURPENTINING

Resin is formed in some of the conifers, especially the southern pines. Amounts up to 6 percent of the weight of the dry wood are common, and pieces with a resin content up to 50 percent are sometimes found.

Tests at the Forest Products Laboratory on southern yellow pine indicate that resin will slightly increase some strength properties but the effect is too small to be of any practical significance (10). An excessive amount of resin is sometimes associated with an injury such as a compression failure that may have greatly reduced the strength.

Longleaf and slash pine trees are frequently "tapped" for turpentine. The results of a special investigation, involving mechanical tests, and physical and chemical analyses of the wood of turpented and unturpented trees from the same locality (10), show that (1) turpented timber is as strong as unturpented if of the same weight (table 15); (2) the weight and shrinkage of the wood is not affected by turpentine; and (3) except in parts adjacent to the "faces" where there may be a concentration of resin, turpented trees contain practically neither more nor less resin than unturpented trees, the exudation of resin occurring only from the sapwood, and therefore the resin content of the heartwood is not affected by the turpentine process.

TABLE 15.—*Effect of turpentine on the strength of longleaf pine*

Item	Tests	Relative specific gravity of test pieces	Modulus of rupture	Maximum crushing strength (parallel to grain)
	Number		Lb. per sq. in.	Lb. per sq. in.
Unboxed (not turpented) trees.....	400	1.00	12,358	7,166
Boxed (turpented) and recently abandoned.....	390	1.07	12,961	7,813
Boxed (turpented) and abandoned 5 years.....	535	1.03	12,586	7,575

EXTRACTIVES AS RELATED TO STRENGTH

Extractives are constituents that dissolve when a piece of wood is placed in a solvent that has little or no effect on the wood substance. They are referred to as cold-water, hot-water, or alcohol-soluble extractives, depending on the solvent used. Extractives are found in the heartwood of many species and are especially abundant in redwood, western red cedar, and black locust. These species are also relatively high in certain strength properties for the amount of wood substance they contain, particularly when unseasoned, and tests have shown that the presence of extractives is probably accountable. The extent to which extractives affect the strength is apparently dependent upon the amount and nature of the extractives, the species of wood, the moisture condition of the piece, and the mechanical property under consideration. Of the properties examined, maximum crushing strength in compression parallel to the grain showed the greatest increase as the result of the infiltration of extractives accompanying the change of

sapwood into heartwood, and shock resistance the least, with modulus of rupture intermediate. In fact, under some conditions shock resistance appears to be actually lowered by extractives. That extractives may affect different species differently is indicated by the fact that they appear to affect the strength of western red cedar less than the strength of black locust, although black locust has a smaller percentage of extractives (23). Difference in the character of the extractives is probably also a factor in this connection.

TIME OR SEASON OF CUTTING

The time or season of cutting is sometimes thought to affect the properties and durability of wood, but so far as is known it actually has very little direct effect on the characteristics of the wood itself. The method of handling after cutting, however, may be very important. During the summer, for instance, seasoning proceeds more rapidly and is more apt to produce checking than in the winter. Insects, stains, and decay-producing fungi are more vigorous in the summer and the freshly-cut wood is most subject to attack at this time. Winter cutting, therefore, has the advantage that more favorable seasoning conditions and greater freedom from stains, molds, decay, and insects simplify the problem of caring for the timber before conversion. There is but little difference in the moisture content of green wood in winter and in summer.

MOISTURE AS RELATED TO STRENGTH

Wood in the green state contains considerable moisture varying from about 30 to 40 percent (based on the weight of the dry wood) in the heartwood of some of the pines to over 200 percent in some other species. Part of this moisture is held absorbed by the cell walls and part is held within the cell cavities as water is held in a container (15, 47, 60). As wood dries, the cell walls do not give off moisture until the adjacent cavities are empty. The condition in which the cell walls are fully saturated and the cell cavities empty is known as the "fiber-saturation point." It varies from 25 to 35 percent moisture content.

Increase in strength begins when the cell walls begin to lose moisture; that is, after the wood is dried to below the fiber-saturation point. From this point on most strength properties increase rapidly as drying progresses. This increased strength of dry over green wood of the same dimensions is due to two causes: (1) Actual strengthening and stiffening of the cell walls as they dry out, and (2) increase in the compactness or the amount of wood substance in a given volume because of the shrinkage that accompanies drying below the fiber-saturation point.

Drying wood down to 5-percent moisture may add from about 2½ to 20 percent to its density, while in small pieces its end-crushing strength, and bending strength, may easily be doubled and in some woods tripled. Thus, the first of the two factors mentioned is the one chiefly responsible for the increase in strength.

The increase in strength with seasoning is much greater in small clear specimens of wood than in large timbers containing defects. In the latter the increase in strength is to a large extent offset by the influence of defects that develop in seasoning.

The various strength properties are not equally affected by changes in moisture content. Whereas some properties, such as crushing strength and bending strength increase greatly with decrease in moisture, others, such as stiffness, change only moderately, and still

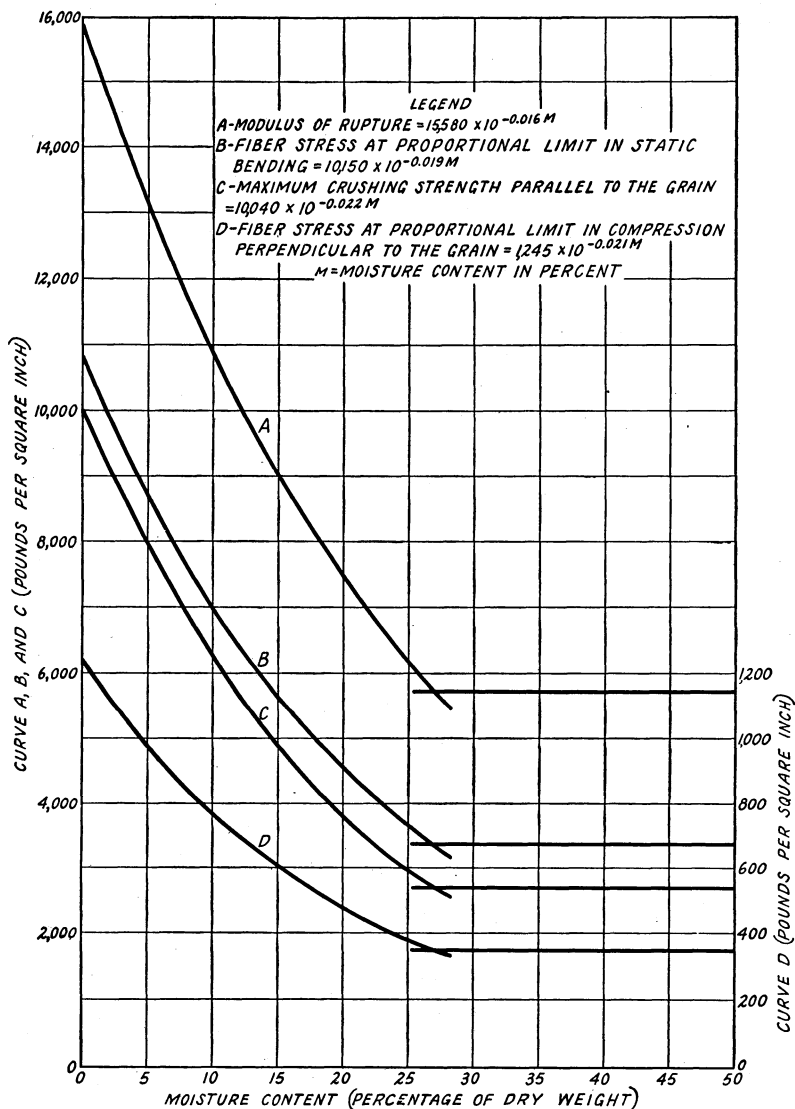


FIGURE 10.—The relation between mechanical properties and the moisture content of small clear specimens of Sitka spruce.

others, such as shock resistance, may even show a slight decrease. This last effect is due to the fact that drier wood does not bend so far as green wood before failure, although it will sustain a greater load, and because shock resistance or toughness is dependent upon both strength and pliability.

The following tabulation shows the average variation of the strength properties of wood with change in moisture content, and figure 10 shows graphically the effect of moisture on certain strength properties of Sitka spruce.

Average increase (or decrease) in value effected by lowering (or raising) the moisture content 1 percent

Property:	Percent
Static bending:	
Fiber stress at proportional limit.....	5
Modulus of rupture, or cross-breaking strength.....	4
Modulus of elasticity or stiffness.....	2
Work to proportional limit.....	8
Work to maximum load or shock-resisting ability.....	$\frac{1}{2}$
Impact bending:	
Fiber stress at proportional limit.....	3
Work to proportional limit.....	4
Height of drop of hammer causing complete failure.....	$-\frac{1}{2}$
Compression parallel to grain:	
Fiber stress at proportional limit.....	5
Maximum crushing strength.....	6
Compression perpendicular to grain:	
Fiber stress at proportional limit.....	$5\frac{1}{2}$
Hardness, end grain.....	4
Hardness, side grain.....	$2\frac{1}{2}$
Shearing strength parallel to grain.....	3
Tension perpendicular to grain.....	$1\frac{1}{2}$

METHODS OF MOISTURE-STRENGTH ADJUSTMENT

It is often desirable to adjust strength values for wood at one moisture content to what they would be under some other condition. This can be done quite accurately when the data apply to small clear specimens which are quite uniformly dried so that the moisture content is approximately the same at all points of the cross section.

Three general methods, differing materially in their accuracy, and in simplicity and facility of application, may be used for moisture-strength adjustments. These are referred to as the (1) approximate method, (2) the equation method, and (3) the graphical method.

APPROXIMATE METHOD

The approximate method of moisture-strength adjustment consists simply in an application of the percentage figures of the tabulation above for the property under consideration, regardless of species. For example, if the maximum crushing strength of Sitka spruce at 12-percent moisture content is 5,610 pounds per square inch, what is the approximate value at 10-percent moisture? From the tabulation it may be noted that the average change in maximum crushing strength for 1-percent change in moisture is 6 percent. For 2-percent change in moisture content (12-percent moisture to 10-percent moisture) the average expected change in maximum crushing strength would consequently be 12 percent. Since this property increases with decrease in moisture content, the approximate increase in strength is 12 percent of 5,610=673, and the approximate maximum crushing strength at 10-percent moisture is 5,610+673=6,283 pounds per square inch.

This is the least accurate of the several methods described, and is useful only for making rough approximations. For comparison it may be noted that application of the equation method to the foregoing example gives a value of 6,194 pounds per square inch.

EQUATION METHOD

Studies at the Forest Products Laboratory (60) have led to the derivation of a formula for strength adjustment, the numerical solution of which affords more accurate estimates than any other method. This formula, known as the exponential formula is based on the fact that for any one species and strength property, moisture-content values within certain limits and the logarithms of corresponding strength values have been found to conform closely to a straight-line relationship.

The formula may be written

$$\text{Log } S_D = \text{log } S_C + (C - D) \frac{\text{log } (S_B \div S_A)}{A - B}$$

where A , B , C , and D , are values of moisture content and S_A , S_B , S_C , and S_D are corresponding strength values; S_C is the strength value from tests made at moisture content C and S_D is this strength value adjusted to moisture content D . The expression

$$\frac{\text{log } (S_B \div S_A)}{A - B}$$

which is equivalent to

$$\frac{\text{log } S_B - \text{log } S_A}{A - B}$$

measures the change in strength property caused by a change of 1 percent in the moisture content. Required for evaluation of this expression are strength values S_A and S_B found from tests made at two different moisture contents A and B on matched specimens; that is, specimens that can be assumed to be alike except for the single factor of moisture content, such as specimens from closely adjacent positions within the same annual growth layers.

When in any instance a strength value is that for green material, the corresponding moisture content to be used for the species under consideration is listed in the following tabulation:

Moisture content	
Species ^a :	Percent
Ash, white	24
Birch, yellow	27
Chestnut	24
Douglas fir	24
Hemlock, western	28
Larch, western	28
Pine:	
Loblolly	21
Longleaf	21
Norway	24
Redwood	21
Spruce:	
Red	27
Sitka	27
Tamarack	24

^a The exact value has been determined only for the species listed here. For other species the value of 24 percent may be assumed to apply.

Three types of moisture-strength adjustment differing with respect to the source of the data for evaluating the expression

$$\frac{\log (S_B \div S_A)}{A-B}$$

are defined and illustrated in the following paragraphs:

TYPE 1. From tests on matched groups of material at two different moisture-content values, a strength value corresponding to a third value of moisture content is computed, the data for evaluating the expression

$$\frac{\log (S_B \div S_A)}{A-B}$$

being supplied by the tests on the material under consideration.

Example: The average maximum crushing strength of Sitka spruce as listed in table 1 is 2,670 pounds per square inch for green material and 5,610 pounds per square inch for material at 12 percent moisture. Compute the maximum crushing strength corresponding to a moisture content of 14 percent.

$S_A=2,670$ from table 1, and A for green material is 27.

$S_B=5,610$, $B=12$. C may be taken either as 27 or 12 with corresponding choice of S_C ; that is, either the value for green material or that for material at 12-percent moisture may be adjusted to 14-percent moisture content.

$D=14$.

Taking $C=12$, and $S_C=5,610$.

$$\text{Log } S_{14} = \log 5,610 + (12-14) \frac{\log (5,610 \div 2,670)}{27-12}$$

$$= 3.7490 - 2 \times \frac{0.3224}{15}$$

$$= 3.7490 - 0.0430 = 3.7060$$

Then
or

$$S_{14} = \text{antilog } 3.7060 = 5,082.$$

Taking $C=27$ and $S_C=2,670$

$$\text{Log } S_{14} = \log 2,670 + (27-14) \frac{\log (5,610 \div 2,670)}{27-12}$$

$$= 3.4265 + 13 \times \frac{0.3224}{15}$$

$$= 3.4265 + 0.2794 = 3.7059$$

Then $S_{14} = \text{antilog } 3.7059 = 5,082$ as before, and the maximum crushing strength of Sitka spruce at 14-percent moisture content, as obtained by adjusting to this moisture content the average values given in table 1, is 5,082 pounds per square inch.

TYPE 2. A strength value obtained at one moisture content is adjusted to a second value of moisture content, the data for evaluating the expression

$$\frac{\log (S_B \div S_A)}{A-B}$$

as found in other tests on the same species being assumed to apply.

Example: A specimen of longleaf pine at 9.8-percent moisture content was found from test to have a modulus of rupture of 13,500 pounds per square inch. Estimate the value of modulus of rupture that would have resulted had the test been made at a moisture content of 12 percent.

Values of modulus of rupture on matched specimens of longleaf pine are given in table 1 as 8,700, which is equal to S_A , and 14,700, which is equal to S_B , pounds per square inch for the green and 12-percent moisture conditions, respectively. A , from the tabulation (p. 51) =21, B =12, C =9.8, and D =12.

Then substituting in the formula

$$\begin{aligned}\text{Log } S_{12} &= \log 13,500 + (9.8 - 12) \frac{\log (14,700 \div 8,700)}{21 - 12} \\ &= 4.1303 - 2.2 \times \frac{0.2278}{9} \\ &= 4.1365 - 0.0557 = 4.0746\end{aligned}$$

$S_{12} = \text{antilog } 4.0746 = 11,874$

and the modulus of rupture at 12-percent moisture as estimated from the value determined at 9.8-percent moisture is 11,874 pounds per square inch.

TYPE 3. As in type 2, except that the data for evaluating the expression

$$\frac{\log (S_B \div S_A)}{A-B}$$

for the same species not being known an average value as computed from tests of other species is assumed to apply.

Example: The modulus of rupture of a sample of a hardwood species tested at 9-percent moisture content was 11,700 pounds per square inch. Estimate the value at 12-percent moisture. Here $S_C = 11,700$, $C = 9$, and $D = 12$. No values of S_A and S_B for the same species being available, it is assumed that the strength-moisture relationship for this hardwood is similar to that for hardwood species

in general and 1.59, the value of $\frac{S_{12}}{S_G}$ as given for modulus of rupture

of hardwood species in table 16, is used for $\frac{S_A}{S_B}$. $A = 12$ and for B the

value of 24 from the tabulation on page 51 is taken. Substituting in the formula:

$$\begin{aligned}\text{Log } S_{12} &= \log 11,700 + (9 - 12) \frac{\log 1.59}{24 - 12} \\ &= 4.0682 - 3 \times \frac{0.2014}{12} \\ &= 4.0682 - 0.0503 = 4.0179\end{aligned}$$

TABLE 16.—Average strength ratios $\left(\frac{S_{12}}{S_a}\right)$ for species in drying from a green condition to 12-percent moisture content

Property	Hardwoods (113 species)	Softwoods (54 species)
Static bending:		
Fiber stress at proportional limit.....	1.80	1.81
Modulus of rupture.....	1.59	1.61
Modulus of elasticity.....	1.31	1.28
Work to proportional limit.....	2.49	2.56
Work to maximum load.....	1.05	1.13
Impact bending:		
Fiber stress at proportional limit.....	1.44	1.39
Work to proportional limit.....	1.68	1.59
Height of drop causing complete failure.....	.89	1.03
Compression parallel to grain:		
Fiber stress at proportional limit.....	1.74	1.86
Maximum crushing strength.....	1.95	1.97
Compression perpendicular to grain: Fiber stress at proportional limit.....	1.84	1.96
Hardness:		
End.....	1.55	1.67
Side.....	1.33	1.40
Shear parallel to the grain: Maximum shearing strength.....	1.43	1.37
Tension perpendicular to grain: Maximum tensile strength.....	1.20	1.23

$$S_{12} = \text{antilog } 4.0179 = 10,400$$

Obviously, adjustments of type 1 are most and those of type 3 least accurate. The inaccuracy in types 2 and 3 is due to the assumed values of the expression

$$\frac{\log (S_B \div S_A)}{A - B}$$

not being definitely applicable.

In types 2 and 3 the accuracy of the computed or estimated value decreases with increase in moisture difference for which adjustment is made.

GRAPHICAL METHOD

The graphical method consists of using a chart (fig. 11) for the solution of the formula described under the equation method, thus avoiding the use of logarithms as required in the arithmetical calculation. This method is, therefore, simpler than the equation method, but due to the personal equation in reading the chart and the small scale of the chart, the adjustment is less accurate.

The procedure in the use of the chart is as follows:

1. First determine K , the ratio of the strength when dry to the strength when green for the strength property and species under consideration. This ratio should be determined from one of the three following sources, with preference in the order named:

(a) From the tests of matched green and dry material for which the adjustment is to be made.

(b) From the data for green and dry material of table 1.

(c) From the ratios of table 16.

2. Determine the difference in moisture between the value to be used for green material (table 1) and the moisture content of the dry material on which the preceding dry to green strength ratio is based. (For all species listed in table 1 the moisture content of the dry material is 12 percent.)

3. Determine the difference between the moisture content of the material at test and the moisture content to which adjustment is to

be made. This difference represents the range in moisture over which the adjustment is to be made.

4. Locate on the chart a point corresponding to the difference in moisture content as determined under 2 and the ratio K as determined under 1. From the line joining this point with the lower

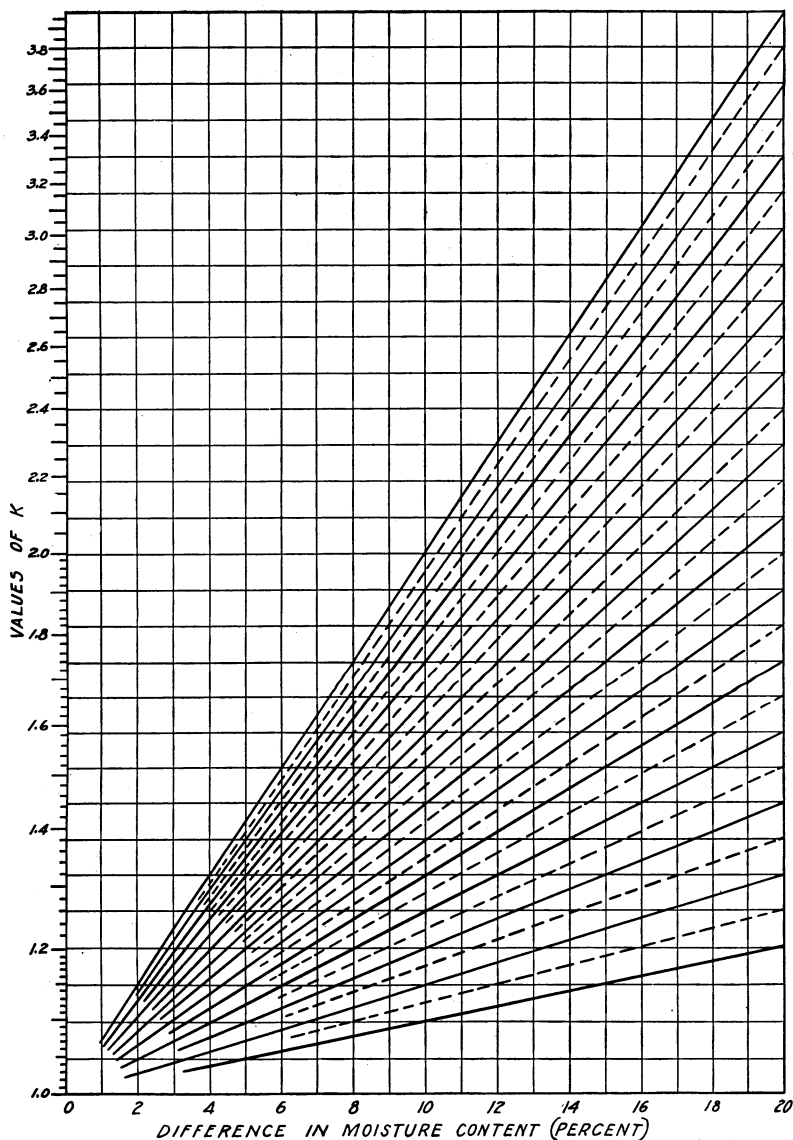


FIGURE 11.—Chart for making strength-moisture adjustments.

left-hand corner of the chart the ratio corresponding to any difference in moisture content can be found.

5. Locate on this line, the point that corresponds to the difference in the moisture content as determined under 3, and read the corresponding new strength ratio K on the left-hand scale.

6. (a) If the adjustment is being made to a lower moisture content than that at which the tests were made, multiply the strength at test by the new ratio (as obtained in 5 above) to get the adjusted strength value.

(b) If the adjustment is being made to a higher moisture content than that at which the tests were made, divide the strength at test by the new ratio (as obtained in 5 above) to get the adjusted strength value.

Example 1. Tests of matched specimens of Douglas fir gave values of maximum crushing strength of 3,940 and 10,680 pounds per square inch, respectively, for green wood and wood at 6.2-percent moisture content. What is the strength at 12-percent moisture content?

1. The ratio $K = \frac{10,680}{3,940} = 2.71$.

2. The difference between the moisture content to be used for green material (tabulation on p. 51) and that at test is $24 - 6.2 = 17.8$ which is the difference in moisture content to which the ratio 2.71 applies.

3. The difference between the moisture content of the dry material at test and the moisture content to which adjustment is desired is $12 - 6.2 = 5.8$.

4. Starting with the ratio 2.71 on the left-hand margin of figure 11, and following horizontally to the vertical representing the 17.8-percent moisture difference, locate a point.

5. Following the converging line on which this point is located to its intersection with a vertical corresponding to the moisture difference of 5.8 (step 3), and thence horizontally to the left-hand margin, a new ratio K of 1.38 is found.

6. The maximum crushing strength at 12 percent moisture is $\frac{10,680}{1.38} = 7,740$ pounds per square inch. The moisture content of 12 percent to which adjustment is made is higher than the moisture content at test. Consequently the strength value at test is divided by the ratio.

Example 2. The modulus of rupture of a sample of hardwood species tested at 13-percent moisture content was 10,030 pounds per square inch. What is the estimated value at 9-percent moisture?

1. Since data on matched green and dry material are not available, the average ratio of strength when dry (12-percent moisture content) to that when green for a hardwood is taken from table 16, and is 1.59.

2. From the tabulation on page 51, the moisture content to be used for green material is assumed to be 24-percent moisture content. The ratio of 1.59 applies to material at 12-percent moisture content. The moisture difference is, therefore, $24 - 12 = 12$ -percent moisture content.

3. The differences between the moisture content of the sample at test and the moisture to which adjustment is desired is $13 - 9 = 4$ percent.

4. Starting with the ratio 1.59 on the left-hand margin of figure 11, and following horizontally to the vertical representing 12-percent moisture difference, locate a point.

5. Following the converging line through this point to its intersection with the vertical corresponding to the moisture difference of 4

percent (step 3), and thence horizontally to the left-hand margin, the ratio K of 1.165 is found.

6. The modulus of rupture at 9-percent moisture content is $10,030 \times 1.165 = 11,680$ pounds per square inch. In this instance the moisture content of 9 percent to which adjustment is made is lower than the moisture content at test and the strength value at test is multiplied by the ratio K .

LIMITATIONS TO MOISTURE-STRENGTH ADJUSTMENTS

When the strength data are from tests on material in which the moisture is not uniformly distributed in the cross section, moisture-strength adjustments on the basis of the methods just outlined cannot be considered as reliable, and no acceptable general method for the adjustment of such data is available.

COMPARATIVE STRENGTH OF AIR-DRIED AND KILN-DRIED WOOD

Some wood users contend that kiln-dried wood is brash and not equal in strength to wood that is air-dried. Others advance figures purporting to show that kiln-dried wood is much stronger than air-dried. However, comparative strength tests, made by the Forest Products Laboratory on kiln-dried and air-dried specimens of 28 common species of wood, show that good kiln drying and good air drying have the same effect upon the strength of wood but that severe conditions in the kiln will lower most of the strength properties (56).

The belief that kiln drying produces stronger wood than air drying is usually the result of failure to consider differences in moisture content. The moisture content of wood on leaving the kiln is generally from 2 to 6 percent lower than that of thoroughly air-dried stock. Since wood rapidly increases in most strength properties with loss of moisture, higher strength values may be obtained from kiln-dried than from air-dried wood. Such a difference in strength is not permanent, since in use a piece of wood will come to practically the same moisture condition whether it is kiln-dried or air-dried.

It must be emphasized that the appearance of wood is not a reliable criterion of the effect the drying process may have upon its strength. The strength properties may be seriously injured without visible damage to the wood. Also, it has been found that the same kiln-drying process cannot be applied with equal success to all species. To insure kiln-dried material of the highest strength, a knowledge of the correct kiln conditions to use with stock of a given species, grade, and thickness, and a record showing that no more severe treatment has been employed, are necessary.

TEMPERATURE AS RELATED TO STRENGTH

The moisture content of wood determines to a large extent how it is affected by temperature.

Lowering the temperature of wet or green wood decidedly increases its stiffness and its strength in compression parallel to grain. Freezing temperatures have resulted in increases of from 5 to 25 percent as compared to values at normal room temperature, the results varying with the strength property considered, the species, and the moisture condition (12, 47). Such effects are much less pronounced in wood

whose moisture content is below the fiber-saturation point and become comparatively small at very low moisture content values.

Tests in compression parallel to grain have shown values for green wood at temperatures near the boiling point about one-fifth as great as at normal room temperature. Including both moisture and temperature effects a tenfold difference in maximum crushing strength has been observed between specimens tested immediately after soaking in hot water and other matched specimens that were tested after cooling subsequent to over drying to expel all moisture. This illustrates the importance of establishing comparable conditions of moisture and temperature when making comparisons involving strength.

Aside from the current or immediate effects of temperature as just cited, tests have shown that heating to or above the boiling point for several hours or to more moderate temperatures, such as are used in kiln drying, for longer periods may permanently lower the strength properties as compared to unheated wood at the same moisture content. The effect on the strength at some lower moisture content is somewhat less than on the strength of wood in the green or wet condition. The amount of this lowering apparently depends on a large number of variables including species, size, and moisture content of the material when heated, the temperature, and the duration of the heating period (22, 42, 59).

Steaming or boiling of wood for brief periods is used to make it pliable and prepare it for bending to curved form. Such preparation makes it possible to bend the wood to curvatures otherwise unattainable. The heating is usually for comparatively brief periods and probably has little permanent effect on the strength.

EFFECT OF PRESERVATIVE TREATMENT ON STRENGTH

Coal-tar creosote, water-gas tar, wood-tar creosote, creosote-tar mixtures, and creosote-petroleum mixtures are practically inert to wood and have no chemical influence upon it that would affect its strength (6). The 2- to 5-percent solutions of zinc chloride commonly used in preservative treatment apparently have no important effect.

Although wood preservatives are not harmful in themselves, the treatment used in injecting them into the wood may result in considerable loss of strength to the wood. Green wood conditioned for the injection of preservatives by steaming or by boiling under vacuum may be seriously reduced in strength if extreme temperatures or heating periods are employed. Consequently, care should be used to keep the temperature as low and the duration of the treatment as short as is consistent with satisfactory absorption and penetration of the preservative (59). A gage pressure of 20 pounds (259° F.) is sufficiently high for steam conditioning. No advantage is known to result from higher pressures, and the resulting higher temperatures are much more likely to damage the wood. The maximum temperature employed in the boiling-under-vacuum process is usually less than 210°.

The use of pressures greater than 175 pounds in injecting preservatives into wood that is soft from long heating is likely to cause severe end checking and collapse. Considerably higher pressures can be used if the wood has been heated for a short time only, or not at all. Woods of low density are more subject to injury from high pressures than woods of high density.

STRENGTH AS AFFECTED BY RATE AND METHOD OF LOADING

DURATION OF STRESS

The duration of stress or the time during which a load or force acts on a beam or other wooden member has an important bearing on the use of the timber, and on the adaptation of results of tests to the design of different kinds of structures or members. For instance, when an airplane traveling at high speed suddenly changes its course as in flattening out following a dive, wooden members may without damage be subjected for a few seconds to forces which would cause complete failure if applied for a longer time. In impact-bending tests, where the load is suddenly applied and is maintained for but a fraction of a second, a stick will resist a force more than double that required to produce failure in a standard static-bending test. On the other hand, beams under continuous loading for years, as in warehouse floors, will fail at loads one-half to three-fourths as great as would be required to produce failure in the standard static bending test where the maximum load is reached in a few minutes (5, 27, 31, 49).

From the foregoing it is clear that tests made under widely different conditions of loading are not comparable, and that the allowable stress in a wooden beam must be determined in accordance with the loading to which it will be subjected in service. The rapidity with which the load is applied and the duration of the stress are material factors in the result.

Figure 12 presents an interpretation of some data on the influence of rate of loading from tests of small clear specimens. A tenfold increase or decrease in the rate of loading produces approximately a 10-percent increase or decrease, respectively, in bending strength.

In timber testing, the allowable tolerance in rate of loading is limited to ± 25 percent of the required rate in order to keep the variation in test results from this cause within about 1 percent (48).

FATIGUE

Some tests have been carried on both in the United States and in Europe to determine the effect of repeated stress and vibration although no extended and thorough or complete investigation has been made (30).

In tests made at the Forest Products Laboratory on beams of circular cross section, rotated so that the outer fibers were stressed in compression and tension alternately at each revolution, the fatigue limit was found to be about one-third of the modulus of rupture as determined in static tests, on beams having square cross sections. Sometimes the fatigue limit of wooden beams with circular cross section is expressed as a ratio to the static modulus of rupture of beams also of circular cross section. Expressed in this way the ratio is less than one-third, since a beam of circular section has a form factor of 1.18. These tests involved over 300,000 stress cycles (table 17).

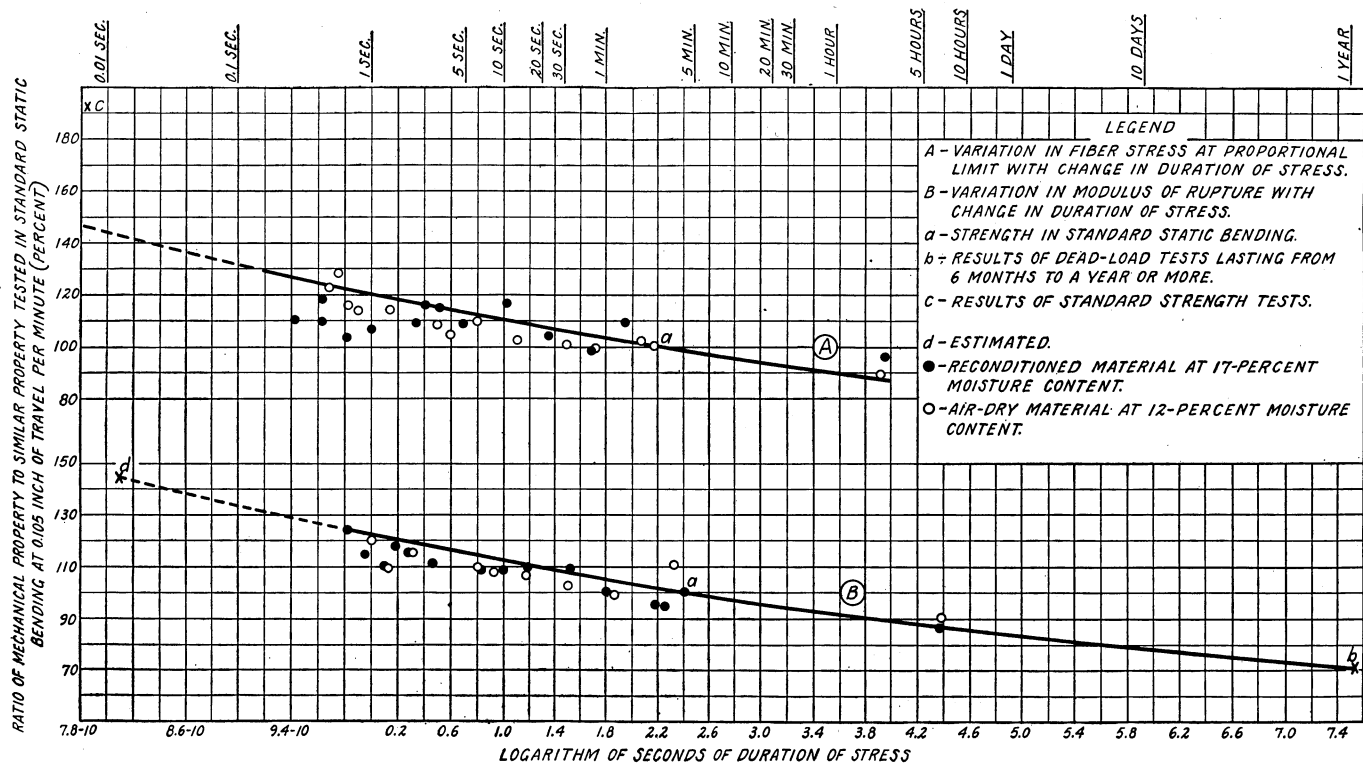


FIGURE 12.—The relation between fiber stress at proportional limit in static bending and modulus of rupture of Sitka spruce, and duration of stress. Each point is the average of the results of from 5 to 10 tests. Duration of stress is the total time between application of load and reaching the proportional limit or the maximum load.

TABLE 17.—*Results of static tests and fatigue rotating beam tests of wood*

Kind of wood	Moisture content	Specific-gravity ¹	Static modulus of rupture for specimens of circular cross section	Estimated endurance limit (rotating beam test specimens of circular cross section)	Ratio of endurance limit to modulus of rupture of beams of circular cross section	Ratio of endurance limit to modulus of rupture of beams of square cross section ²
	<i>Percent</i>		<i>Lb. per sq. in.</i>	<i>Cycles</i>		
Sitka spruce.....	13.8	0.38	12,100	3,200	0.27	0.32
Southern white oak.....	82.4	.58	10,600	3,200	.30	.35
Douglas fir.....	14.3	.50	15,000	4,000	.27	.32
Do.....	23.8	.52	12,800	3,900	.31	.37

¹ Specific gravity, oven dry, based on volume at test.² Calculated on basis that form factor of beam of circular cross section is 1.18.

Studies made on cantilever beams having an enlarged cross section at the point of support demonstrated that the fatigue limit varied greatly depending on whether the change of cross section was abrupt or gradual.

With even what is normally considered a generous fillet the fatigue limit is lowered markedly. This effect, together with the influence of form factors, has led some investigators erroneously to place the fatigue limit for wood as low as one-sixth of the static modulus of rupture.

Tests made at the Forest Products Laboratory on tapered specimens of a form to obviate changes in cross section that would influence failure show that, for a stress just slightly greater than the fatigue limit, failure occurs at not more than 2,000,000 load reversals and in some species at less than 1,000,000 reversals. Tests at stresses only slightly less than the fatigue limit showed no failure after reversals ranging in number from 14,000,000 to 125,000,000.

Other tests on Sitka spruce in which specimens of rectangular cross section were vibrated through approximately 5,000,000 cycles indicate that the modulus of elasticity is not greatly affected by vibration. No effect on fiber stress at proportional limit and modulus of rupture could be detected from these tests, the values being about the same for specimens which had and which had not been vibrated. The tests indicate that the same stress prevails at the fatigue limit with vibrated specimens of rectangular cross section as with rotated specimens of circular cross section.

Further studies to obtain more specific information on the effects of vibration and fatigue, particularly when subjected to a large number of stress cycles, and to determine the variation of these properties with different species are needed.

EFFECT OF TIME OR LENGTH OF SERVICE ON THE STRENGTH OF WOOD

It is sometimes assumed that wood is perishable and is suitable only for use in temporary structures. Although wood, like other materials, is subject to attack by destructive agents, there is ample historical evidence of its permanence when protected from attack by such agencies as fungi, insects, marine borers, and rodents.

So far as is known the lignin and cellulose which constitute the wood substance are not subject to chemical change with time when

wood is adequately protected from the elements and other destructive agencies, although the color of wood may be slightly changed by long-continued exposure to air. Possibly this change of color results from oxidation of substances that are not parts of the wood substance.

The effect of time cannot be appraised accurately by tests of wood from old structures since the original strength of the material is unknown. The evidence from such tests as are on record is that no significant loss of strength has occurred in the absence of the destructive agencies enumerated (1, 2, 14).

The shrinkage that occurs in the drying of wood induces internal stresses. In time, equalization of differentials of moisture content combined with the action of wood as a plastic material relieves such stresses. This effect would tend to increase the resistance to external forces but its effect is probably not great enough to be significant in most uses of wood.

A recent survey has shown that literally hundreds of bridges made entirely or partly of wood have served satisfactorily and with but little attention for long periods. Many that are more than a century old are still in service. Others have given way, while still in good condition, to the demands for greater width of roadway and higher load capacity than that for which they were built (11).

SIZE OF PIECE AS RELATED TO STRENGTH

It is well known that the size and form of a timber have a definite bearing on its load-carrying ability for different purposes, but the manner in which the load-carrying ability and stiffness vary with dimensions is not so generally understood.

SIZE OF COLUMNS OR COMPRESSION MEMBERS

In a short column, that is, a column whose ratio of length to least dimension is 11 to 1 or less, the end load that can be carried varies simply with the area of the cross section of the piece, other factors being equal. However, with a long column, one whose length exceeds about 20 times its least dimension, the end load that can be supported (with a given "end condition") varies not as the cross-sectional area, but directly as the greater dimension of the cross section, directly as the cube of the lesser, and inversely as the square of the length. Columns are usually either square or round. Hence the load that can be carried by a long column of square or circular cross section varies directly as the fourth power of the side of the square or diameter of circle, and inversely as the square of the length. The load that can be supported by columns of intermediate length is intermediate between that for the short and long column (32).

SIZE OF BEAMS

The load that a beam of rectangular cross section can carry, other factors being equal, varies directly as the width, directly as the square of the depth, and inversely as the span. The deflection for a given load varies inversely as the width, inversely as the cube of the depth, and directly as the cube of the span.

A few numerical examples will serve to illustrate these relations. Let it be assumed that a beam $1\frac{1}{2}$ by $7\frac{1}{2}$ inches (nominal 2 by 8) is used on edge on a 12-foot span.

EFFECT OF WIDTH

If the width of beam were increased from 1½ to 3½ inches (nominal 4-inch width) a total load about two and one-fourth times as large ($3\frac{1}{2} \div 1\frac{1}{2} = 2.23$) could be carried, and the deflection for a given load would be about 45 percent as great

$$\left(\frac{1}{3\frac{1}{2}} \div \frac{1}{1\frac{1}{2}} = 0.448 \right)$$

EFFECT OF DEPTH

If the depth were increased from 7½ to 9½ inches (nominal 10-inch depth) a total load 1.6 times as large, $(9\frac{1}{2})^2 \div (7\frac{1}{2})^2 = 1.60$, could be carried, and the deflection for a given load would be about 49 percent as great

$$\left(\frac{1}{(9\frac{1}{2})^3} \div \frac{1}{(7\frac{1}{2})^3} = 0.492 \right)$$

EFFECT OF LENGTH

If the span were increased from 12 to 15 feet a total load 80 percent as large ($\frac{1}{15} \div \frac{1}{12} = 0.80$) could be carried, and the deflection for a given load would be nearly twice as great ($15^3 \div 12^3 = 1.95$).

The preceding relations are those expressed by the usually accepted engineering formulas and are based on assumptions that are more or less inaccurate under certain conditions. Their use, however, has been long established and they may be regarded as the best general basis for calculation.

Since strength and stiffness are dependent on the form and size of piece as well as on the inherent strength of the wood, it is usually possible to compensate for the lower strength of the weaker species by increasing the size of members in accordance with engineering principles.

FORM OF CROSS SECTION AS RELATED TO STRENGTH OF WOODEN BEAMS

Calculations by the commonly accepted engineering formulas as previously illustrated are sufficiently accurate for use in the design of members of rectangular cross section for common structural purposes. Experiments have demonstrated, however, that beams may carry more or less load, depending on the form of the cross section, than would be calculated from the general beam formula, using the modulus-of-rupture value based on specimens 2 by 2 inches in cross section as given in table 1. Hence, when members of other than rectangular section are used, or when maximum accuracy is essential, as in the design of aircraft parts, modification of these formulas is necessary (36).

Tests have shown that a beam of given cross-sectional area carries the same load regardless of whether the cross section is circular, square, or diamond shape (square with diagonal in the direction of load). This is true both of loads at proportional limit and of maximum load. The corresponding stresses computed from the usual formula are 18 percent higher for the circular and 41 percent higher

for the diamond-shaped beam than for the square. Thus the circular and diamond-shaped sections may be said to have form factors of 1.18 and 1.41, respectively. On the other hand, the form factor for beams with I and box-shaped sections has been found to be less than unity and may in extreme instances be as small as 0.50.

The stresses developed in a wooden beam also depend on its size—or rather its depth. In general, the shallower the beam the greater the stresses that will be developed and conversely. This effect is sufficient to make about 7 percent difference between depths of 8 and 2 inches.

Theoretically, also, the stresses developed are affected by the width of the piece. As far as is known, this effect is not sufficiently large to be of practical significance. If, however, the width is too small in comparison with the height and span a beam may deflect sideways and fail at a lower stress than would a wider beam with other dimensions the same or than the same beam if braced against deflection sideways (52).

The effects of shape and depth of beams as just discussed apply to loads and stresses. Modulus of elasticity is not affected. Consequently, the same value of modulus of elasticity may be used for computing deflections by the usual engineering formulas regardless of the shape or depth of a beam. When, however, the relation of depth to span is such that high horizontal shearing stress is involved, the effect of shearing deformation should be considered in computing deflections (35).

DEFECTS

Defects are any irregularities occurring in or on wood that may lower some of the strength, durability, or utility values. Defects may be divided into two groups on the basis of their effect on structural timbers: (1) Those that materially affect the strength and must be considered in formulating specifications. This group includes decay, cross grain, knots, shakes, checks, and splits; and structural grading rules definitely limit the sizes of such defects according to the grade (9, 33, 34, 61). (2) Those that would normally be excluded for other reasons than their effect on the strength. This second group includes pitch pockets, wane, wormholes, warp, pith, and imperfect manufacture. These may ordinarily be disregarded in grading structural timbers but must be considered in selecting material of smaller size for special uses, such as handles or ladder parts.

DECAY

Vegetable organisms known as fungi, of which there are many varieties, are the cause of decay or rot in timber. Aside from food, which is supplied by the wood, the three essentials to their development are air, suitable temperature, and favorable moisture content. Wood that is completely submerged in water does not decay because the necessary air is lacking. Wood whose moisture content is constantly below about 16 percent does not decay because insufficient moisture is available for decay-producing organisms. The so-called dry rot develops in timber that is apparently below such a moisture content because the producing organism is capable of conducting the needed moisture from sources outside the timber itself.

Wood decays more rapidly in warm humid climates than in cool dry regions. High altitudes are as a rule less favorable to decay than nearby low areas because the average temperature is lower and the growing season for fungi is shorter.

Not all properties are affected to the same extent by a given degree of decay. Shock-resisting ability as reflected in the work values in static bending, or the height of drop in impact bending, is one of the first properties to be affected, and decay which has not progressed far enough to be visible may seriously impair this quality. Crushing strength parallel to the grain is slowest to give way, with hardness and strength as a beam holding an intermediate position. Decay often develops in localized regions or pockets and may not affect the strength of a piece uniformly.

Because of the fact that it is impossible to estimate satisfactorily either the extent to which decay has progressed, or the probability of its further development, timber containing decay in any stage should be regarded with misgiving for use where strength is important.

Two methods are available for prolonging the life of timber exposed to conditions favorable to decay: (1) Use the heartwood of species that are naturally resistant to decay; (2) impregnate the wood with a preservative (18).

The danger of decay can in many instances be lessened materially by careful attention to details of design and construction. For example, proper insulation of water pipes will prevent excess humidity and the deposition of water on woodwork in their vicinity; joints in exterior woodwork can be made so that they are readily drained or ventilated; ventilation can be provided beneath the floors of houses without basements; basement posts or columns can be raised a few inches above the floor by means of pedestals.

The sapwood of all species has low natural decay resistance and generally short life under decay-producing conditions. Common native species vary greatly with respect to the durability of the heartwood. Furthermore, all pieces of the heartwood of a species are not equally durable.

General comparisons of the relative decay resistance of different species must be estimates. They cannot be exact and they may be very misleading if interpreted as mathematically accurate and applicable in all instances. They may be very useful, however, if understood as approximate averages only, from which specific cases may vary considerably, and as having application only where conditions are favorable to decay. The classification of a number of common native species with respect to the durability of the untreated heartwood as presented in table 7 is to be so understood.

CROSS GRAIN

The term "cross grain" denotes any deviation of wood fibers from a direction parallel to the longitudinal axis of a piece.

In order to correlate cross grain with the strength properties of timber, a definite method of measurement is necessary. This is afforded by the angle between the direction of the fibers and the axis of the piece, or edge if it is parallel to the axis. The angle is usually expressed as a slope; for instance, 1 in 15, or 1 to 15, means that the grain deviates 1 inch from the edge of the piece in a distance of 15 inches.

An extensive series of tests on Sitka spruce, Douglas fir, and commercial white ash has shown that the several strength properties differ in the degree to which they are affected by cross grain and that for properties materially affected the tendency of values to fall off occurs with even slight deviations of grain (19, 57). Values presented

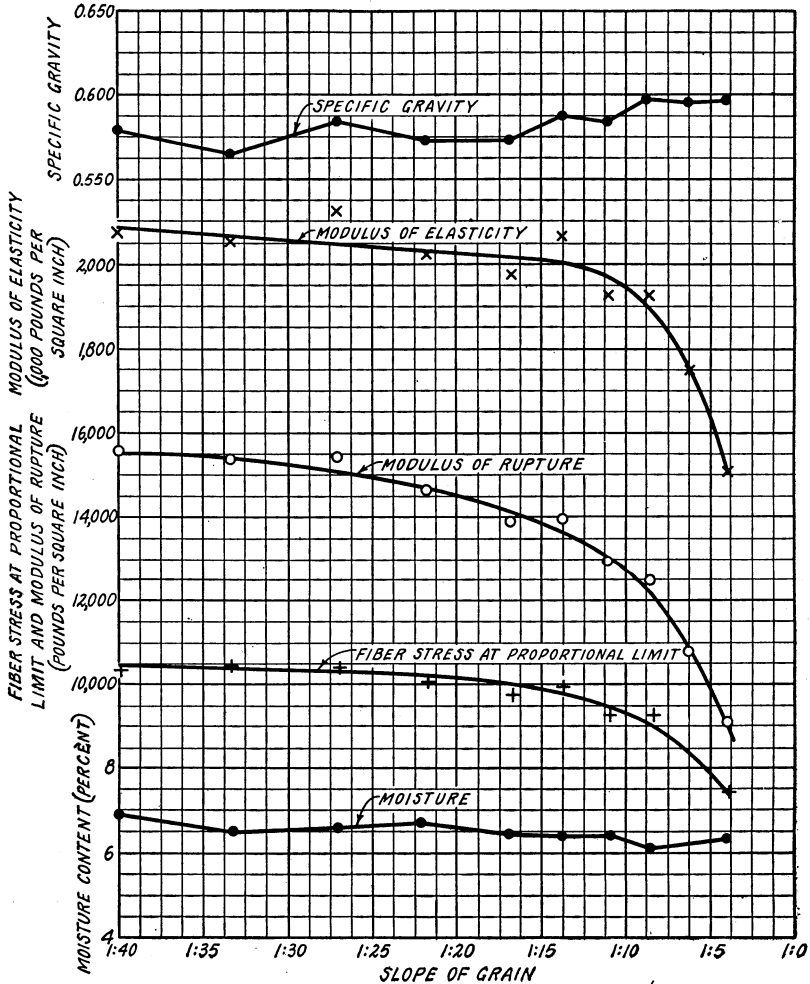


FIGURE 13.—Effect of spiral and diagonal grain on fiber stress at proportional limit, modulus of rupture, and modulus of elasticity in static bending on white ash.

in table 18 are the average percentage deficiencies for various slopes of cross grain in material that is free from checks and other defects, as compared with straight-grained stock. Figure 13 presents the results for white ash graphically. Specific gravity and moisture content are plotted in this figure merely to show that they do not vary greatly among the groups of material representing various slopes of grain.

TABLE 18.—Average percentage deficiency in strength properties of cross-grained material of various slopes with respect to straight-grained material

[From tests of kiln-dried material with moisture content as shown in fig. 13]

Species of wood and slope of grain	Static bending			Impact bending maximum drop	Compression parallel to grain, maximum crushing strength
	Modulus of rupture	Modulus of elasticity	Work to maximum load		
White ash:					
1:25.....	4	2	9	6	0
1:20.....	6	3	17	12	0
1:15.....	11	4	27	22	0
1:10.....	18	7	43	37	1
1:5.....	36	22	61	59	7
Douglas fir:					
1:25.....	7	4	17	1	-----
1:20.....	10	6	24	4	-----
1:15.....	15	8	34	13	-----
1:10.....	25	14	46	31	-----
1:5.....	54	40	68	65	-----
Sitka spruce:					
1:25.....	2	2	14	8	-----
1:20.....	4	4	21	13	-----
1:15.....	8	7	33	22	-----
1:10.....	17	13	55	45	-----
1:5.....	44	36	76	69	-----
Average:					
1:25.....	4	3	13	5	-----
1:20.....	7	4	21	10	-----
1:15.....	11	6	31	19	-----
1:10.....	19	11	48	38	-----
1:5.....	45	33	68	64	-----

The weakening effect of cross grain results from the wide difference in properties of wood along and across the grain. Cross grain is accompanied by an increased variability of properties, increased checking, and a tendency of the wood to twist and warp.

The data presented on the influence of cross grain are based on tests of clear pieces 2 by 2 inches in cross section, free from checks. In larger sizes, and when other defects are present, checks are apt to be present along with the cross grain, and in such instances greater weakening occurs than in the test results cited. The values given are thus indicative of the minimum effect.

The weakening effect on stress in extreme fiber in bending becomes significant with a slope of about 1 in 20 and increases rapidly with increase in slope. The permissible slope of grain depends on the use to which the wood is put. In general a slope greater than 1 in 20 should not be permitted in a main structural aircraft member. In structural timbers, the permissible slope varies with the grade and with the kind of stress, and ranges from 1 in 20 for high-grade beams to 1 in 8 for low-grade posts.

Cross grain may be of three fundamentally different types as follows:

DIAGONAL GRAIN

This form of deviation of grain is caused by failure to saw parallel to the annual growth layers because of either crooked logs, carelessness in manufacture, or the practice of sawing parallel to the pith instead of parallel to the bark in logs of large taper. Diagonal grain shows on the edge-grain or quarter-sawed face of a board or timber.

SPIRAL GRAIN

This form of deviation of grain results from a corkscrew or spiral rather than vertical arrangement of fibers in a tree. Spiral grain thus refers to the direction of fibers within the annual growth layers and its true direction is evident only on a plain or flat-sawn surface where it is measured by the direction of checks, splits, or other indication of the direction in which the grain runs. Interlocked grain is a special form of spiral grain varying in slope or reversing slope between successive growth periods. An approximation to spiral grain results when a piece is cut so that the grain of the wood on the flat-sawn face is at an angle to the axis.

IRREGULAR GRAIN

This term applies to a more or less irregular wood structure usually accompanying knots, or occasionally appearing as waves in otherwise clear wood.

METHODS OF CALCULATING CROSS-GRAIN

When the grain slopes on both flat-sawn and quarter-sawn faces of a piece these slopes being 1 in a and 1 in b , the resultant or effective slope is given by the expression

$$\frac{\sqrt{a^2+b^2}}{ab};$$

for example, if the slopes are 1 in 12 and 1 in 5 the effective slope is

$$\frac{\sqrt{5^2+12^2}}{5 \times 12} = \frac{13}{60} = 1 \text{ in } 4.6,$$

or if the slopes are both 1 in 20 the effective slope is

$$\frac{\sqrt{20^2+20^2}}{20 \times 20} = \frac{28.3}{400} = 1 \text{ in } 14.1$$

KNOTS

A knot is that portion of a branch which has become incorporated in the body of a tree. The influence on strength is due to the fact that the knot interrupts the continuity and direction of fibers and that the direction of fibers in the knot is essentially at right angles to those in the adjacent wood.

The influence of knots depends on their size, location, shape, and soundness; the kind, size, and proportions of the piece; the kind of stress to which the piece is subjected; and the amount of the attendant cross-grain.

Knots actually increase hardness and strength in compression perpendicular to grain, and are objectionable in regard to these properties only to the extent that they cause nonuniform wear or a nonuniform distribution of pressure at contact surfaces. Knots, however, are harder to work and machine than the surrounding wood, may project from the surface when shrinkage occurs, and also are a cause of twisting.

Knots have relatively little effect on the stiffness of a member. Hence, it is possible to effect some economy by using low-grade material where stiffness is the controlling factor as in joists in small buildings. In such instances the size of the member is usually governed by stiffness, and hence relatively knotty material can be satisfactorily used, although at some sacrifice of bending strength. For example, tests of two 2- by 8-inch by 10-foot joists cut from the same species showed, in pounds per square inch, a modulus of elasticity of 1,100,000 and a modulus of rupture of 5,470 for a practically clear joist and a modulus of elasticity of 1,246,000 and a modulus of rupture of 2,940 for a knotty joist. The slightly higher modulus of elasticity of the knotty joist is attributed to the slightly higher specific gravity of the wood over that of the clear joist.

In a long column, that is, a column in which the length exceeds about 20 times its least dimension, the maximum load depends on the stiffness alone, and knots are consequently less detrimental than in a short column in which the crushing strength of the wood determines the maximum load (32).

Knots have approximately one-half as much effect on compressive as on tensile strength. Hence, for a given percentage reduction in strength larger knots are permissible in a short column than on the tension side of a beam.

Knots are most serious in their effect on the bending strength of beams. The influence of a knot on the tension face is approximately measured by the ratio of the diameter of the knot to the width of the face, the diameter being taken as the distance between lines enclosing the knot and parallel to the edges of the face. Thus, a knot which measures one-fourth the width of the tension face reduces the bending strength 25 percent. The same knot on the compression side of the beam would have about half the influence. Large knots have a somewhat greater influence on the bending strength than is indicated by the foregoing rule, owing to the increased distortion of grain around them. This effect is taken care of in the structural grading rules conforming to American lumber standards (54, 61). The effect of knots is greater in the center half of the length of a beam than near the ends, and is greater near the upper and lower faces than at the center of the height (9).

SHAKES

A shake is a separation of wood along the grain, the greater part of which occurs between or within the rings of annual growth. Shakes can best be detected at the end of the piece where they extend in a general circumferential direction. In structural grading, shakes that appear on an end of a piece are assumed to extend to the center of its length. In beams the principal effect of shakes and one effect of checks is to reduce resistance to horizontal shear or the sliding of the upper on the lower part of the piece. Not only do shakes and checks reduce the area acting in resistance to shear but because of concentration of stress at their extremities the average shearing strength of the remaining area is much less than the shearing strength of unchecked wood as found from shear or torsion tests. These effects are important in large timbers in which the concentration of stress accompanying shakes and/or the checking that usually occurs either prior or subsequent to the placement of timbers in service is sufficient to cause failure at a shearing stress, as averaged over the unchecked area, of

less than half the ultimate value found in standard shear block tests (table 1). The effect of shakes on strength in horizontal shear is appraised in the grading of beams by determining the width of the shake, as measured on the end between lines parallel to the faces, in terms of the width of the piece. For green timbers the allowable shake is the same percentage of the width of the piece as the grade is below an assumed strength for the clear wood (61). Thus, in beams of a grade that permits defects that reduce the strength by one-fourth, the allowable shake would be one-fourth the width of the piece. Shakes tend to increase in size with seasoning. A slightly larger shake is allowable in seasoned material.

CHECKS

A check is a separation along the grain, the greater part of which occurs across the rings of annual growth. Checks other than heart and star checks which occur in green wood and whose cause is unknown occur in seasoning and are due to difference in shrinkage in radial and tangential, or circumferential, directions, and to difference in shrinkage between adjacent parts induced by differences in moisture content. Checks are classed as end checks, heart checks, star checks, surface checks, and through checks. An end check is one at an end of a piece; a heart check is one starting near the pith and extending toward but not to the surface of the piece; a star check consists of a number of heart checks; a surface check is one into a piece from the surface, and a through check is one extending through the piece from one surface to another. Difference between forms of checks need not be considered in determining their effect on strength.

Checks, like shakes, are injurious to beams to the extent that they reduce the area resisting horizontal shear. It is evident that checks in the narrow or horizontal face have practically no effect upon the strength of straight-grained beams. Checks in the wide or vertical faces are most serious in their effect on resistances to horizontal shear when straight and at or near the center of the height.

The effect of checks in beams and columns depends on the area of the longitudinal section they cover, but, unlike shakes, they are not assumed to extend from the end of the piece to the center of the length. The same method of measurement and limitation may be applied as for shakes. If more refinement is desired, however, it may be obtained by estimating the actual reduction of area in a longitudinal plane within that portion of the length extending from the end to a distance three times the depth from the end. The aggregate area of checks permissible within this distance is equal to the width of the allowable shake multiplied by three times the height of the beam (61).

Checks also cause serious weakening in tension perpendicular to grain, but are less injurious in straight-grained members subjected to direct compression or tension along the grain.

Checks are more difficult to prevent in large timbers than in small pieces, and they increase in size and depth with the degree of seasoning during the earlier stages but later close partially or entirely. Checks usually appear first on the ends of a piece, but the development of end checks can be retarded, and in smaller sizes prevented, by the application of an end coating, such as hardened gloss oil prior to seasoning. Season checks form in round timbers because the radial shrinkage differs from the tangential or circumferential.

PITCH POCKETS

Pitch pockets are openings within or between the annual growth rings that contain more or less pitch or bark. Pitch pockets vary greatly in size. Ordinarily, their dimension at right angles to the annual rings is less than one-half inch, whereas they may extend for several inches along the grain (vertically in the tree) and/or in the direction of the annual rings (circumferentially in the tree).

Native species in which pitch pockets are found are the pines, the spruces, Douglas fir, western larch, and tamarack. Pitch pockets in structural timbers ordinarily are not important as (1) their extent is not sufficient to cause significant weakening in shear, (2) they do not cause serious deviations of grain, and (3) they occupy only a small proportion of the cross section of a piece. However, numerous pitch pockets in or close to the same annual growth layer may denote the presence of shakes or may be equivalent in effect to a shake.

In small members the size of the pitch pockets may represent an appreciable portion of the cross section and be located so as to have a marked effect on the strength.

The weakening effect of pitch pockets is more serious when they cause distortion or "dip" of the grain. It is, of course, necessary to limit pitch pockets in aircraft parts, and rules have been established for this purpose (53, 55) but in general they are of importance chiefly because of their effect on appearance.

COMPRESSION FAILURES

A compression failure is a local buckling of the fibers, essentially at right angles to the length, due to excessive compression along the grain. Compression failures appear as wrinkles on the surface of a piece, and range from a well-defined buckling of the fibers visible with the unaided eye to a slight crinkling visible only with a microscope (7, 21, 25).

Compression failures may occur when standing trees are bent severely by wind or snow, when trees are felled over logs or irregularities of the ground, from rough handling of logs or sawed stock, and excessive stresses in service. They weaken the wood in tension, and when on the tension side of a beam produce brash appearing and sudden failures. Material containing compression failures should be rejected for uses in which strength and shock resistance are important, such as in handles and ladder parts. Compression failures are usually so inconspicuous that careful search is necessary to detect them. Often tilting of a piece of wood with respect to the line of vision or source of light will help make them visible. It is seldom possible to detect them in rough-sawn material.

The results of static bending tests on four specimens from a board containing compression failures sufficiently prominent to be readily detected, as compared with the average of uninjured material are given in table 19. These data, while but fragmentary, illustrate the serious reduction in modulus of rupture caused by pronounced compression failures, the even greater reduction in shock resistance as shown by work to maximum load. and the variability in strength properties which they cause.

TABLE 19.—*Results of static bending test on 4 specimens¹ from a board containing prominent compression failures*

Kind of specimen	Specific gravity ²	Moisture content	Modulus of rupture	Work to maximum load
		Percent	Lb. per sq. in.	In.-lb. per cu. in.
Containing compression failures.....	0.53	10.3	5,770	1.44
	.48	11.3	3,050	.59
	.46	11.2	2,510	.38
	.52	11.3	5,830	1.30
Average figures for uninjured material.....	.45	12	10,690	7.8

¹ The bending tests were made on specimens $\frac{3}{4}$ by 2 by 20 inches, using center loading and an 18-inch span. Specimens 1, 2, 3, and 4 were cut so that the compression failures were located at the center of the span.

² Specific gravity based on weight when oven dry and volume when green.

COMPRESSION WOOD

Compression wood, also known as red wood (rotholz), is wood of abnormal growth and structure, slightly above the average in weight, which is usually distinguished by very wide and eccentric annual rings, a lack of contrast between spring and summer wood, and a more or less dark-reddish to brown color. This growth occurs on the under side of limbs and leaning trunks of coniferous trees (16, 21).

Table 20 compares compression wood with normal wood in ponderosa pine, southern yellow pine, and redwood. The values given should not be regarded as the true averages either for normal wood or compression wood, but as indicative of the relationships between the two types. The reason for this is that compression wood varies greatly in degree from material bordering on normal wood to pronounced types. The normal wood represented was cut from the same pieces as the compression wood, and hence was selected to match the latter rather than to be representative of the species.

TABLE 20.—*Strength properties of compression wood compared with normal wood of redwood, ponderosa pine, and southern yellow pine*¹

Average values	Redwood				Ponderosa pine				Southern yellow pine, air-dry	
	Green		Air-dry		Green		Air-dry		Normal wood	Compression wood
	Normal wood	Compression wood	Normal wood	Compression wood	Normal wood	Compression wood	Normal wood	Compression wood		
Specific gravity, based on oven-dry volume.....									0.57	0.66
Shrinkage, longitudinal, green to oven-dry.....percent.....	0.14	1.19			0.21	0.80			.4	2.5
Shrinkage, radial, green to oven-dry.....do.....	2.4								4.6	2.2
Shrinkage, tangential, green to oven-dry.....do.....	4.0								6.2	2.6
Static bending:										
Moisture content.....do.....	114	102	9.9	10.5	133	88	12.0	12.6	11.6	12.4
Specific gravity, based on volume as tested.....	.38	.51	.38	.51	.35	.47	.37	.50		
Fiber stress at proportional limit.....pounds per square inch.....					3,010	3,730	7,250	6,620	8,550	6,520
Modulus of rupture.....do.....	7,310	7,470	10,210	8,890	4,640	6,120	9,840	11,710	11,730	9,000
Modulus of elasticity.....1,000 pounds per square inch.....	1,110	685	1,253	788	1,074	842	1,345	1,019	1,495	994
Work to proportional limit.....inch pounds per cubic inch.....					.47	.94	2.19	.63		
Work to maximum load.....do.....	7.5	6.9	6.0	6.5	4.0	8.8	7.6	15.7	8.2	5.5
Work, total.....do.....					14.4	45.6	10.8	16.2		
Toughness:										
Moisture content.....percent.....	129	89	8.8	9.7	121	85	10.0	10.6		
Specific gravity, based on volume as tested.....	.37	.52	.37	.49	.37	.49	.38	.53		
Toughness per specimen.....inch-pounds.....	83.0	69.5	64.5	64.4	100.7	173.4	79.2	100.4		
Compression parallel to grain:										
Moisture content.....percent.....	126	106	8.6	10.0	138	78	12.1	12.7	11.7	10.8
Specific gravity, based on volume as tested.....	.37	.51	.38	.51	.35	.47	.37	.50	.55	.68
Crushing strength at proportional limit.....pounds per square inch.....	3,950	4,640	7,160	7,250	2,140	2,090				
Maximum crushing strength.....do.....					2,340	3,300	5,210	5,970	7,370	8,100
Modulus of elasticity.....1,000 pounds per square inch.....					1,476	996				

¹ Exact species unknown.

It may be noted that compression wood is characterized by high longitudinal shrinkage, by low stiffness, and for its weight, a general deficiency in most other properties.

When compression wood and wood of normal structure are present in the same piece very high stresses are set up in drying on account of the large difference in longitudinal shrinkage of the two types of wood. This causes bowing or other distortion and may even result in splitting of the piece or in tension failure in the compression wood.

INSECT HOLES

The effect of wormholes on strength is somewhat similar to that of knots or knot holes, except that they do not involve distortion of grain. Inasmuch as wormholes found in lumber usually have only small diameters, occasional ones do not seriously weaken the wood.

In lumber which has been in storage for some time wormholes may be more serious on the interior than is indicated on the surface. This is especially true of the sapwood of ash, oak, hickory, elm, and some other hardwoods that are subject to attack by the powder post beetle (45).

SAP STAIN

Sap stains (blue, red, and yellow) are caused by organisms which germinate in the sapwood, absorbing starches and sugars. Most sap stains, unlike wood-destroying fungi, do not as a rule penetrate the cell walls and consume the wood substance, and therefore sap stain is not in itself so serious from the strength standpoint. However, severe sap stain of certain varieties causes sufficient injury to appreciably reduce the shock resistance or toughness.

Sap stain exerts a marked effect on appearance. Its presence, furthermore, indicates that the wood has been subjected to unfavorable conditions and the possible development of wood-destroying fungi should be considered in the use of such material (17).

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APPENDIX

DETAILS OF TEST PROCEDURE

The information on strength and related properties of woods grown in the United States, which is given in table 1, was obtained from tests in static bending, impact bending, compression parallel to grain, compression perpendicular to grain, hardness, shear parallel to grain, tension perpendicular to grain, and cleavage. Data on weight and shrinkage were also obtained by means of standardized tests. The foregoing 8 tests furnish information on more than 25 different properties of wood.

SELECTION OF MATERIAL

The material for test was identified botanically in the woods, and was brought to the Forest Products Laboratory in the green condition in log form. The logs were generally 4 or 8 feet in length and were usually taken from each of five or

more representative trees of each species, the upper end of the log selected being in most instances 16 feet above the stump. Each 4-foot log or bolt was divided into sticks as shown in figure 14. Insofar as was possible without testing pieces having imperfections that would reduce their strength, the following procedure was followed: A test in compression parallel to the grain was made on a specimen from each stick and a test in static bending on a specimen from one stick of each pair. A pair consists of two tangentially adjacent sticks as N1 and N2, W7 and W8, and so forth. Tests in compression perpendicular to grain were made on specimens cut from one-half the sticks that supplied the static bending specimens, and hardness tests on the other half. Sticks from various

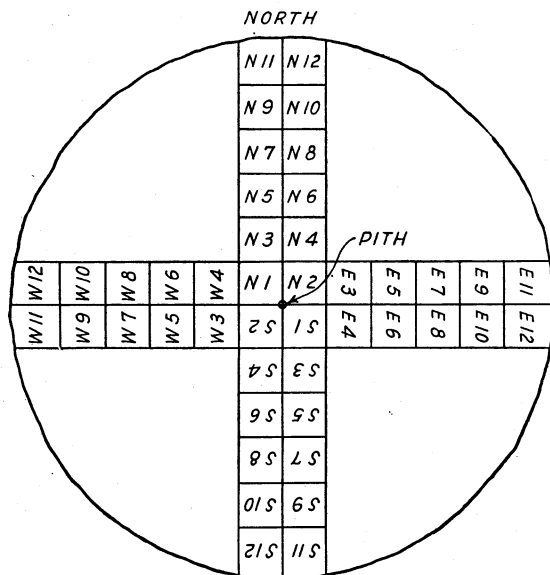


FIGURE 14.—Method of cutting up the bolt and marking the sticks.

parts of the cross section were tested in impact bending, shear, cleavage, and tension perpendicular to grain. This was the system followed when the tree furnished material for tests in the green condition only. For each species from each locality tests were also made on both green and air-dry material from one or more trees. Two adjacent bolts from each of such trees were cut into sticks as indicated by figure 14. Two composite bolts each consisting of one stick from each pair from each of the two adjacent bolts were then formed. The sticks from one composite bolt were tested in the green condition, those from the other after air drying; the assignment of sticks to the various tests being as previously described. This system of division of logs and assignment of sticks provided tests of each kind from various parts of the cross section of the log and afforded for test air-dry material closely matched to that tested in the green condition.

A further feature was the testing in a similar manner of green material taken at various heights above the stump from one or more trees of a number of species. The resulting data are not tabulated herein but are the basis of the discussion of variation of properties with height in tree (p. 40).

TESTING METHODS

The detailed procedure of testing conformed closely to standards of the American Society for Testing Material (4). Specimens for mechanical tests are 2 by 2 inches in cross section and of different lengths, depending on the kind of test. Those for radial and tangential shrinkage are 1 inch thick, 4 inches wide, and 1 inch in length along the grain, the width being radial or tangential according to

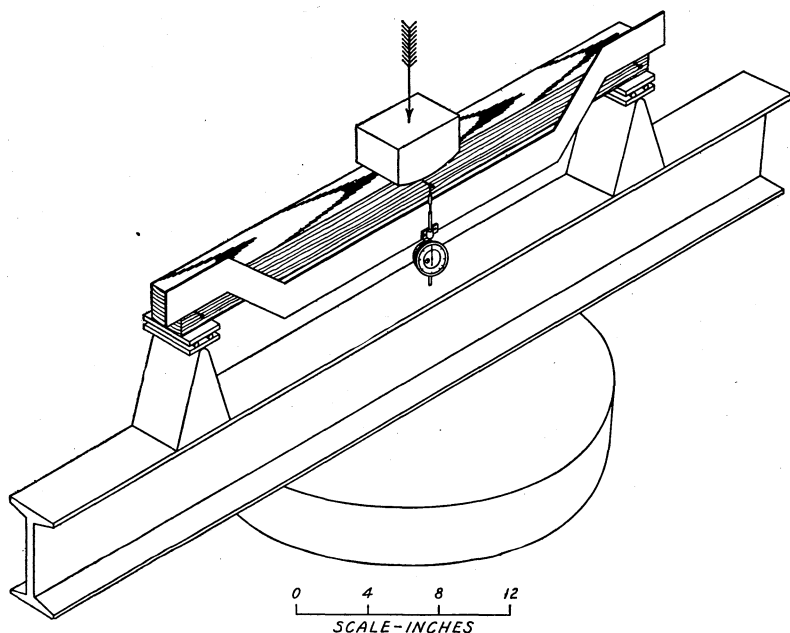


FIGURE 15.—Method of conducting static-bending test.

whether radial or tangential shrinkage is to be measured. Moisture determinations are made on all test specimens.

Only specimens free from knots, cross grain, shakes, checks, and the like were tested. The effects of such characteristics on strength values has been investigated in other tests (9).

A brief outline of the procedure in making each kind of test and of computing the results follows.

DESCRIPTION OF TESTS

STATIC BENDING

In the static-bending test resistance of a beam to slowly applied loads is measured. The specimen is 2 by 2 inches in cross section and 30 inches long and is supported on roller bearings which rest on knife edges placed 28 inches apart (fig. 15). Load is applied at the center of the length through a hard maple block, $3\frac{13}{16}$ inches wide, having a compound curvature. The curvature has a radius of 3 inches over the central $2\frac{1}{8}$ inches of arc, and is joined by an arc of 2 inches radius on each side (fig. 15). The standard placement is with the annual rings of the specimen horizontal. A constant rate of deflection (0.1 inch per minute) is maintained until the beam fails. Load and deflection are read simultaneously at suitable intervals. Figure 16 is a sample data sheet on which such readings are plotted and other information is shown, and figure 17 is a sample computation data card. In figure 16 it may be noted that a line is drawn through the origin parallel to that through the initial points of the curve in order to determine the deflection at proportional limit.

Data on a number of properties are obtained from static-bending tests, the most important of which are stress at proportional limit, modulus of rupture, modulus of elasticity, work to proportional limit, work to maximum load, and total work, discussions of which follow.

STRESS AT PROPORTIONAL LIMIT

As may be noted the first several plotted points in figure 16 are approximately on a straight line showing that the load is proportional to the deflection. As the test progresses, however, the load ceases to increase in direct proportion

U. S. DEPARTMENT OF AGRICULTURE
FOREST SERVICE

Timber Test Log Sheet

Project No. 124Working Plan No. 124Station MADISON Date _____Ship. No. L-315 Stick No. 5-8Laboratory No. 100,831Piece No. 4 Mark dSpecies DOUGLAS FIRKind of test STATIC BENDINGGrade CLEAR

Group _____

Loading CENTERSpan 28 IN.

Distance between collars _____

Width of plate _____

Machine M-1037Speed of mach. 0.105 in. per min.

Weight of hammer _____

Height 2.02 IN.Width 2.00 IN.Length 30.10 IN.

Cross section _____

Weight 1251 G.Rings per inch 9Sap 0 %Summer wood 40 %Seasoning GREENMoisture 31.4 %Kind of failure COMPRESSIONFOLLOWED BY SPLINTERINGTENSION

Remarks _____

Sketch

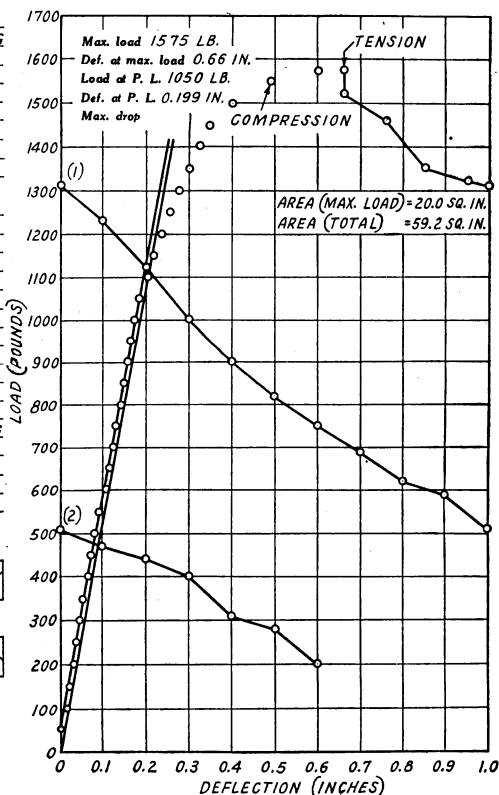
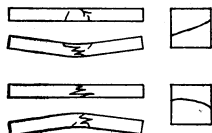


FIGURE 16.—Data sheet for static-bending test.

to the deflection. The point where this occurs, at a load of 1,050 pounds in figure 16, is known as the proportional limit. The corresponding stress in the top and bottom fibers of the beam is the stress at proportional limit.

Using formula 1 on page 98, the stress at proportional limit for the specimen represented by figure 16 is

$$S_{PL} = \frac{3 \times 1050 \times 28}{2 \times 2.00 \times (2.02)^2} = 5,400 \text{ pounds per square inch}$$

MODULUS OF RUPTURE

The modulus of rupture is computed by the same formula as stress at proportional limit, using the maximum load instead of the load at proportional limit. From formula 2 (p. 98), the modulus of rupture of the test specimen of figure 16 is

$$R = \frac{3 \times 1,575 \times 28}{2 \times 2.00 \times (2.02)^2} = 8,110 \text{ pounds per square inch}$$

MODULUS OF ELASTICITY

The modulus of elasticity is determined by the slope of the straight line portion of the load-deflection graph (fig. 16), the steeper the line the higher being the modulus. From formula 3 (p. 98), the modulus of elasticity of the test specimen of figure 16 is

$$E = \frac{1,050 \times (28)^3}{4 \times 2.00 \times (2.02)^3 \times 0.199} = 1,757,000 \text{ pounds per square inch}$$

The value of 0.199 used in this computation is the deflection in inches at the proportional limit.

Form 507.
(Revised January,)

STATIC BENDING

L-315 S-8 CENTER Loading 100831
(Ship No.) (Stake No.) (Lab. No.)
4 d Station MADISON Date AUG. 24, 124
(Piece No.) (Mark.) (Project No.)
Species DOUGLAS FIR Grade CLEAR Seasoning GREEN
Rings 9 Sap 0 Summer wood 40 Moisture 31.4%
Span 28 IN. Length 30.10 IN. Height 2.02 IN. Width 2.00 IN. Weight 1251 G.

SPECIFIC GRAVITY.		F. S. AT P. L.	M. OF R.	M. OF E.	SHEAR.	WORK TO P. L.	WORK TO MAX. LOAD.	TOTAL WORK.	
AS TEST.	OV. DRY.								
<u>0.628</u>	<u>0.478</u>	<u>5410</u>	<u>8120</u>	<u>1756</u>	<u>292</u>	<u>0.92</u>	<u>7.1</u>	<u>20.9</u>	

Rings: Up. $\frac{1}{2}$ Mid. $\frac{1}{2}$ Low. $\frac{1}{2}$

Sum. wood: Up. $\frac{1}{2}$ Mid. $\frac{1}{2}$ Low. $\frac{1}{2}$

Defects

Failure COMPRESSION FOLLOWED BY

SPLINTERING TENSION.

b-1434

MOISTURE
DISTRIBUTION.

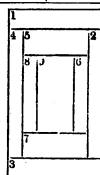
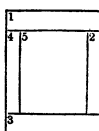


FIGURE 17.—Sample computation card for static-bending test.

WORK TO PROPORTIONAL LIMIT

Work to proportional limit is the product of the average load up to the proportional limit times the deflection at the proportional limit. It is represented by the area under the load-deflection curve from the origin to a vertical line through the abscissae representing the deflection at proportional limit, and is expressed in inch-pounds per cubic inch (fig. 16). From formula 5 (p. 98), the work to proportional limit for the test specimen of figure 16 is

$$W_{PL} = \frac{1,050 \times 0.199}{2 \times 2.00 \times 2.02 \times 28} = 0.92 \text{ inch-pounds per cubic inch}$$

WORK TO MAXIMUM LOAD

The work to maximum load is represented by the area under the load-deflection curve from the origin to the vertical line through the abscissae representing the maximum deflection at which the maximum load is sustained. It is expressed in the same units as work to proportional limit.

From formula 6 (p. 98), the work to maximum load for the test specimen of figure 16 is

$$W_{ML} = \frac{20 \times 200 \times 0.2}{2.00 \times 2.02 \times 28} = 7.1 \text{ inch-pounds per cubic inch}$$

(The area under the curve in the graph reproduced in figure 16 was 20 square inches, and with the scales used in plotting, each square inch represents 200 (pounds) times 0.2 (inch) or 40 inch-pounds.)

TOTAL WORK

The total work is represented by the complete area under the curve from the beginning of the test until it is discontinued. The test is arbitrarily discontinued in this series when the load after attaining its maximum value first decreases to 200 pounds, or when a deflection of 6 inches is reached, whichever occurs first.

From formula 7 (p. 98), the total work for the test specimen of figure 16 is

$$W_T = \frac{59.2 \times 40}{2 \times 2.02 \times 28} = 20.9$$

inch-pounds per cubic inch

The total area under the curve in the original graph represented by figure 16 was 59.2 square inches.

IMPACT BENDING

The impact-bending test is made to determine the resistance of beams to suddenly applied loads. The specimen is 2 by 2 inches in cross section and 30 inches long, and the span is 28 inches. A 50-pound ram or hammer falling between two vertical guides is dropped upon the stick at the center of the span; first from a height of 1 inch, next 2 inches, and so on to 10 inches, then increasing 2 inches at a time until complete failure occurs (fig. 18). A stylus attached to the hammer moves against paper mounted on a revolving drum and records the deflection at each blow, and the position of the specimen when the hammer comes to rest after rebounding. Thus, data are obtained for determining various properties of the wood. Figure 19 is a sample record taken on the

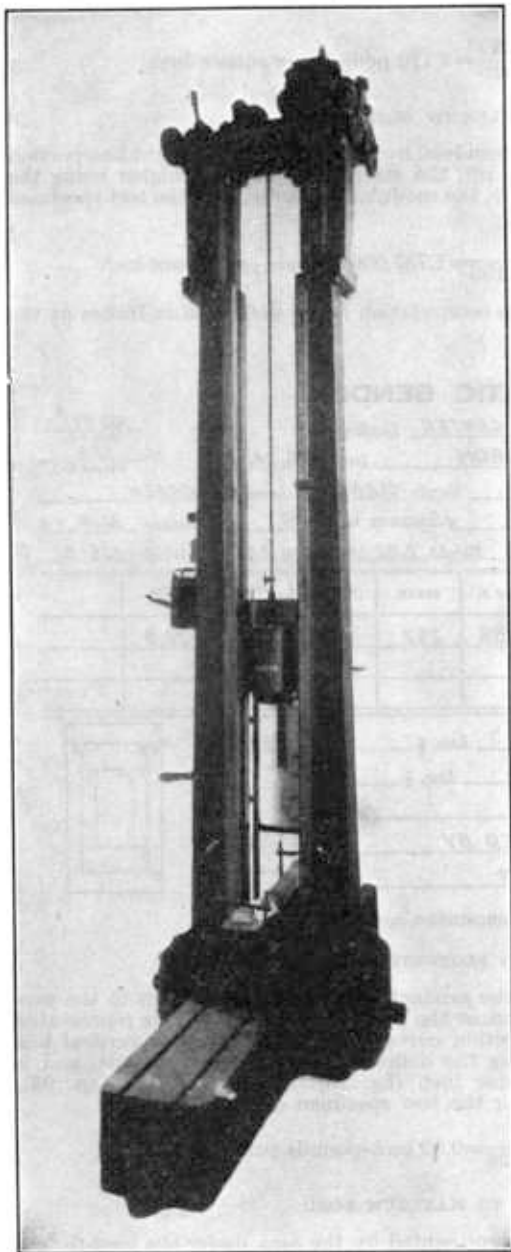


FIGURE 18.—Machine used for impact-bending test.

drum. Figure 20 is a sample computation card, and figure 21 is a sample data sheet on which the test results are plotted to determine the stress at propor-

tional limit and the modulus of elasticity. Other properties on which data are obtained are height of drop in impact bending and work to proportional limit.

STRESS AT PROPORTIONAL LIMIT

In figure 21, height of drop is plotted against the square of the deflection. The first several points are approximately on a straight line, and are used to determine the limit of proportionality. Practically all the factors influencing the test tend to reduce the deflection for a given height of drop, so that after finding the deflection at proportional limit as usual, the head or drop at this deflection is read from a line passing through the origin and the point within the proportional limit which gives this line the least slope. From formula 13 (p. 98), the stress at proportional limit for the specimen represented by figure 21 is

$$S_{PL} = \frac{3 \times 50 \times 7.88 \times 28}{2.00 \times (2.00)^2 \times 0.39} = 10,610 \text{ pounds per square inch}$$

WORK TO PROPORTIONAL LIMIT

The work to proportional limit is equivalent to the energy of the drop that stresses the piece to the proportional limit. From formula 14 (p. 98), the work to the proportional limit for the test specimen of figure 21 is

$$W_{PL} = \frac{50 \times 7.88}{28 \times 2 \times 2} = 3.51 \text{ inch-pounds per cubic inch}$$

HEIGHT OF DROP

The height of drop is recorded as the maximum drop of the hammer causing complete failure of the specimen, or causing a 6-inch deflection. When it is necessary to use a hammer heavier than the 50-pound standard, the height of drop is converted to the equivalent value for a 50-pound hammer.

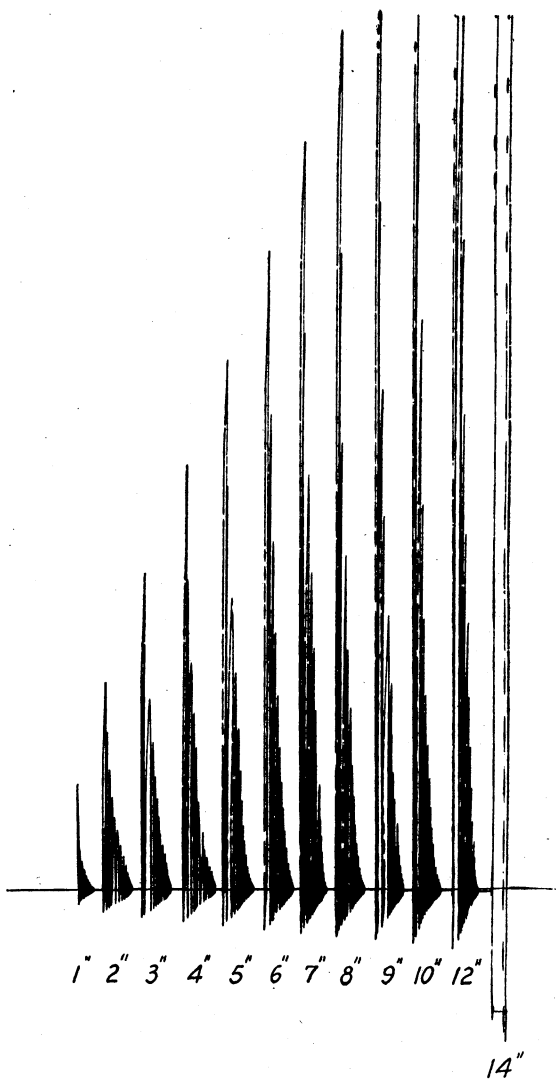


FIGURE 19.—Record taken on the drum of the impact-bending machine in testing northern white pine in a green condition. A maximum drop of 14 inches is recorded.

Form 500 (Revised Jan.,

IMPACT BENDING

L-315 E-12 101151
 (Ship. No.) (Stick No.) (Lab. No.)
1 C Station MADISON Date AUG. 20 124
 (Piece No.) (Mark) (Project No.)
 Species DOUGLAS FIR Grade CLEAR Seasoning GREEN
 Rings 8 Sap 100 % Summer wood 30 % Moisture 61.4
 Hammer 50 lbs. Span 28 IN. Length 29.94 IN. Height 2.00 IN. Width 2.00 IN. Weight 1370 G.

Drop No.	DROP	Def.	(Def.) ²	Set.	Drop No.	DROP	Def.	(Def.) ²	Set.	Sp. Gr. (at test),	
1	1.0	0.13	0.017		11	12.0	0.50	0.250		Sp. Gr. (oven dry),	0.698
2	2.0	0.18	0.032		12	14.0	0.55	0.302		F. S. at P. L.,	0.432
3	3.0	0.22	0.048		13	16.0	0.62	0.384		M. of E.,	10610
4	4.0	0.26	0.068		14	18.0	0.67	0.593		E. Resil.,	1776
5	5.0	0.30	0.090		15					Max. Drop,	22 IN.
6	6.0	0.34	0.116		16					d,	0.010
7	7.0	0.36	0.130		17					H	7.88
8	8.0	0.38	0.144		18					Δ	0.39
9	9.0	0.43	0.185		19						
10	10.0	0.46	0.212		20						

Failure: COMPRESSION FOLLOWED BY SPLINTERING TENSION

FIGURE 20.—Sample computation card for impact-bending test.

8-1431

Timber Test Log Sheet

U. S. DEPARTMENT OF AGRICULTURE
FOREST SERVICE

Project No. 124
 Working Plan No. 124 Station MADISON Date _____ Ship. No. L-315 Stick No. E-12
 Laboratory No. 101151 Piece No. 1 Mark C

Species DOUGLAS FIR
 Kind of test IMPACT BENDING
 Grade CLEAR
 Group _____
 Loading CENTER
 Span 28 IN.
 Distance between collars _____
 Width of plate _____
 Machine M-1036
 Speed of mach. _____ in. per min.
 Weight of hammer 50 LB.
 Height 2.00 IN.
 Width 2.00 IN.
 Length 29.94 IN.
 Cross section _____
 Weight 1370 G.
 Rings per inch 8
 Sap 100 %
 Summer wood 30 %
 Seasoning GREEN
 Moisture 61.4 %
 Kind of failure COMPRESSION
FOLLOWED BY SPLINTERING
TENSION
 Remarks _____

Sketch

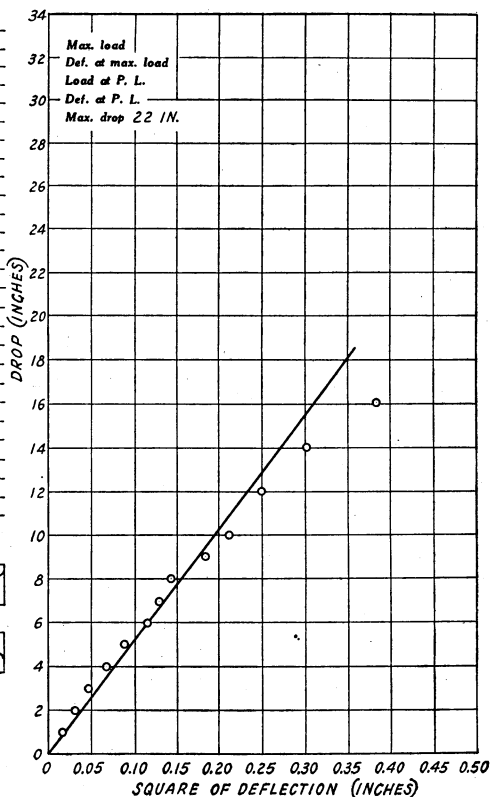
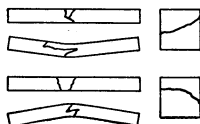


FIGURE 21.—Data sheet for impact-bending test.

COMPRESSION PARALLEL TO GRAIN

In the compression-parallel-to-grain test a 2- by 2- by 8-inch block is compressed in the direction of its length (fig. 22) at a constant rate (0.024 inch per minute). The load is applied through a spherical bearing block, preferably of the suspended self-aligning type, to insure uniform distribution of stress. On some of the specimens, the load and the deformation in a 6-inch central gage length are read simultaneously until the proportional limit is passed. The test is discontinued when the maximum load is passed, and the failure appears. Figure 23 is a sample data sheet on which the test readings are plotted and figure 24 is a sample computation data card.

An alternate form of test specimen has a circular cross section $1\frac{1}{2}$ inches in diameter except at the ends which are left 2 inches square (4). This specimen requires less exacting technic than the square prism, to get good results in testing, but is less simple to prepare.

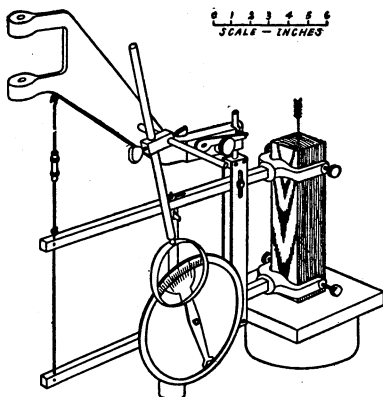


FIGURE 22.—Diagrammatic sketch of compression-parallel-to-grain tests.

Timber Test Log Sheet

U. S. DEPARTMENT OF AGRICULTURE
FOREST SERVICE

Project No. 124
Working Plan No. 124

Station MADISON Date _____

Ship. No. L-315 Stick No. E-3

Laboratory No. 101329

Piece No. 7 Mark d-1

Species DOUGLAS FIR
Kind of test COMP. PAR. TO GR.
Grade CLEAR
Group _____
Loading _____
Span _____
Distance between collars 6 IN.
Width of plate _____
Machine M 1040
Speed of mach. 0.024 in. per min.
Weight of hammer _____
Height _____
Width _____
Length 7.99 IN.
Cross section 1.97 IN. X 2.00 IN.
Weight 287 G.
Rings per inch 8
Sap 0%
Summer wood 26%
Seasoning GREEN
Moisture 28.7%
Kind of failure CRUSHING
NEAR TOP

Remarks _____

Sketch

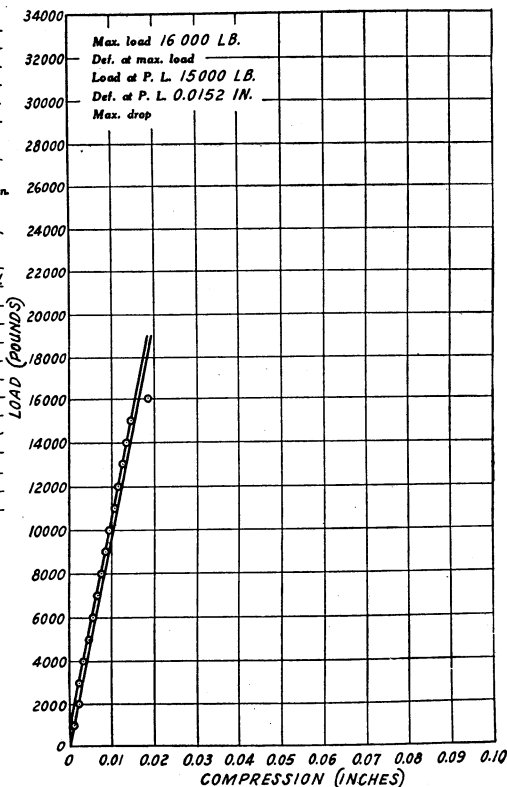
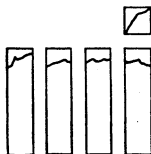


FIGURE 23.—Data sheet for compression-parallel-to grain test.

Form 508
(Revised Nov. 27,

COMPRESSION PARALLEL TO GRAIN

L-315 F-3
(Ship. No.) (Stick No.)101329
(Lab. No.)7 d-1
(Piece No.) (Mark)

Station MADISON Date AUG. 26

124
(Project No.)

Species DOUGLAS FIR Grade CLEAR Seasoning GREEN

Rings 8 Sap 0 % Summer wood 26 % Moisture 28.7 %

Length 7.99 IN. Cross section 1.97 IN. X 2.00 IN. Weight 287 G.

SPECIFIC GRAVITY		MAX. LOAD	CRUSH. ST. AT P. L.	MAX. CRUSH. ST.	M. OF E.	LOAD AT P. L.	DEF. AT P. L.
At Test	Ov. Dry						
0.556	0.432	16000	3810	4060	1502	15000	0.0152

Defects

Failure CRUSHING AT TOP

FIGURE 24.—Sample computation card for compression-parallel-to-grain test.

Data on stress at proportional limit, stress at maximum load (maximum crushing strength), and modulus of elasticity are obtained. The data on modulus of elasticity from this test, however, are not included in table 1.

STRESS AT PROPORTIONAL LIMIT

When the simultaneous readings of load and compression are plotted as in figure 23, the first several points are approximately on a straight line. The point beyond which the compression increases at more rapid rate than the load is the proportional limit, and the accompanying stress is the stress at proportional limit. From formula 15, (p. 98), the stress at proportional limit for the test specimen represented by figure 23 is

$$S_{PL} = \frac{15,000}{1.97 \times 2.00} = 3,810$$

pounds per square inch

MAXIMUM CRUSHING STRENGTH

The maximum crushing strength is computed from the same formula as stress at proportional limit, using the maximum load instead of load at proportional limit. From formula 16, (p. 98), the maximum crushing strength of the test specimen of figure 23 is

$$S_c = \frac{16,000}{1.97 \times 2} = 4,060$$

pounds per square inch

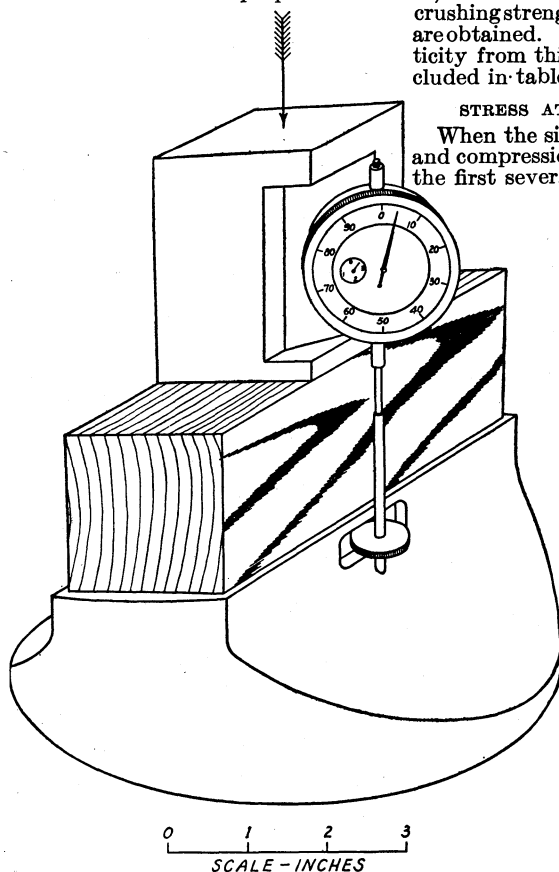


FIGURE 25.—Method of conducting compression-perpendicular-to-grain test.

COMPRESSION PERPENDICULAR TO GRAIN

The specimen for the compression-perpendicular-to-grain test is 2 by 2 inches in cross section and 6 inches long. Pressure is applied through an iron plate 2 inches wide placed across the center of the specimen and at right angles to its length (fig. 27). Hence the plate covers one-third of the surface. The standard placement is with the growth rings vertical. The rate of descent of the movable head of the testing machine is 0.024 inch per minute. Simultaneous readings of load and compression are taken until the test is discontinued at 0.1-inch compression. The principal property determined is the stress at proportional limit. Figure 25 is a sample data sheet and figure 26 a sample computation card for compression-perpendicular-to-grain test.

STRESS AT PROPORTIONAL LIMIT

Figure 25 illustrates a load compression curve. The proportional limit is located as indicated from the straight-line portion of the curve. From formula 18, (p. 98), the stress at proportional limit for the test specimen represented by figure 25 is

$$S_{PL} = \frac{2000}{2 \times 2.01} = 498 \text{ pounds per square inch}$$

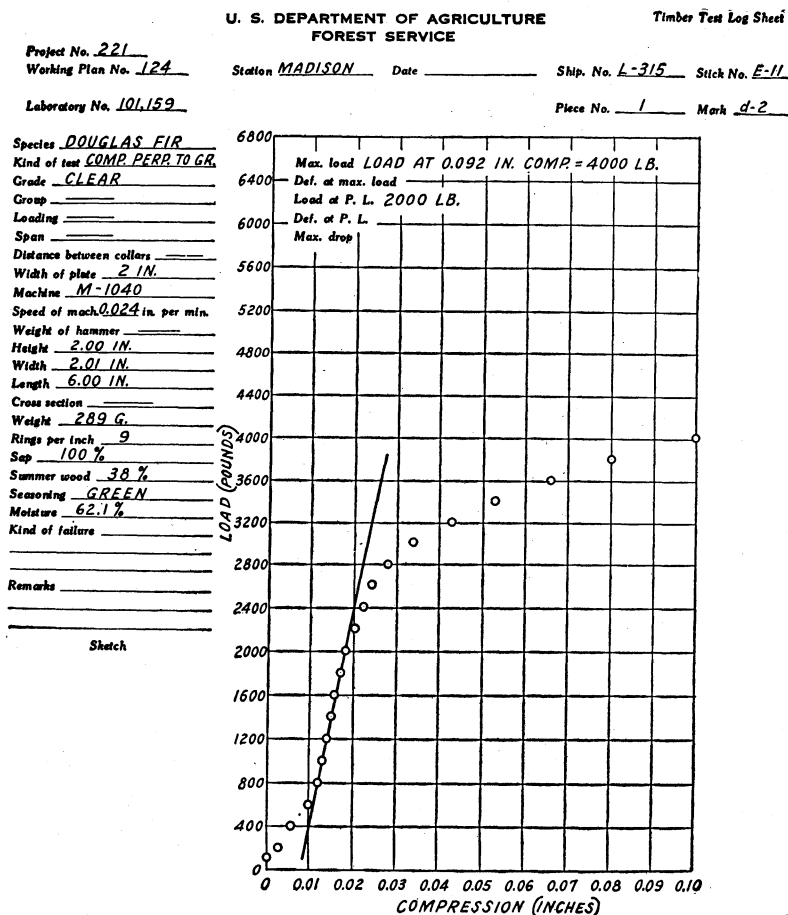


FIGURE 26.—Data sheet for compression-perpendicular-to-grain test.

Form 509
(Revised January,)

COMPRESSION AT RIGHT ANGLES TO GRAIN

101159
(Lab. No.)

1-315 (Ship. No.) E-11 (Stick No.)

1 (Piece No.) d-2 (Mail)

Station MADISON

Date AUG. 20

124
(Project No.)

Species DOUGLAS FIR Grade CLEAR Seasoning GREEN

Rings 9 Sap 100 % Summer wood 38 % Moisture 62.1 %

Width of plate 2 IN. Length 6.00 IN. Height 2.00 IN. Width 2.01 IN. Weight 289 G.

SPECIFIC GRAVITY		LOAD AT P. L.	CRUSH. ST. AT P. L.	$\Delta \div h$	
At Test.	Ov. Dry				
0.732	0.452	2000	498		

FIGURE 27.—Sample computation card for compression-perpendicular-to-grain test.

HARDNESS

Hardness is measured by the load required to embed a 0.444-inch ball (fig. 28) to one-half its diameter in the wood. (The diameter of the ball is such that its projected area is 1 square centimeter). The rate of penetration of the ball is 0.25 inch per minute. Two penetrations are made on each end, two on a radial, and two on a tangential surface of the wood. A special tool makes it easy to determine when the proper penetration of the ball has been reached. The accompanying load is recorded as the hardness value (fig. 29).

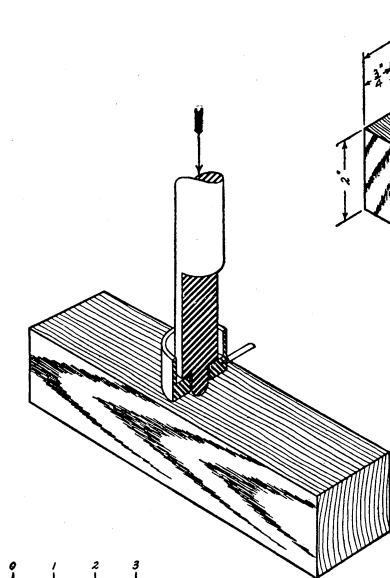


FIGURE 28.—Method of conducting hardness test.

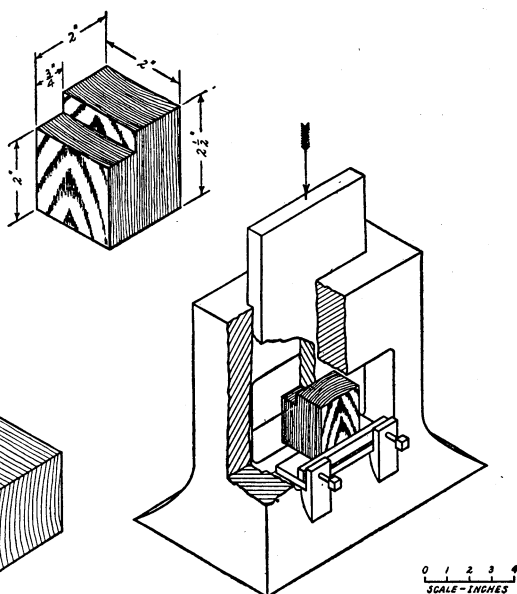


FIGURE 30.—Method of conducting shear-parallel-to-grain test.

SHEAR PARALLEL TO GRAIN

The shearing-parallel-to-grain test is made by applying force to a 2-by-2-inch lip projecting three-fourths of an inch from the side of a block $2\frac{1}{2}$ inches long (fig. 30). The block is placed in a special tool having a plate that is seated on the lip and moved downward at a rate of 0.015 inch per minute. The specimen is supported at the base so that a $\frac{1}{8}$ -inch off-set exists between the outer edge of the support and the inner surface of the plate. The improved shear tool has

Form 501
(Revised Dec. 30,)

HARDNESS

L-315 E-10 101170
 (Ship. No.) (Stick No.) (Lab. No.)
1 d-4 Station MADISON Date AUG. 20 124
 (Piece No.) (Mark) (Project No.)
 Species DOUGLAS FIR Grade CLEAR Seasoning GREEN
 Rings 8 Sap _____ % Summer wood 33 % Moisture 31.7 %
 Length 6.01 IN. Cross section 2.00 IN. x 2.00 IN. Weight 246 G.

	SPECIFIC GRAVITY.		RADIAL SURFACE.	TANGENTIAL SURFACE.	END SURFACE.
	At Test.	Ov. Dry.			
1	0.622	0.472	460	570	525
2			520	460	500
3					510
4					510
AVG.,			490	515	511
AVG. RAD. AND TANG.,			502		

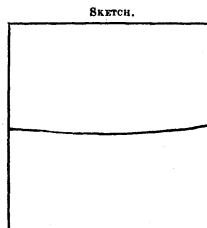


FIGURE 29.—Sample computation card for hardness test.

roller guides to reduce the friction of the plate, and an adjustable seat in the plate to insure uniform lateral distribution of the load.

Specimens are cut so that a radial surface of failure is obtained in some and a tangential surface of failure in others. The property obtained from the shear parallel-to-grain test is the maximum shearing strength.

MAXIMUM SHEARING STRENGTH

The maximum load required to shear off the lip of the specimen is recorded in the test. From formula 19, (p. 99) the maximum shear strength for the test specimen represented by figure 31 is

$$S_s = \frac{3600}{2.02 \times 2.01} = 887 \text{ pounds per square inch}$$

Form 510
(Revised Dec.,)

TANGENTIAL SHEAR

L-315 E-12 101183
 (Ship. No.) (Stick No.) (Lab. No.)
1 C-5 Station MADISON Date AUG. 21 124
 (Piece No.) (Mark) (Project No.)
 Species DOUGLAS FIR Grade CLEAR Seasoning GREEN
 Rings 8 Sap 75 % Summer wood 30 % Moisture 80.0 %

SHEARING AREA	MAX. LOAD	SHEARING STR.	TIME	SKETCH	
<u>2.02 x 2.01</u>	<u>3600</u>	<u>887</u>			

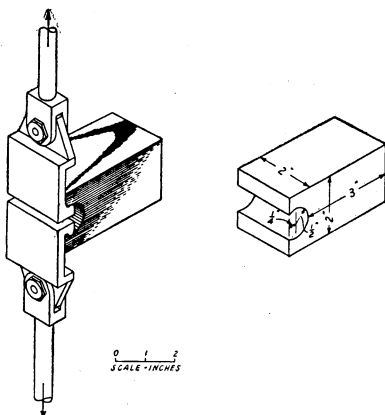
FIGURE 31.—Sample computation card for shear-parallel-to-grain test.

CLEAVAGE

The cleavage test is made to determine the resistance of wood to forces that produce a splitting action. The specimen is 2 by 2 inches in cross section, and 3¾ inches in overall length, with a cleavage section 3 inches long. The forces are applied with special grips as shown in figure 32, the rate of motion of the movable head of the testing machine being 0.25 inch per minute. Tests are made on some specimens cut so as to give a radial surface of failure, and on others cut to give a tangential surface of failure. The value obtained from the cleavage test is the load to cause splitting.

The maximum load causing failure of the specimen is observed. From formula 20 (p. 99), the load to cause splitting, for the specimen represented by figure 33, is

$$S_{CL} = \frac{365}{2.01} = 182 \text{ pounds per inch of width. FIGURE 32.—Method of conducting cleavage test.}$$



Form 503
(Revised Dec.,)

1-315 E-12
(Ship. No.) (Stick No.)

R CLEAVAGE

101177
(Lab. No.)

1 C-3
(Piece No.) (Mark)

Station MADISON

Date AUG. 21

124
(Project No.)

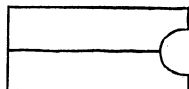
Species DOUGLAS FIR Grade CLEAR Seasoning GREEN

Rings 8 Sap 75% Summer wood 30% Moisture 31.6%

HEIGHT.	WIDTH.	LENGTH.	MAX. LOAD.	LOAD PER INCH WIDTH.
	<u>2.01</u>	<u>2.98</u>	<u>365</u>	<u>182</u>

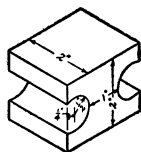
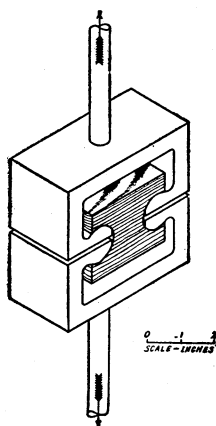


SKETCH.



8-1433

FIGURE 33.—Sample computation card for cleavage test.



TENSION PERPENDICULAR TO GRAIN

The tension-perpendicular-to-grain test is made to determine the resistance of wood across the grain to slowly applied loads. The test specimen is 2 by 2 inches in cross section, and 2½ inches in overall length, with a length at mid-height of 1 inch. The load is applied with the special grips shown in figure 34, the rate of motion of the movable head of the testing machine being 0.25 inch per minute. Some specimens are cut to give a radial, and others to give a tangential surface of failure.

MAXIMUM TENSILE STRENGTH

The maximum tensile strength is the only property evaluated. From formula 21 (p. 99) the maximum tensile strength (perpendicular to the grain) for the specimen represented by figure 35 is

$$S_{TF} = \frac{533}{2.01 \times 0.97} = 273 \text{ pounds per square inch.}$$

FIGURE 34.—Method of conducting tension-perpendicular-to-grain test.

Form 511
(Revised Dec.,)

R TENSION PERPENDICULAR TO GRAIN

L-315 E-12 101189
(Ship. No.) (Stick No.) (Lab. No.)
1 C-6 Station MADISON Date AUG. 21 124
(Piece No.) (Mark) (Project No.)
Species DOUGLAS FIR Grade CLEAR Seasoning GREEN
Rings 8 Sap 80 % Summer wood 30 % Moisture 44.2 %

HEIGHT.	WIDTH.	LENGTH.	MAXIMUM LOAD.	TENSION, Lbs. per sq. in.
	2.01	0.97	533	273

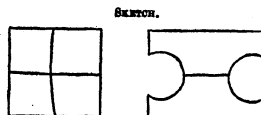


FIGURE 35.—Sample computation card for tension-perpendicular-to-grain test.

TENSION PARALLEL TO GRAIN

The tension-parallel-to-grain test is made to determine the resistance of wood to slowly applied loads acting along the grain. The test specimen is 30 inches long (fig. 1). The specimen is supported by the shoulders near the ends. The rate of motion of the movable head of the testing machine is 0.05 inch per minute. Simultaneous readings of load and of deformation over a 2-inch or 4-inch gage length are taken when it is desired to determine modulus of elasticity.

MAXIMUM TENSILE STRENGTH

From formula 22 (p. 99), the maximum tensile strength parallel to the grain for the specimen represented by figure 36 is

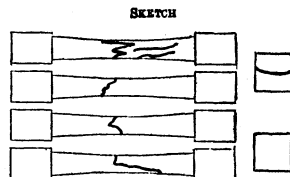
$$S_{TPA} = \frac{2,085}{0.485 \times 0.482} = 8,920 \text{ pounds per square inch.}$$

Form 511-B

TENSION PARALLEL TO GRAIN

1326 L-3 632170
(Ship. No.) (Stick No.) (Lab. No.)
6 a Station MADISON Date 124
(Piece No.) (Mark) (Project No.)
Species LOBLOLLY PINE Grade CLEAR Seasoning GREEN
Rings 6 Sap 100 % Summer wood 35 % Moisture 29.3 %

Cross Section	LENGTH	MAXIMUM LOAD	TENSION Lbs. per sq. in.
0.485 x 0.482"		2085	8920



FAILURE: SPLINTERING TENSION

FIGURE 36.—Sample computation card for tension-parallel-to-grain test.

LINEAR SHRINKAGE

Shrinkage measurements are made to determine the change in dimension with change in moisture content. The test specimen is 1 inch thick, 4 inches wide, and 1 inch in length along the grain. Two specimens are taken from each tree, one for measuring radial shrinkage, the other tangential. The width is measured in the apparatus shown in figure 37, which employs a micrometer reading to 0.001

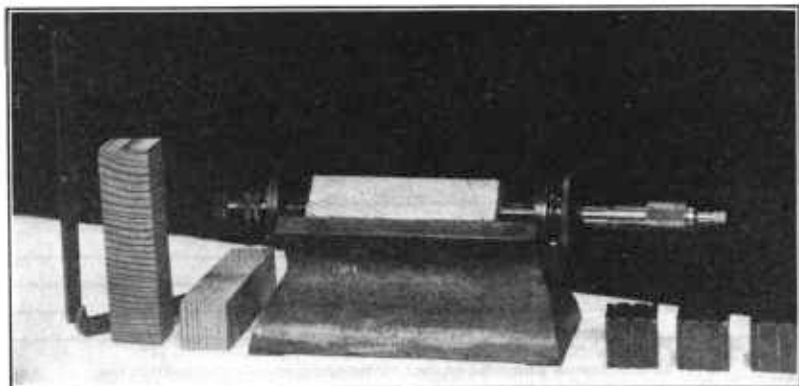


FIGURE 37.—Method of measuring linear shrinkage.

inch. The width of the specimens is measured when green, and after oven drying. In some instances measurements are also taken at intermediate stages of drying.

The linear shrinkage from the green to the oven-dry condition is the original width minus the width when oven-dry, divided by the original width. This ratio is expressed as a percentage.

From formula 23 (p. 99), the radial shrinkage for the specimen represented by figure 38 is

$$F_R = \frac{4.006 - 3.808}{4.006} \times 100 = 4.9 \text{ percent.}$$

Form 541

SHRINKAGE—RADIAL AND TANGENTIAL

101 200

101 199

(LAB. NO.)

124

(PROJECT NO.)

L-315
(SHIP NO.)

(STICK NO.)

STATION—MADISON, WIS.

1
(PIECE NO.)d
(MARK)

SPECIES DOUGLAS FIR

NOMINAL SIZE OF SPECIMEN 1 IN. X 4 IN. X 1 IN.

SEASONING	DATE	RINGS PER INCH	% SAP	% SUM- MER WOOD	WIDTH INCHES	WEIGHT GRAMS	% MOISTURE	% X SHRINKAGE
RADIAL								
GREEN	AUG. 19.	11	30	41	4.006	49.8	66.5	
OVEN-DRY	OCT. 5.				3.808	29.9		4.9
TANGENTIAL								
GREEN	AUG. 19.	12	95	34	4.016	64.0	119.1	
OVEN-DRY	OCT. 5.				3.632	29.2		9.5

X BASED ON GREEN WIDTH

FIGURE 38.—Sample computation card for linear shrinkage measurements.

SHRINKAGE IN VOLUME

Shrinkage-in-volume determinations are made on specimens 2 by 2 inches cross section and 6 inches long. Volume measurements are made by an immersion method (fig. 39). The specimens when oven dry are dipped in hot paraffin before immersion to prevent the absorption of moisture, the oven-dry weight being taken before the paraffin is applied. These final measurements afford data for computing specific gravity based on volume when oven dry.

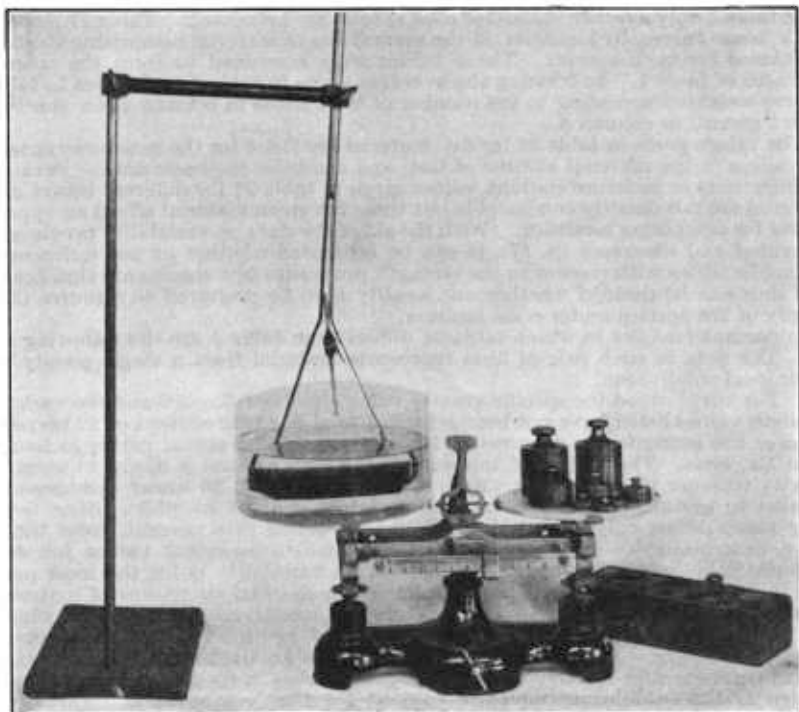


FIGURE 39.—Method of determining volume by means of immersion.

Form 554
(Rev. July,)

SPECIFIC GRAVITY AND VOLUMETRIC SHRINKAGE

SHIP NO. 4-315 STICK NO. S-6 101197
(LAB. NO.)

PIECE NO. 1 MARK d-8 STATION Madison DATE AUG. 20 124
(PROJECT NO.)

SPECIES DOUGLAS FIR

NOMINAL SIZE OF SPECIMEN 2 IN. X 2 IN. X 6 IN. % SAP 0 % SUMMER WOOD 40

	DATE	RINGS PER INCH	WEIGHT, GRAMS	% MOIST	VOLUME C. C.	SPECIFIC GRAVITY	WEIGHT, LBS. PER CU. FT.	X % VOL. SHRINKAGE
GREEN	AUG. 20.	8	253	33.2	398	0.477	39.6	13.8
AIR DRY								
KILN DRY								
OVEN DRY	SEP. 25.		190		343	0.554	34.5	

X BASED ON ORIGINAL VOLUME (GREEN, AIR-DRY, KILN-DRY)
NOTE—USE BACK OF CARD FOR CARBON IMPRESSIONS

REMARKS: _____

1ST WT. _____
2D WT. _____
VOL. _____

FIGURE 40.—Sample computation card for specific gravity and volumetric shrinkage determinations.

From formula 24 (p. 99), the shrinkage in volume for the specimen represented by figure 40 is

$$F_B = \frac{398 - 343}{398} \times 100 = 13.8 \text{ percent.}$$

STRENGTH AND RELATED PROPERTIES, BY LOCALITIES, OF WOODS GROWN IN THE UNITED STATES

In table 1 only average values for each species are presented. Table 21 records the average values, by localities, of the several lots of material comprising the test specimens for each species. These values were combined to form the species averages of table 1. In forming the averages given in table 1 each value in table 21 was weighted according to the number of trees listed in column 5 on the line with "green" in column 4.

The values given in table 21 for dry material are those for the moisture content prevailing in the material at time of test, and comprise the basic data. Because of differences in moisture content, values given in table 21 for different lots of dry material are not directly comparable but those for green material afford an opportunity for comparing localities. With the aid of the data on variability previously presented and discussed (p. 17), it can be estimated whether or not differences among localities with respect to the strength properties of a species are significant and thus can be decided whether one locality is to be preferred as a source of a supply of the species under consideration.

Important features in which table 21 differs from table 1 are the following:

1. The data in each pair of lines represents material from a single county or other local subdivision.

2. For "dry" wood the specific-gravity value given in column 9 and the various strength values listed have not been adjusted to a moisture content of 12 percent as have the corresponding figures in table 1 but are the actual values as found from the tests. The values of moisture content in column 8 apply to specific gravity (column 9) and to the values in columns 24 and 25 under compression parallel to grain. The actual value of moisture content at which other tests were made differs only slightly, usually by a fraction of a percent, from those given in column 8. As may be noted, the moisture-content values for dry material vary over a considerable range. This variability is for the most part due to variations in the conditions under which the various groups of material were dried. These moisture-content values are accordingly not those to which the various species or groups of material would be dried by any one set of drying conditions. Under continued exposure to an unchanging combination of temperature and relative humidity wood reaches a fixed moisture content known as the equilibrium moisture content for that combination. Values of equilibrium moisture content vary only slightly among different species.

NOMENCLATURE OF COMMERCIAL WOODS

The names of lumber used by the trade are not always identical with those adopted as official by the Forest Service. Where the names are not identical some confusion may result. Table 22 has therefore been prepared to show the standard commercial names for softwood lumber as prescribed in American lumber standards and the hardwood lumber names current in the trade together with the corresponding botanical names and official Forest Service names used in this bulletin.

TABLE 22.—Nomenclature of commercial woods

Commercial name	Botanical name	Forest Service name used in this bulletin
HARDWOODS		
Red alder.....	<i>Alnus rubra</i>	Red alder.
White ash.....	<i>Alnus rhombifolia</i>	White alder.
	<i>Fraxinus americana</i>	White ash.
	<i>Fraxinus biltmoreana</i>	Biltmore white ash.
	<i>Fraxinus pennsylvanica lanceolata</i>	Green ash.
	<i>Fraxinus pennsylvanica</i>	Red ash.
	<i>Fraxinus quadrangulata</i>	Blue ash.
Black ash.....	<i>Fraxinus nigra</i>	Black ash.
Oregon ash.....	<i>Fraxinus oregona</i>	Oregon ash.
Aspen.....	<i>Populus tremuloides</i>	Aspen.
	<i>Populus grandidentata</i>	Largetooth aspen.
Basswood.....	<i>Tilia glabra</i>	Basswood.
	<i>Tilia heterophylla</i>	White basswood.
Beech.....	<i>Fagus grandifolia</i>	Beech.
Birch.....	<i>Betula lutea</i>	Yellow birch.
	<i>Betula lenta</i>	Sweet birch.
	<i>Betula nigra</i>	River birch.
Paper birch.....	<i>Betula papyrifera</i>	Paper birch.
	<i>Betula populifolia</i>	Gray birch.
Alaska birch.....	<i>Betula kenaica</i>	Kenai birch.
Buckeye.....	<i>Aesculus octandra</i>	Yellow buckeye.
	<i>Aesculus glabra</i>	Ohio buckeye.
Butternut.....	<i>Juglans cinerea</i>	Butternut.
Catalpa.....	<i>Catalpa speciosa</i>	Hardy catalpa.
Cherry.....	<i>Prunus serotina</i>	Black cherry.
Chestnut.....	<i>Castanea dentata</i>	Chestnut.
	<i>Castanea pumila</i>	Chinquapin.
Chinquapin.....	<i>Castanopsis chrysophylla</i>	Golden chinquapin.
Black cottonwood.....	<i>Populus trichocarpa</i>	Black cottonwood.
	<i>Populus trichocarpa hastata</i>	Northern black cottonwood.
	<i>Populus macdougalii</i>	Maddougal cottonwood.
	<i>Populus fremontii</i>	Cottonwood.
Cottonwood.....	<i>Populus deltoides virginiana</i>	Southern cottonwood.
	<i>Populus heterophylla</i>	Swamp cottonwood.
	<i>Populus balsamifera</i>	Balsam poplar.
	<i>Populus deltoides</i>	Eastern cottonwood.
	<i>Populus sargentii</i>	Cottonwood.
Cucumber.....	<i>Magnolia acuminata</i>	Cucumber magnolia.
Dogwood.....	<i>Cornus florida</i>	Dogwood.
Pacific dogwood.....	<i>Cornus nuttallii</i>	Pacific dogwood.
Rock elm.....	<i>Ulmus racemosa</i>	Rock elm.
Soft elm.....	<i>Ulmus americana</i>	American elm.
	<i>Ulmus fulva</i>	Slippery elm.
Black gum.....	<i>Nyssa sylvatica</i>	Black gum.
	<i>Nyssa biflora</i>	Swamp black gum.
Red gum (heartwood only).....	<i>Liquidambar styraciflua</i>	Red gum.
Sap gum (sapwood only).....	<i>Liquidambar styraciflua</i>	Do.
Hackberry.....	<i>Celtis occidentalis</i>	Hackberry.
	<i>Celtis laevigata</i>	Sugarberry.
Hickory.....	<i>Hicoria ovata</i>	Shagbark hickory.
	<i>Hicoria laciniosa</i>	Bigleaf shagbark hickory.
	<i>Hicoria alba</i>	Mockernut hickory.
	<i>Hicoria glabra</i>	Pignut hickory.
	<i>Hicoria cordiformis</i>	Bitternut hickory.
	<i>Hicoria cordiformis elongata</i>	Do.
Holly.....	<i>Ilex opaca</i>	Holly.
Ironwood.....	<i>Ostrya virginiana</i>	Hophornbeam.
Black ironwood.....	<i>Krugiodendron ferreum</i>	Black ironwood.
Black locust.....	<i>Robinia pseudoacacia</i>	Black locust.
Honeylocust.....	<i>Gleditsia triacanthos</i>	Honeylocust.
Madrono.....	<i>Arbutus menziesii</i>	Pacific madrone.
Magnolia.....	<i>Magnolia grandiflora</i>	Evergreen magnolia.
Hard maple.....	<i>Acer saccharum</i>	Sugar maple.
	<i>Acer nigrum</i>	Black maple.
Soft maple.....	<i>Acer saccharinum</i>	Silver maple.
	<i>Acer rubrum</i>	Red maple.
White maple (unstained sapwood).....	<i>Acer saccharum</i>	Sugar maple.
Oregon maple.....	<i>Acer macrophyllum</i>	Bigleaf maple.
Red oak.....	<i>Quercus borealis maxima</i>	Red oak.
	<i>Quercus borealis</i>	Do.
	<i>Quercus velutina</i>	Black oak.
	<i>Quercus shumardii</i>	Shumard red oak.
	<i>Quercus texana</i>	Texas red oak.
	<i>Quercus palustris</i>	Pin oak.
	<i>Quercus phellos</i>	Willow oak.
	<i>Quercus laurifolia</i>	Laurel oak.
	<i>Quercus rubra</i>	Southern red oak.
	<i>Quercus rubra pagodaefolia</i>	Swamp red oak.
	<i>Quercus nigra</i>	Water oak.
	<i>Quercus ellipsoidalis</i>	Jack oak.
	<i>Quercus coccinea</i>	Scarlet oak.
	<i>Quercus marilandica</i>	Blackjack oak.

TABLE 22.—*Nomenclature of commercial woods*—Continued

Commercial name	Botanical name	Forest Service name used in this bulletin
HARDWOODS—continued		
Red oak.....	<i>Quercus kelloggii</i>	California black oak.
	<i>Quercus catesbaei</i>	Turkey oak.
White oak.....	<i>Quercus alba</i>	White oak.
	<i>Quercus stellata</i>	Post oak.
	<i>Quercus lyrata</i>	Overcup oak.
	<i>Quercus bicolor</i>	Swamp white oak.
	<i>Quercus muehlenbergii</i>	Chinquapin oak.
	<i>Quercus garryana</i>	Oregon white oak.
	<i>Quercus prinus</i>	Swamp chestnut oak.
	<i>Quercus montana</i>	Chestnut oak.
	<i>Quercus macrocarpa</i>	Bur oak.
	<i>Quercus utahensis</i>	Rocky mountain white oak.
Live oak.....	<i>Quercus wislizenii</i>	Highland live oak.
	<i>Quercus agrifolia</i>	Coast live oak.
	<i>Quercus chrysolepis</i>	Canyon live oak.
	<i>Quercus virginiana</i>	Live oak.
Osage-orange.....	<i>Taxylon pomiferum</i>	Osage-orange.
Pecan.....	<i>Hicoria pecan</i>	Pecan.
	<i>Hicoria cordiformis</i>	Bitternut hickory.
	<i>Hicoria cordiformis elongata</i>	Do.
Persimmon.....	<i>Diospyros virginiana</i>	Persimmon.
Sassafras.....	<i>Sassafras varifolium</i>	Sassafras.
Silverbell.....	<i>Halesia carolina</i>	Silverbell.
Sycamore.....	<i>Platanus occidentalis</i>	Sycamore.
Tupelo.....	<i>Nyssa aquatica</i>	Tupelo gum.
Black walnut.....	<i>Juglans nigra</i>	Black walnut.
Willow.....	<i>Salix nigra</i>	Black willow.
Yellow poplar.....	<i>Liriodendron tulipifera</i>	Yellow poplar.
SOFTWOODS		
Alaska cedar.....	<i>Chamaecyparis nootkatensis</i>	Alaska cedar.
Eastern red cedar.....	<i>Juniperus virginiana</i>	Eastern red cedar.
	<i>Juniperus lucayana</i>	Southern red cedar.
	<i>Juniperus mexicana</i>	Mountain cedar.
Incense cedar.....	<i>Libocedrus decurrens</i>	Incense cedar.
Northern white cedar.....	<i>Thuja occidentalis</i>	Northern white cedar.
Port Orford cedar.....	<i>Chamaecyparis lawsoniana</i>	Port Orford cedar.
Southern white cedar.....	<i>Chamaecyparis thyoides</i>	Southern white cedar.
Western juniper.....	<i>Juniperus utahensis</i>	Utah juniper.
	<i>Juniperus pachyphloea</i>	Alligator juniper.
	<i>Juniperus scopulorum</i>	Rocky mountain red cedar.
	<i>Juniperus occidentalis</i>	Western juniper.
Western red cedar.....	<i>Thuja plicata</i>	Western red cedar.
Red cypress (coast type).....	<i>Taxodium distichum</i>	Southern cypress.
Yellow cypress (inland type).....	<i>Taxodium distichum</i>	Do.
White cypress (inland type).....	<i>Taxodium distichum</i>	Do.
Douglas fir.....	<i>Pseudotsuga taxifolia</i>	Douglas fir.
Red fir (intermountain type).....	<i>Pseudotsuga taxifolia</i>	Do.
Red fir (Rocky Mountain type).....	<i>Pseudotsuga taxifolia</i>	Do.
Alpine fir.....	<i>Abies lasiocarpa</i>	Alpine fir.
	<i>Abies arizonica</i>	Corkbark fir.
Balsam fir.....	<i>Abies balsamea</i>	Balsam fir.
	<i>Abies fraseri</i>	Southern balsam fir.
Golden fir.....	<i>Abies magnifica</i>	California red fir.
Noble fir.....	<i>Abies hobbsii</i>	Noble fir.
Silver fir.....	<i>Abies amabilis</i>	Silver fir.
White fir.....	<i>Abies concolor</i>	White fir.
	<i>Abies grandis</i>	Lowland white fir.
Eastern hemlock.....	<i>Tsuga canadensis</i>	Eastern hemlock.
	<i>Tsuga caroliniana</i>	Carolina hemlock.
Mountain hemlock.....	<i>Tsuga mertensiana</i>	Mountain hemlock.
West coast hemlock.....	<i>Tsuga heterophylla</i>	Western hemlock.
Western larch.....	<i>Larix occidentalis</i>	Western larch.
Arkansas soft pine.....	<i>Pinus echinata</i>	Shortleaf pine.
	<i>Pinus taeda</i>	Loblolly pine.
Idaho white pine.....	<i>Pinus monticola</i>	Western white pine.
Jack pine.....	<i>Pinus banksiana</i>	Jack pine.
Loblolly pine.....	<i>Pinus taeda</i>	Loblolly pine.
Lodgepole pine.....	<i>Pinus contorta</i>	Lodgepole pine.
Longleaf pine.....	<i>Pinus palustris</i>	Longleaf pine.
North Carolina pine.....	<i>Pinus taeda</i>	Loblolly pine.
	<i>Pinus echinata</i>	Shortleaf pine.
	<i>Pinus virginiana</i>	Virginia pine.
	<i>Pinus strobus</i>	Northern white pine.
Norway pine.....	<i>Pinus resinosa</i>	Norway pine.
Pond pine.....	<i>Pinus rigida serotina</i>	Pond pine.
Ponderosa pine.....	<i>Pinus ponderosa</i>	Ponderosa pine.
Pondosa pine.....	<i>Pinus ponderosa</i>	Do.

TABLE 22.—Nomenclature of commercial woods—Continued

Commercial name	Botanical name	Forest Service name used in this bulletin
SOFTWOODS—continued		
Shortleaf pine.....	<i>Pinus echinata</i>	Shortleaf pine.
Slash pine.....	<i>Pinus caribaea</i>	Slash pine.
Southern pine.....	<i>Pinus taeda</i>	Loblolly pine.
	<i>Pinus palustris</i>	Longleaf pine.
	<i>Pinus rigida serotina</i>	Pond pine.
	<i>Pinus echinata</i>	Shortleaf pine.
	<i>Pinus caribaea</i>	Slash pine.
	<i>Pinus rigida</i>	Pitch pine.
	<i>Pinus glabra</i>	Spruce pine.
Sugar pine.....	<i>Pinus lambertiana</i>	Sugar pine.
Redwood.....	<i>Sequoia sempervirens</i>	Redwood.
Eastern spruce.....	<i>Picea mariana</i>	Black spruce.
	<i>Picea rubra</i>	Red spruce.
	<i>Picea glauca</i>	White spruce.
Engelmann spruce.....	<i>Picea engelmannii</i>	Engelmann spruce.
	<i>Picea pungens</i>	Blue spruce.
Sitka spruce.....	<i>Picea sitchensis</i>	Sitka spruce.
Tamarack.....	<i>Larix laricina</i>	Tamarack.
Pacific yew.....	<i>Taxus brevifolia</i>	Pacific yew.

FORMULAS USED IN COMPUTING

LEGEND

- S_{CL} = strength in cleavage, pounds per inch of width.
 S_{PL} = stress at proportional limit, pounds per square inch.
 S_{TP} = stress in tension perpendicular to grain, pounds per square inch.
 S_{TPA} = stress in tension parallel to grain, pounds per square inch.
 P' = load at proportional limit, pounds.
 P = maximum load, pounds.
 R = modulus of rupture, pounds per square inch.
 S_s = shear stress, pounds per square inch.
 M = bending moment, in inch-pounds.
 S = computed unit stress, pounds per square inch.

$$I = \text{moment of inertia, inches}^4 \left(\text{for a rectangular beam } I = \frac{b \times d^3}{12} \right).$$

- c = distance from neutral axis of beam to extreme fiber, inches.
 V = total vertical shear at any cross section of a beam, pounds.
 L = length, inches; in static bending, L = span, inches.
 b = breadth, inches.
 d = depth, inches.
 y = deflection, inches.
 b_1 = width of specimen when green, inches.
 b_2 = width of specimen when oven-dry, inches.
 K_1 = volume of specimen when green, cubic inches.
 K_2 = volume of specimen when oven-dry, cubic inches.
 G = specific gravity.
 W = work, inch-pounds per cubic inch.
 W_{PL} = work to proportional limit, inch-pounds per cubic inch.
 W_{ML} = work to maximum load, inch-pounds per cubic inch.
 W_T = total work, inch-pounds per cubic inch.
 E = modulus of elasticity, pounds per square inch.
 A = area under direct stress, square inches.
 H = head or total drop of hammer, plus impact deflection, inches.
 W = weight of hammer, impact bending test, pounds.
 Δ = impact deflection plus static deflection (0.01 inch).
 F_R = radial shrinkage from green to oven-dry condition.
 F_T = tangential shrinkage from green to oven-dry condition.
 F_V = volumetric shrinkage from green to oven-dry condition.

BENDING (SQUARE OR RECTANGULAR BEAMS)

LOAD APPLIED AT CENTER

$$S_{PL} = \frac{3 \times P' \times L}{2 \times b \times d^2} \quad (1)$$

$$R = \frac{3 \times P \times L}{2 \times b \times d^2} \quad (2)$$

$$E = \frac{P' \times L^3}{4 \times b \times d^3 \times y} \quad (3)$$

$$S_s = \frac{3 \times P}{4 \times b \times h} \quad (4)$$

$$W_{PL} = \frac{P' y}{2 \times b \times d \times L} \quad (5)$$

$$W_{ML} = \frac{\text{area under curve to maximum load in inch-pounds}}{b \times d \times L} \quad (6)$$

$$W_T = \frac{\text{total area under curve in inch-pounds}}{b \times d \times L} \quad (7)$$

UNIFORMLY DISTRIBUTED LOAD

$$S_{PL} = \frac{3 \times P' \times L}{4 \times b \times d^2} \quad (8)$$

$$R = \frac{3 \times P \times L}{4 \times b \times d^2} \quad (9)$$

$$E = \frac{5 \times P' \times L^3}{32 \times b \times d^3 \times y} \quad (10)$$

ANY LOADING

$$M = \frac{SI}{c} \quad M_{max} = \frac{RI}{c} \quad (11)$$

$$S_s = \frac{3 \times V}{2 \times b \times d} \quad (12)$$

IMPACT BENDING

$$S_{PL} = \frac{3WHL}{bd^2\Delta} \quad (13)$$

$$W_{PL} = \frac{WH}{Lbh} \quad (14)$$

COMPRESSION PARALLEL TO GRAIN

$$S_{PL} = \frac{P'}{A} \quad (15)$$

$$S_c = \frac{P}{A} \quad (16)$$

$$E = \frac{P'L}{Ay} \quad (17)$$

COMPRESSION PERPENDICULAR TO GRAIN

$$S_{PL} = \frac{P'}{A}, \text{ where } A = \text{area of specimen under plate, square inches} \quad (18)$$

SHEAR PARALLEL TO GRAIN

$$S_s = \frac{P}{A}, \text{ where } A = \text{area under shear, square inches} \quad (19)$$

CLEAVAGE PARALLEL TO GRAIN

$$S_{CL} = \frac{P}{b} \quad (20)$$

TENSION PERPENDICULAR TO GRAIN

$$S_{TP} = \frac{P}{A} \quad (21)$$

TENSION PARALLEL TO GRAIN

$$S_{TPA} = \frac{P}{A} \quad (22)$$

LINEAR SHRINKAGE (PERCENT)

$$F_R \text{ or } F_T = \frac{b_1 - b_2}{b_1} \times 100 \quad (23)$$

VOLUMETRIC SHRINKAGE (PERCENT)

$$F_V = \frac{K_1 - K_2}{K_1} \times 100 \quad (24)$$

SPECIFIC GRAVITY

$$G = \frac{\text{weight in grams}}{\left(1 + \frac{\text{percent moisture}}{100}\right) \times \text{volume in cubic centimeters}} \quad (25)$$

TABLE 21.—Strength and related properties, by localities, of woods grown in the United States

Ship- ment no.	Species (common and botanical names)	Place of growth of material tested	Moisture condition	Trees tested	Rings per inch	Summer wood	Moisture con- tent	Specific gravity, oven- dry, based on volume—		Weight per cubic foot	Shrinkage from green to oven- dry condition based on dimen- sions when green			Static bending						Impact bending			Compression parallel to grain		Com- pression perpen- dicular to grain; stress at proportional limit	Hardness; load required to em- bed a 0.444-inch ball to ½ its diameter		Shear parallel to grain; maxi- mum shearing strength	Cleav- age; load to cause splitting	Tension perpen- dicular to grain; maxi- mum tensile strength
								At test	When oven- dry		Volu- metric	Rad- ial	Tang- ential	Stress at pro- portional limit	Modu- lus of rupture	Modu- lus of elastic- ity	Work			Stress at pro- portional limit	Work to proportional limit	Height of drop causing complete failure (50-pound hammer)	Stress at pro- portional limit	Maxi- mum crushing strength		End	Side			
																	Proportional limit	Maximum load	Total											
1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31
	HARDWOODS			Num- ber	Num- ber	Per- cent	Per- cent			Pounds	Per- cent	Per- cent	Per- cent	Lb. per sq. in.	Lb. per sq. in.	1,000 lb. per sq. in.	In.-lb. per cu. in.	In.-lb. per cu. in.	In.-lb. per cu. in.	Lb. per sq. in.	In.-lb. per cu. in.	Inches	Lb. per sq. in.	Lb. per sq. in.	Lb. per sq. in.	Pounds	Pounds	Lb. per sq. in.	Lb. per sq. in.	Lb. per sq. in.
263	Alder, red (<i>Alnus rubra</i>)	Snohomish County, Wash.	Green	6	10.8		98.2	0.368	0.434	46	12.6	4.4	7.3	3,750	6,540	1,167	0.70	8.0	15.3	8,040	2.6	22	2,620	2,960	313	554	440	770	217	394
			Dry	2			8.6	.418						8,140	10,850	1,435	2.37	8.5	9.8	13,040	5.9	20	5,290	7,050	651	1,170	652	1,210	284	431
746	Apple (<i>Malus pumila</i> var.)	Botetourt County, Va.	Green	10	5.8		46.5	.606	.745	55	17.6	5.6	10.1	3,640	7,400	1,047	.75	15.7	36.4	7,590	3.0	33	1,990	3,000	814	1,041	1,088	1,636	476	905
			Dry	4			10.2	.678						6,770	13,020	1,277	2.40	23.3	44.0	16,850	8.6	44	3,340	6,690	1,418	2,300	1,800	1,764		
257	Ash, biltmore white (<i>Frazinus biltmoreana</i>)	Overton County, Tenn.	Green	5	16.6	49	41.5	.507	.584	45	12.6	4.2	6.9	5,530	9,270	1,335	1.31	11.6	27.4	11,930	4.9	30	3,530	3,980	875	953	853	1,232	343	536
			Dry	1			5.4	.573						12,110	15,560	1,760	4.60	11.7	17.2	19,850	10.4	46	7,360	10,370	2,020	2,060	1,300	1,972	452	813
219	Ash, black (<i>Frazinus nigra</i>)	Ontonagon County, Mich.	Green	6	23.1	53	90.6	.447	.526	53	15.2	5.0	7.8	2,610	6,000	1,107	.42	11.3	28.9	7,230	2.5	35	1,720	2,340	409	610	552	866	266	490
			Dry	1			9.1	.500						10,340	16,130	1,975	2.05	17.9	43.6			44	6,620	8,190	1,270	1,338	994	1,794	397	760
5	do.	Marathon County, Wis.	Green	1	25.0		78.9	.456		51				2,580	6,000	967	.40	13.1	35.0			30	2,260	452	565	490	854	292	490	
			Dry	5			11.6	.497						6,310	11,620	1,395	1.64	13.1	27.3	12,160	5.3	27	3,950	5,590	893	1,101	792	1,600	402	696
222	Ash, blue (<i>Frazinus quadrangulata</i>)	Bourbon County, Ky.	Green	5	12.5	49	39.3	.532	.603	46	11.7	3.9	6.5	5,700	9,650	1,241	1.47	14.7	38.2	11,140	5.0	43	3,580	4,180	994	1,140	1,028	1,544	353	584
			Dry	1			9.6	.575						8,720	14,770	1,433	3.00	14.3	30.1	20,550	10.5	42	5,940	7,740	1,911	1,833	1,556	2,153	454	
75	Ash, green (<i>Frazinus pennsylvanica lanceolata</i>)	Richland Parish, La.	Green	5	20.6	60	47.4	.516	.590	47	11.7			4,450	8,880	1,319	.87	10.6	23.7	11,720	5.6	32	3,240	4,040	801	842	732	1,202	349	614
			Dry	1			11.2	.552						8,950	13,680	1,615	2.82	12.6	23.0	15,850	7.3	32	5,150	7,300	1,232	1,676	1,153	1,832	372	712
223	do.	New Madrid County, Mo.	Green	5	13.7	56	48.3	.536	.631	50	13.3	4.6	7.1	6,110	10,040	1,480	1.42	13.0	31.6	11,150	4.4	37	3,890	4,360	1,012	1,073	1,007	1,318	345	564
			Dry	1			9.6	.590						9,970	16,110	1,768	3.18	14.6	22.4	18,900	9.0	30	5,520	7,850	2,220	1,870	1,362	2,336	564	740
318	Ash, Oregon (<i>Frazinus oregona</i>)	Lane County, Oreg.	Green	3	12.5	63	48.5	.497	.575	46	13.2	4.1	8.1	4,230	7,570	1,132	.92	12.2	33.3	8,920	3.0	39	2,760	3,510	653	851	790	1,191	309	587
			Dry	2			8.4	.566						7,960	14,540	1,426	2.58	15.1	20.0	15,000	6.2	31	4,610	7,120	1,998	1,666	1,236	2,092	461	775
223	Ash, pumpkin (<i>Frazinus profunda</i>)	New Madrid County, Mo.	Green	3	21.0	46	51.4	.485	.551	46	12.0	3.7	6.3	4,470	7,600	1,043	1.08	9.4	18.4	8,760	3.7	31	2,850	3,360	989	885	752	1,214	357	574
			Dry	1			9.6	.526						6,980	11,810	1,312	2.11	7.8	14.8	15,150	7.9	22	4,220	6,320	1,995	1,535	1,026	1,893	459	828
101	Ash, white (<i>Frazinus americana</i>)	Stone County, Ark.	Green	5	14.8	51	38.2	.550	.640	47	12.6	4.3	6.4	5,180	9,920	1,416	1.10	13.3	34.3	11,710	4.9	33	3,450	4,220	889	1,121	1,008	1,336	340	658
			Dry	1			10.5	.620						10,770	17,650	1,942	3.42	16.8	30.9	14,920	6.5	34	6,240	7,900	1,315	2,065	1,416	2,215	588	868
214	Ash, white (second growth) (<i>Frazinus americana</i>)	Oswego County, N. Y.	Green	5	8.8	63	40.3	.582	.708	51	14.0	5.3	8.7	6,140	10,760	1,635	1.30	16.3	41.9	13,780	5.9	47	3,960	4,610	794	1,145	1,083	1,604	440	787
			Dry	1			9.5	.637						13,010	18,650	1,985	4.50	17.0	36.9	23,840	12.8	46	8,060	9,420	2,090	2,240	1,680	2,525	471	1,128
256	Ash, white (<i>Frazinus americana</i>)	Pocahontas County, W. Va.	Green	5	17.2	49	48.1	.495	.565	46	12.2	4.1	6.6	4,600	8,310	1,285	.96	13.6	32.4	11,620	5.1	37	2,870	3,390	705	872	785	1,183	336	590
			Dry	1			6.9	.554						9,580	15,960	1,684	3.16	13.4	25.9	19,050	11.0	38	5,540	8,450	1,762	1,833	1,224	2,008	316	897
904	Ash, white (second growth) (<i>Frazinus americana</i>)	Bennington County, Vt.	Green	8	10.2	52	40.4	.556	.639	49	13.9	5.5	9.1	4,720	9,340	1,482	.88	20.8	51.6	16,780	6.9	35	2,750	3,840	842	940	968	1,392	258	430
		Hampshire County, Mass.	Dry	2			11.9	.611						7,740	15,500	1,762	1.97	21.6	39.2	18,280	7.2	50	5,720	7,520	1,364	1,635	1,312	1,877	556	1,090
300	Aspen (<i>Populus tremuloides</i>)	Rusk County, Wis.	Green	5	8.5		106.6	.360	.422	46	11.1	3.3	6.9	2,940	5,280	840	.65	6.9	16.0	6,880	2.5	28	1,600	2,160	203	266	318	625	116	182
			Dry	1			5.2	.421						7,600	10,770	1,290	2.43	7.3		10,470	4.0	24	4,320	6,440	552	848	420	890	224	380
465	do.	San Miguel County, N. Mex.	Green	6	7.3		84.3	.344	.383	40	11.8	3.6	6.6	3,340	5,000	877	.73	5.9	11.2	7,010	2.8	18	1,720	2,130	239	289	286	683	152	276
			Dry	2			7.0	.372						7,150	10,560	1,410	2.07	9.1	13.5	9,680	3.6	18		5,520	718	567	338	1,023		239
211	Aspen, largetooth (<i>Populus grandidentata</i>)	Sauk County, Wis.	Green	5	8.2		96.4	.354	.412	4																				

TABLE 21.—*Strength and related properties, by localities, of woods grown in the United States—Continued*

Ship- ment no.	Species (common and botanical names)	Place of growth of material tested	Moisture condition	Trees tested	Rings per inch	Sum- mer wood	Mois- ture con- tent	Specific gravity, oven dry, based on volume—		Weight per cubic foot	Shrinkage from green to oven- dry condition based on dimen- sions when green			Static bending						Impact bending			Compression parallel to grain		Com- pression perpen- dicular to grain; stress at proportional limit	Hardness; load required to em- bed a 0.444-inch ball to ½ its diameter		Shear parallel to grain; maxi- mum shearing strength	Cleav- age; load to cause splitting	Tension perpen- dicular to grain; maxi- mum tensile strength	
								At test	When oven- dry		Volu- metric	Ra- dial	Tan- gen- tial	Stress at pro- portional limit	Modu- lus of rupture	Modu- lus of elas- ticity	Work			Stress at proportional limit	Work to proportional limit	Height of drop causing complete failure (50-pound hammer)	Stress at proportional limit	Maxi- mum crushing strength		End	Side				
																	Proportional limit	Maxi- mum load	Total												
1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31	
	HARDWOODS—continued			Number	Number	Percent	Percent			Pounds	Percent	Percent	Percent	Lb. per sq. in.	Lb. per sq. in.	1,000 lb. per sq. in.	In.-lb. per cu. in.	In.-lb. per cu. in.	In.-lb. per cu. in.	Lb. per sq. in.	In.-lb. per cu. in.	Inches	Lb. per sq. in.	Lb. per sq. in.	Lb. per sq. in.	Pounds	Pounds	Lb. per sq. in.	Lb. per in. of width	Lb. per sq. in.	
752	Bustic (<i>Diphotis salicifolia</i>)	Dade County, Fla.	Green	1				0.861		77				5,790	12,360	1,860	1.00				18,430	6.6	26	3,750	5,330	1,700					
211	Butternut (<i>Juglans cinerea</i>)	Sauk County, Wis.	Dry	1	9.1		11.8	0.354	0.397	45	9.4	3.6	5.7	8,540	11,040	1,321	2.99	7.2	8.5	22.5	6,990	2.3	21	2,130	2,580	258	414	394	762	225	419
226	do	Sevier County, Tenn.	Dry	5	9.0		8.0	0.382						2,610	4,880	931	.43	7.9	19.9	7,700	2.7	26	1,910	2,250	287	400	379	750	225	443	
752	Buttonwood (<i>Conocarpus erecta</i>)	Dade County, Fla.	Green	1			7.3	0.401	.410	47	11.1	3.0	6.5	6,000	7,620	1,202	1.75	9.3	21.3	11,960	4.7	26	5,240	6,180	749	677	542	1,312	200	472	
318	Cascara (<i>Rhamnus purshiana</i>)	Lane County, Oreg.	Dry	7			12.7	0.694	.851	64	14.6	5.4	8.5	4,560	7,440	1,192	1.00	6.2	15.6	14,060	6.8	40	3,050	4,110	1,139	1,079	1,111	1,222	366	471	
1054	Catalpa, hardy (<i>Catalpa speciosa</i>)	Henry County, Ind.	Green	4	16.7		61.1	0.496	.548	50	7.6	3.2	4.6	6,480	9,820	1,525	1.58	6.0						7,560	1,549						
1054	do	Hancock County, Ind.	Dry	10	8.2	58	72.3	0.370	.414	40	7.4	2.7	4.9	3,360	6,320	631	1.04	13.4	49.7	8,690	3.6	58	1,890	3,270	670	679	731	1,152	260	514	
197	Cherry, black (<i>Prunus serotina</i>)	Potter County, Pa.	Green	10			11.2	0.404						4,500	9,060	1,213	.99	8.2	13.9	11,470	4.6	25	2,460	4,990	568	659	520	1,142	286	555	
226	Cherry, pin (<i>Prunus pennsylvanica</i>)	Sevier County, Tenn.	Dry	5	9.3	58	69.8	0.396	.436	42	7.1	2.2	4.8	2,980	5,630	908	.56	10.0	35.8	7,730	3.0	42	1,480	2,530	377	443	432	738	258	472	
226	Chestnut (<i>Castanea dentata</i>)	do	Green	5			11.5	0.428						5,410	10,410	1,259	1.32	12.3	31.0	10,560	4.1	32	3,560	5,590	572	648	626	1,156	326	570	
245	do	Baltimore County, Md.	Dry	5	10.6		54.8	0.471	.534	45	11.5	3.7	7.1	4,180	8,030	1,308	.80	12.8	31.8	10,180	4.1	33	2,940	3,540	444	754	664	1,127	330	574	
318	Chinquapin, golden (<i>Castanopsis chrysophylla</i>)	Lane County, Oreg.	Green	1			9.2	0.514						11,000	13,820	1,544	4.48	11.0	11.4	14,780	5.9	28	7,030	8,370	1,024	1,690	1,030	1,928	354	558	
368	Cottonwood, eastern (<i>Populus deltoides</i>)	Pemiscot County, Mo.	Dry	5	5.8		45.7	0.361	.425	33	12.8	2.8	10.3	2,880	5,040	1,042	.47	6.2	18.3	6,580	2.1	22	1,810	2,170	265	435	386	678	174	296	
263	Cottonwood, northern black (<i>Populus trichocarpa hastata</i>)	Snohomish County, Wash.	Green	2			6.6	0.408						7,760	10,700	1,396	2.54	9.3	28.9	12,100	5.2	36	5,520	6,490	700	930	579	1,241	261	338	
226	Dogwood (<i>Cornus florida</i>)	Sevier County, Tenn.	Dry	5	11.8	51	133.4	0.338	.449	57	12.9	3.4	6.8	2,840	5,230	910	.53	6.7	14.8	7,870	3.0	24	1,890	2,230	366	493	402	749	234	398	
318	Dogwood, Pacific (<i>Cornus nuttallii</i>)	Lane County, Oreg.	Green	5	9.4	46	109.7	0.404	.459	53	10.4	3.3	6.6	6,900	9,260	1,255	2.25	6.3	13.0	11,800	6.0	19	4,380	6,440	820	780	546	1,192	260	481	
368	Cottonwood, eastern (<i>Populus deltoides</i>)	Pemiscot County, Mo.	Dry	5			8.6	0.439						3,270	6,010	949	.65	7.4	19.2	8,000	2.6	23	2,260	2,710	400	571	448	845	246	471	
263	Cottonwood, northern black (<i>Populus trichocarpa hastata</i>)	Snohomish County, Wash.	Green	5			8.6	0.447						7,940	10,100	1,405	2.57	6.5	10.0	11,280	4.7	18	4,560	6,800	1,032	773	612	1,138	238	450	
226	Dogwood (<i>Cornus florida</i>)	Sevier County, Tenn.	Dry	5	14.8		133.7	0.417	.483	61	13.2	4.6	7.4	4,250	7,030	1,016	1.09	9.5	20.4	8,820	3.4	31	2,030	3,020	491	733	602	1,014	234	477	
318	Dogwood, Pacific (<i>Cornus nuttallii</i>)	Lane County, Oreg.	Green	2			4.8	0.494						11,900	14,060	1,412	6.14	9.5	18.3	12,520	6.0	29	6,380	7,970	850	924	834	1,464			
368	Cottonwood, eastern (<i>Populus deltoides</i>)	Pemiscot County, Mo.	Dry	5	5.6		111.4	0.372	.433	49	14.1	3.9	9.2	4,250	7,030	1,016	1.09	9.5	20.4	8,820	3.4	31	2,030	3,020	491	733	602	1,014	234	477	
263	Cottonwood, northern black (<i>Populus trichocarpa hastata</i>)	Snohomish County, Wash.	Green	1			4.7	0.429						8,610	11,420	1,637	2.61	7.4	21.2	7,460	2.4	19	5,320	7,830	734	744	484	1,116	313	698	
226	Dogwood (<i>Cornus florida</i>)	Sevier County, Tenn.	Dry	5	5.6		131.6	0.315	.368	46	12.4	3.6	8.6	2,860	4,830	1,073	.44	5.0	12.7	6,820	2.2	20	1,760	2,160	204	277	253	602	170	274	
318	Dogwood, Pacific (<i>Cornus nuttallii</i>)	Lane County, Oreg.	Green	1			8.5	0.358						6,180	9,560	1,312	1.62	7.2	10.4	10,860	4.4	22	3,920	5,440	457	666	356	1,156	234	344	
368	Cottonwood, eastern (<i>Populus deltoides</i>)	Pemiscot County, Mo.	Dry	5	24.1		61.6	0.638	.796	64	19.9	7.1	11.3	4,820	8,790	1,175	1.11	21.0	49.1	7,090	3.5	58		3,640	1,033	1,413	1,408	1,516			
226	Dogwood (<i>Cornus florida</i>)	Sevier County, Tenn.	Green	5			7.5	0.774						11,770	18,340	1,697	4.63	18.9	35.6	19,320	10.1	40	6,040	10,200	2,466	2,983	2,532		635		
318	Dogwood, Pacific (<i>Cornus nuttallii</i>)	Lane County, Oreg.	Dry	5	21.4		52.3	0.578	.701	55	17.2	6.4	9.6	4,220	8,210	1,090	.92	17.0	38.7	9,820	3.6	56	2,410	3,640	872	1,140	979	1,298	335	736	
319	Elder, blueberry (<i>Sambucus coerulea</i>)	Douglas County, Oreg.	Dry	4			5.3	0.682						10,090	12,150	1,755	3.26	8.5	52.6	10,960	3.8	26	5,930	11,310	2,468	2,510	1,644	2,056	470		
5	Elm, American (<i>Ulmus americana</i>)	Marathon County, Wis.	Green	5	5.7		123.8	0.464	.570	65	15.6	4.4	9.0	3,400	6,590	904	.72	8.8	30.7	7,980	2.9	38	2,380	3,040	519	758	718	1,092	318	562	
197	do	Potter County, Pa.	Dry	3			4.6	0.552						7,650	11,340	1,120	2.51	10.7		12,130	6.9	30	5,190	6,990	980	917	905				
534	do	Grafton County, N. H.	Green	5	19.0		70.2	0.421		45				2,850	6,940	1,052	.44	11.8		28.0		34		2,700	292	536	486	825		578	
5	Elm, rock (<i>Ulmus racemosa</i>)	Marathon County, Wis.	Dry	5			10.8	0.469						6,790	12,140	1,504	1.75	13.4	21.0	14,620	7.4	35	4,040	5,840	727	892	679	1,447	321	644	
300	do	Rusk County, Wis.	Green	1	18.5	31	91.6	0.438	.537	52	14.4	4.2	9.5	3,830	7,010	1,020	.85	11.2	27.2	8,120	2.9	34	2,260	2,920	410	625	546	922	310	558	
111	Elm, slippery (<i>Ulmus fulva</i>)	Hendricks County, Ind.	Dry	5	27.1	50	52.7	0.569	.658	54	14.1	4.8	8.1	9,770	15,290	1,480	3.89	14.2	31.6	17,000	10.4	46	5,420	7,050	874	1,307	914	1,802	349	605	
211	do	Sauk County, Wis.	Green	6	7.5	73	89.6	0.480	.568	57	14.8			4,130	7,390	1,202	.83	12.3	32.1	8,830	3.0	42	1,630	2,930	486	743	708	1,098	353	626	
752	Fig, golden (<i>Ficus aurea</i>)	Dade Co., Fla.	Dry	6			10.8	0.524						7,850	11,330	1,315	2.64	12.7	39.2	13,490	6.5	35	4,020	5,680	1,208	1,202	892	1,664	389	756	
226	Gum, black (<i>Nyssa sylvatica</i>)	Sevier County, Tenn.	Green	5	30.0		43.8	0.579		52				4,290	9,430	1,212	.90	19.4	52.5		5.0	48		3,740	693	954	898	1,270			
294	Gum, blue (<i>Eucalyptus globulus</i>)	Alameda County, Calif.	Dry	5			11.2	0.630						8,000	16,350	1,755	2.10	24.4	38.9	18,310	9.0	52	4,900	7,750	1,603	1,593	1,257	2,154	518	1,068	
388	Gum, red (<i>Liquidambar styraciflua</i>)	Pemiscot County, Mo.	Green	1			5.8	0.568						4,890	9,550	1,165	1.20	20.3	47.2	10,950	4.1	59	3,000	3,820	813	1,013	988	1,276	406	662	
175	Gum, tupelo (<i>Nyssa aquatica</i>)	St. John the Baptist Parish, La.	Dry	1	8.4	68	57.5	0.541	.639	53	15.5	5.1	9.9	10,700	16,600	1,472	4.64	17.0	46.3	18,700	9.8	60	5,700	9,280	2,109	1,861	1,686	2,128	332	410	
368	do	Pemiscot County, Mo.	Green	1			11.6	0.591						5,560	9,510	1,314	1.32	11.7	36.1	11,700	4.9	40	3,450	3,990	730	919	722	1,186	412	798	
752	Gumbo limbo (<i>Bursera simaruba</i>)	Dade County, Fla.	Dry	1			11.4	0.504	.576	47	14.0	4.2	8.9	7,940	13,950	1,622	2.20	14.4	33.7	16,590	8.5	40	4,540	7,080	1,145	1,631	1,214	2,090	544	967	
111	Hackberry (<i>Celtis occidentalis</i>)	Hendricks County, Ind.	Green	5	17.2	51	90.0	0.474	.554	56	13.4	4.9	8.7	3,740	7,710	1,215	.72	16.1	38.6	8,640	3										

TABLE 21.—Strength and related properties, by localities, of woods grown in the United States—Continued

Ship- ment no.	Species (common and botanical names)	Place of growth of material tested	Moisture condition	Trees tested	Rings per inch	Sum- mer wood	Mois- ture con- tent	Specific gravity, oven dry, based on volume—		Weight per cubic foot	Shrinkage from green to oven- dry condition based on dimen- sions when green			Static bending						Impact bending			Compression parallel to grain		Com- pression perpen- dicular to grain; stress at propor- tional limit	Hardness; load required to em- bed a 0.444-inch ball to ½ its diameter		Shear parallel to grain; maxi- mum shearing strength	Cleav- age; load to cause splitting	Tension perpen- dicular to grain; maxi- mum tensile strength	
								At test	When oven- dry		Volu- metric	Ra- dial	Tan- gen- tial	Stress at prop- ortional limit	Modu- lus of rupture	Modu- lus of elas- ticity	Work			Stress at propor- tional limit	Work to propor- tional limit	Height of drop causing complete failure (50-pound hammer)	Stress at propor- tional limit	Maxi- mum crushing strength		End	Side				
																	Proport- ional limit	Maxi- mum load	Total												
1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31	
				Num- ber	Num- ber	Per- cent	Per- cent			Pounds	Per- cent	Per- cent	Per- cent	Lb. per sq. in.	Lb. per sq. in.	1,000 lb. per sq. in.	In.-lb. per cu. in.	In.-lb. per cu. in.	In.-lb. per cu. in.	Lb. per sq. in.	In.-lb. per cu. in.	Inches	Lb. per sq. in.	Lb. per sq. in.	Lb. per sq. in.	Pounds	Pounds	Lb. per sq. in.	Lb. per in. of width	Lb. per sq. in.	
HARDWOODS—continued																															
211	Haw, pear (<i>Crataegus tomentosa</i>)	Sauk County, Wis.	Green	2	10.6		63.4	0.623		64				3,880	7,650	964	0.89	22.7	52.0	13,500	5.5	20	4,750	3,110	980	1,218	1,204	1,356			
			Dry	1			8.6	.695						9,020	17,250	1,368	3.30	23.8	30.3	14,350	7.8	130	1,820	3,260	1,000	2,270	1,675	1,212			
43	Hickory, bigleaf shagbark (<i>Hicoria laciniosa</i>)	Fulton County, Ohio	Green	9	13.8	71	58.0	.647		64	20.9	7.9	14.2	4,800	9,880	1,099	1.23	36.2	99.0	14,350								1,212			
			Dry	1			9.1	.734						9,250	21,500	2,080	2.09	23.3				100	11,000	2,970	1,000			2,348			
42	do	Sardis, Miss.	Green	10	23.3	60	63.2	.599		61	17.6	7.4	11.2	6,370	11,110	1,562	1.47	24.3	78.0	13,970	6.3	80	3,570	4,520	994			1,162			
			Dry	1			8.4	.690						10,350	19,300	2,010	3.04	21.6	56.8	25,060	14.5	70		8,550	2,386			2,512			
43	Hickory, bittersnut (<i>Hicoria cordiformis</i>)	Fulton County, Ohio	Green	11	11.4	70	66.0	.604		63				5,470	10,280	1,399	1.22	20.0	75.5	15,860	8.5	66	4,330	4,570	986			1,237			
			Dry	1			9.2	.676						10,260	18,850	1,880	3.19	17.9	67.5	26,540	14.0	66		10,600	2,390						
48	Hickory, mockernut (<i>Hicoria alba</i>)	Chester County, Pa.	Green	11	16.4	69	57.0	.663		65	18.9	8.4	11.4	6,550	11,110	1,508	1.50	31.7	84.4	15,390	6.7	120	3,990	4,320	958			1,282			
			Dry	1			8.8	.711						14,300	21,950	2,555	4.04	24.2	74.1	24,130	12.0	96		10,400	2,719						
42	do	Sardis, Miss.	Green	8	19.4	55	63.0	.613		62	16.5	6.9	10.4	5,900	10,840	1,625	1.22	18.6	58.2	14,670	6.8	44	3,600	4,600	1,065			1,270			
			Dry	1			9.0	.713						12,860	21,200	2,150	4.39	18.9	52.0	18,860	8.9	44		10,900	2,303			1,899			
46	do	Webster County, W. Va.	Green	1	31.0	62	48.7	.656		61				6,890	12,720	1,883	1.41	24.1	98.7				5,230	5,240							
			Dry	1																											
42	Hickory, nutmeg (<i>Hicoria myristicaeformis</i>)	Sardis, Miss.	Green	5	21.6	59	74.0	.556		60				4,860	9,060	1,829	1.06	22.8	58.2	12,780	6.1	54	3,620	3,980	938			1,032			
			Dry	1			8.9	.617						9,150	19,270	1,821	2.40	25.7						7,960	2,315						
46	Hickory, pignut (<i>Hicoria glabra</i>)	Webster County, W. Va.	Green	19	22.0	63	52.0	.607		63	21.2	8.5	13.8	5,860	11,810	1,769	1.12	30.6	86.7	19,520	10.6	74	3,520	4,820	1,114			1,396			
			Dry	1				.780						12,990	20,620	2,405	4.12	29.9						10,200	2,816			2,482			
42	do	Sardis, Miss.	Green	4	17.3	60	59.0	.626		62	15.0	5.6	9.8	6,430	11,780	1,565	1.42	24.7	65.1	16,610	8.7	96	3,450	4,870	1,101			1,208			
			Dry	1			9.4	.732						12,900	23,460	2,520	3.74	23.9	58.6	25,600	13.6	58		10,700	2,375						
43	do	Fulton County, Ohio	Green	10	18.9	63	55.0	.657		64	15.3	6.3	9.5	6,820	12,360	1,553	1.71	27.7	88.7	13,520	7.9	98	4,100	4,760	1,224			1,428			
			Dry	1				.767						11,680	22,500	2,508	3.40	23.8	68.5	22,930	9.9	76		10,110	2,994			2,708			
48	do	Chester County, Pa.	Green	27	18.6	67	54.0	.664		64	16.9	6.8	10.9	6,140	11,450	1,605	1.34	34.9	87.9	16,270	7.8	96	4,270	4,820	1,130			1,358			
			Dry	1				.706						12,900	24,000	2,370	3.89	33.7	90.9	27,480	14.6	72		11,130	2,918			2,316			
42	Hickory, shagbark (<i>Hicoria ovata</i>)	Sardis, Miss.	Green	4	20.4	61	64.0	.606		62	16.0	6.5	10.2	6,220	11,330	1,638	1.34	16.7	64.2	14,380	6.6	52	4,160	5,060	1,158			1,262			
			Dry	1			9.0	.706						12,220	21,700	2,245	3.88	20.7	66.1	17,890	8.1	48		10,120	2,197			2,360			
43	do	Fulton County, Ohio	Green	9	17.1	70	58.0	.642		63	18.4	7.9	11.4	5,430	10,990	1,346	1.27	34.1	86.4	12,460	5.5	100	2,840	4,360	1,080			1,421			
			Dry	1			8.4	.647						10,000	22,600	2,120	2.53	29.9						10,500	2,717						
46	do	Webster County, W. Va.	Green	10	20.0	64	60.0	.647		65	15.5	6.5	9.7	6,120	11,000	1,752	1.22	18.3	72.3	16,060	7.2	60	3,730	4,600	972			1,264			
			Dry	1				.754						13,360	22,900	2,495	4.02	26.3						11,200	2,370			2,390			
48	do	Chester County, Pa.	Green	1	24.2	63	64.1	.609		62				5,900	10,170	1,392	1.61	11.9	75.3				2,780	4,370							
			Dry	1			9.1	.710						14,000	22,000	1,885	6.65	16.8						9,850	2,250						
42	Hickory, water (<i>Hicoria aquatica</i>)	Sardis, Miss.	Green	2	15.3	67	80.0	.606		68				5,980	10,740	1,563	1.29	18.8	52.9	13,730	6.1	56	3,240	4,660	1,088			1,440			
			Dry	1			8.8	.604						11,750	20,200	2,165	3.55	19.4	39.9					6,190	10,140	2,208					
226	Holly (<i>Ilex opaca</i>)	Sevier County, Tenn.	Green	5	26.9		81.7	.503	0.606	57	16.2	4.5	9.5	3,370	6,540	897	.72	10.8	26.7	8,900	4.4	51	2,050	2,640	610	858	792	1,130	362	606	
			Dry	2			4.4	.604						8,020	12,700																

TABLE 21.—Strength and related properties, by localities, of woods grown in the United States—Continued

Ship- ment no.	Species (common and botanical names)	Place of growth of material tested	Moisture condition	Trees tested	Rings per inch	Sum- mer wood	Mois- ture con- tent	Specific gravity, oven dry, based on volume—		Weight per cubic foot	Shrinkage from green to oven- dry condition based on dimen- sions when green			Static bending						Impact bending			Compression parallel to grain		Com- pression perpen- dicular to grain; stress at proportional limit	Hardness; load required to em- bed a 0.444-inch ball to ½ its diameter		Shear parallel to grain; maxi- mum shearing strength	Cleav- age; load to cause splitting	Tension perpen- dicular to grain; maxi- mum tensile strength
								At test	When oven- dry		Volu- metric	Ra- dial	Tan- gen- tial	Stress at pro- portional limit	Modu- lus of rupture	Modu- lus of elas- ticity	Work			Stress at proportional limit	Work to proportional limit	Height of drop causing complete failure (50-pound hammer)	Stress at proportional limit	Maxi- mum crushing strength		End	Side			
																	Proportional limit	Maxi- mum load	Total											
1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31
	HARDWOODS—continued			Num- ber	Num- ber per inch	Per- cent	Per- cent			Pounds	Per- cent	Per- cent	Per- cent	Lb. per sq. in.	Lb. per sq. in.	1,000 lb. per sq. in.	In.-lb. per cu. in.	In.-lb. per cu. in.	In.-lb. per cu. in.	Lb. per sq. in.	In.-lb. per cu. in.	Inches	Lb. per sq. in.	Lb. per sq. in.	Lb. per sq. in.	Pounds	Pounds	Lb. per sq. in.	Lb. per in. of width	Lb. per sq. in.
197	Maple, red (<i>Acer rubrum</i>)	Potter County, Pa.	(Green— Dry)	5 1	11.5		70.0 8.5	0.464 .529	0.539	49	12.5	3.8	8.1	3,770 9,720	7,470 14,820	1,395 1,716	0.60 3.15	11.3 13.1	23.6 28.0	9,900 17,020	3.7 7.9	31 31	2,370 5,710	3,090 7,910	1,568 1,268	715 1,530	603 974	1,084 2,116	334 487	765 545
865	do.	Strafford County, N. H.	(Green— Dry)	5 3	8.8		51.4 14.0	.494 .544	.552	47	13.7	4.2	8.3	3,430 8,160	7,410 12,820	1,330 1,516	.76 3.06	12.9 12.5	29.4 33.8	9,030	2.9	35 35	2,100 3,780	3,160 5,840	461 1,209	850 1,437	746 928	1,154 1,828	256 486	635 1,047
211	Maple, silver (<i>Acer saccharinum</i>)	Sauk County, Wis.	(Green— Dry)	5 1	7.1		65.7 8.2	.439 .479	.506	45	12.0	3.0	7.2	3,120 7,680	5,820 10,100	943 1,206	.61 2.67	11.0 7.6	22.3 11.2	6,830 15,000	2.6 9.4	29 24	1,930 5,640	2,490 6,600	456 1,181	671 1,376	592 746	1,053 1,714	302 353	564 489
904	Maple, striped (<i>Acer pennsylvanicum</i>)	(Hampshire County, Mass. Bennington County, Vt.)	Green— Dry	4 3	11.6		35.3 13.2	.438 .462		37	12.3	3.2	8.6	3,620 5,110	7,230 10,780	1,080 1,352	.68 1.06	10.9 11.3	13.4 16.7	8,740 11,310	2.3 5.0	36 27	1,790	2,920	496	970		1,146		
111	Maple, sugar (<i>Acer saccharum</i>)	(Hendricks and Morgan Coun- ties, Ind.)	(Green— Dry)	4	19.4		55.4	.553	.649	54	14.4	4.9	9.1	4,780	9,090	1,496	.92	12.7	30.3	12,060	4.8	35	3,020	3,780	631	1,006	928	1,373	447	742
197	do.	Potter County, Pa.	(Green— Dry)	6 2	22.2		67.1 9.1	.554 .642	.671	58	14.7	4.8	9.2	5,710 11,760	9,490 16,700	1,524 1,736	1.25 4.54	13.6 14.6	35.6 22.8	12,370 23,270	5.3 11.9	42 40	3,200 7,620	3,870 9,790	704 1,893	1,035 2,110	920 1,584	1,380 2,784	436 648	796 813
904	Maple, sugar (second growth) (<i>Acer saccharum</i>)	Bennington County, Vt.	(Green— Dry)	7 4	9.5		49.7 13.7	.581 .644	.695	54	15.3	4.9	10.0	5,240 8,650	9,960 16,040	1,672 1,856	1.02 2.17	15.9 19.9	45.7 37.5	13,550	4.7	49 51	3,430 4,420	4,270	917	1,200	1,095	1,612	436	785
5	Maple, sugar (<i>Acer saccharum</i>)	Marathon County, Wis.	(Green— Dry)	4	22.0		62.5 12.5	.560 .621		57				4,620 9,110	8,820 14,830	1,437 1,930	.85 2.41	9.6 13.8	17.1 21.3	12,250	5.6	28 29	4,020 5,360	870	965	1,342	1,434	603		
752	Mastic (<i>Sideroxylon foetidissimum</i>)	Dade County, Fla.	(Green— Dry)	5 2			38.6 10.8	.886 .936	1.034	77	11.7	6.1	7.5	7,070 6,550	10,390 13,700	1,576 1,794	1.79 1.36	8.1 6.0	19.8 6.0	18,000 13,900	8.7 4.9	52 22	4,950 3,550	5,880	2,679	1,667	1,766	1,667	428	1,034
5	Oak, black (<i>Quercus velutina</i>)	Marathon County, Wis.	(Green— Dry)	5	19.0		84.5 11.4	.538 .593		62				3,720 8,220	7,650 14,670	1,121 1,641	.71 2.31	13.2 14.2	23.4 34.3	12,250	5.6	35 30	3,080 4,650	802	847	1,060	1,292			
101	do.	Stone County, Ark.	(Green— Dry)	5 1	12.5	71	76.7 11.8	.569 .625	.669	63	14.2	4.5	9.7	5,060 7,850	8,570 13,700	1,219 1,662	1.20 2.11	11.7 13.4	28.1 23.6	10,840	4.4	43 47	2,900 4,640	3,700	912	1,093	1,057	1,179	424	828
211	Oak, bur (<i>Quercus macrocarpa</i>)	Sauk County, Wis.	(Green— Dry)	5 2	12.1	59	69.6 10.2	.583 .653	.671	62	12.7	4.4	8.8	3,640 7,000	7,180 10,900	877 1,060	.89 2.79	10.7 9.6	26.1 16.2	10,020	4.7	47 24	2,380 3,810	3,290	836	1,158	1,108	1,354	428	804
294	Oak, California black (<i>Quercus kelloggii</i>)	Butte County, Calif.	(Green— Dry)	5 1	20.1	48	106.9 5.3	.491 .605	.547	63	13.6	4.1	6.4	3,210 10,890	5,740 12,950	786 1,264	.81 5.31	7.5 7.3	12.1 13.1	8,320	3.1	28 12	1,800 5,020	2,530	696	807	728	987	334	609
319	do.	Douglas County, Oreg.	(Green— Dry)	5 2	11.9	55	104.7 5.2	.529 .608	.608	68	10.6	3.1	6.8	3,640 5,900	6,630 8,160	684	1.25 1.033	10.2 2.14	20.0	8,050	3.6	31 12	1,960 4,040	3,070	1,093	1,020	980	1,298	360	784
294	Oak, canyon live (<i>Quercus chrysolepis</i>)	Butte County, Calif.	(Green— Dry)	3 1	12.9		61.8 5.0	.702 .822	.838	71	16.2	5.4	9.5	6,330 11,900	10,550 14,660	1,340 1,810	1.70 4.67	14.4 7.8	30.9 17.0	11,150	3.9	47 32	3,940 7,880	4,690	1,475	1,592	1,570	1,696	525	974
226	Oak, chestnut (<i>Quercus montana</i>)	Sevier County, Tenn.	(Green— Dry)	5 1	23.4	50	71.8 9.5	.573 .676	.674	61	16.7	5.5	9.7	4,630 10,600	8,030 15,040	1,372 1,645	.90 3.85	11.5 9.4	22.4 18.8	12,010	4.6	35 42	2,890 4,830	3,520	657	971	894	1,212	383	686
258	Oak, laurel (<i>Quercus laurifolia</i>)	Winn Parish, La.	(Green— Dry)	5 1	11.0	61	84.3 9.5	.564 .645	.703	65	19.0	4.0	9.9	4,520 8,680	7,940 14,050	1,393 1,770	.86 2.47	11.2 12.0	28.3 28.6	10,350	3.4	39 39	2,650 5,220	3,170	707	1,019	996	1,182	384	770
751	Oak, live (<i>Quercus virginiana</i>)	Marion County, Fla.	(Green— Dry)	5 2	8.2		49.7 13.4	.810 .879	.977	76	14.7	6.6	9.5	8,440 8,730	11,930 18,080	1,575 1,956	2.54 2.20	12.6 15.3	26.0 38.4	17,200	8.5	54 37	4,170 5,000	5,430	2,517	1,674	1,882	2,210	550	1,041
319	Oak, Oregon white (<i>Quercus garryana</i>)	Douglas County, Oreg.	(Green— Dry)	10 4	16.3	49	71.6 6.6	.644 .760	.748	69	13.4	4.2	9.0	4,630 7,780	7,720 11,740	792	1.51 2.73	13.7 8.5	29.8 14.6	10,260	4.8	49 23	2,480 4,880	3,570	1,375	1,432	1,392	1,634	449	943
904	Oak, pin (<i>Pinus palustris</i>)	Hampshire County, Mass.	(Green— Dry)	5 5	9.0	58	75.2 11.4	.577 .629	.677	63	14.5	4.3	9.5	4,000 8,360	8,330 14,490	1,318 1,754	.71 2.37	14.0 14.9	35.2 30.2	11,920	4.2	48 45	4,770 2,750	3,680	883	996	1,074	1,293	470	804
101	Oak, post (<i>Quercus stellata</i>)	Stone County, Ark.	(Green— Dry)	5 1	30.4	61	64.5 11.2	.590 .677	.732	61	16.0	5.7	10.6	4,720 7,860	7,380 12,450	913	1.39 2.68	9.1 10.0	18.0 19.2	11,260	4.4	38 44	2,750 3,240	3,330	1,148	1,139	1,074	1,299	420	819
258	do.	Winn Parish, La.	(Green— Dry)	5 1	21.2	47	73.6 11.2	.602 .683	.745	65	16.5	5.2	8.9	5,230 7,810	8,780 14,860	1,259 1,770	1.23 1.99	13.0 16.0	32.8 39.9	10,570	3.7	49 48	4,930 4,2,							

TABLE 21.—Strength and related properties, by localities, of woods grown in the United States—Continued

Ship- ment no.	Species (common and botanical names)	Place of growth of material tested	Moisture condition	Trees tested	Rings per inch	Sum- mer wood	Mois- ture con- tent	Specific gravity, oven dry, based on volume—		Weight per cubic foot	Shrinkage from green to oven- dry condition based on dimen- sions when green			Static bending						Impact bending			Compression parallel to grain		Com- pression perpen- dicular to grain; stress at proportional limit	Hardness; load required to em- bed a 0.444-inch ball to ½ its diameter		Shear parallel to grain; maxi- mum shearing strength	Cleav- age; load to cause splitting	Tension perpen- dicular to grain; maxi- mum tensile strength
								At test	When oven- dry		Volumetric	Radial	Tangential	Stress at proportional limit	Modulus of rupture	Modulus of elasticity	Work			Stress at proportional limit	Work to proportional limit	Height of drop causing complete failure (50-pound hammer)	Stress at proportional limit	Maximum crushing strength		End	Side			
																	Proportional limit	Maximum load	Total											
1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31
	HARDWOODS—continued			Number	Number	Percent	Percent			Pounds	Percent	Percent	Percent	Lb. per sq. in.	Lb. per sq. in.	1,000 lb. per sq. in.	In.-lb. per cu. in.	In.-lb. per cu. in.	In.-lb. per cu. in.	Lb. per sq. in.	Inches	Lb. per sq. in.	Lb. per sq. in.	Lb. per sq. in.	Pounds	Pounds	Lb. per sq. in.	Lb. per in. of width	Lb. per sq. in.	
75	Oak, white (<i>Quercus alba</i>)	Richland Parish, La.	(Green	5	16.0	67	78.5	0.596	0.708	66	16.0	4.8	9.2	4,410	7,760	1,194	0.94	8.9	20.2	11,750	5.2	35	3,000	3,490	1,004	1,183	1,155	1,253	422	805
101	do	Stone County, Ark.	Dry	1	22.1	65	58.0	.591	.704	58	15.8	6.2	8.3	9,080	15,580	1,795	2.59	15.6	30.9	17,750	8.0	44	5,160	7,580	1,085	1,590	1,528	2,048	488	672
111	do	(Hendricks, Marion, and Mor- gan Counties, Ind.	Green	5	15.6	60	62.5	.603	.696	61	14.3	4.9	9.0	4,320	8,090	1,137	.95	12.1	31.4	9,860	3.8	40	2,980	3,520	829	1,113	1,080	1,194	420	678
258	do	Winn Parish, La.	Dry	1	14.7	49	71.8	.591	.732	63	16.9	5.4	9.5	7,700	16,100	1,948	2.67	14.3	32.3	14,600	6.4	28	4,020	7,580	1,456	1,622	1,275	2,090	484	818
258	Oak, willow (<i>Quercus phellos</i>)	do	Green	5	13.7	56	94.3	.556	.688	67	18.9	5.0	9.6	5,430	8,690	1,344	1.26	11.9	29.6	10,390	3.3	25	3,130	3,530	727	1,087	1,048	1,306	424	814
111	Osage-orange (<i>Toxylon pomiferum</i>)	Morgan County, Ind.	Dry	1	6.5	82	31.2	.761	.838	62	8.9			9,000	15,410	1,790	2.56	18.2	30.2	18,760	8.9	53	4,710	7,180	1,310	1,412	1,190	1,950	398	856
751	Palmetto, cabbage (<i>Sabal palmetto</i>)	Marion County, Fla.	Green	5			133.5	.372	.453	54	25.0			4,370	7,400	1,286	.88	8.8	21.3	9,220	2.9	35	2,340	3,000	754	1,022	978	1,184	404	760
752	Paradise-tree (<i>Simarouba glauca</i>)	Dade County, Fla.	Dry	2			10.6	.389						10,670	16,400	2,043	3.18	16.0	41.3	17,480	9.3	44	4,920	8,230	1,640	1,534	1,620	1,804		
368	Pecan (<i>Hicoria pecan</i>)	Pemiscot County, Mo.	Green	5	12.1	63	62.9	.601	.694	61	13.6	4.9	8.9	7,760	13,660	1,329	2.53	37.9	101.7	15,520	8.9	120	3,980	5,810	2,260	1,838	2,037			
368	Persimmon (<i>Diospyros virginiana</i>)	do	Dry	2			6.2	.697						1,910	3,750	485	.45	4.0	15.8	5,000	2.7	15	1,410	1,750	186	333	281	571	114	224
752	Pigeon-plum (<i>Coccolobis laurifolia</i>)	Dade County, Fla.	Green	5	13.8		58.5	.639	.776	63	18.3	7.5	10.8	3,240	4,930	571	1.07	4.5	20.2	6,380	2.9	16	1,450	2,280	173	298	375	377	72	114
752	Poisonwood (<i>Metopium toxiferum</i>)	do	Dry	2			5.5	.812						1,910	3,490	696	.40	1.8	2.4	5,400	1.9	7	1,260	1,810	265	349	245	711	149	308
904	Poplar, balsam (second growth) (<i>Populus balsamifera</i>)	Bennington County, Vt.	Green	5	4.6		121.2	.301	.331	42	8.0	2.0	5.4	4,320	5,660	877	.99	3.4	4.4	5,510	1.5	7	2,550	3,510	456	684	388	593		338
939	Poplar, balsam (<i>Populus balsamifera</i>)	Near Girdwood, Alaska.	Dry	4			12.3	.316						5,230	9,770	1,367	1.18	14.6	43.4	12,320	5.0	53	3,100	3,990	959	1,274	1,308	1,482	420	677
226	Poplar, yellow (<i>Liriodendron tulipifera</i>)	Sevier County, Tenn.	Green	5	14.0		64.0	.371	.419	38	11.4	4.1	6.9	11,980	16,210	1,941	4.33	13.4	32.2	20,450	10.4	41	6,630	10,890	3,348	2,370	2,142	2,536	856	
634	do	Breathitt County, Ky.	Dry	1			6.1	.411						5,600	10,030	1,367	1.35	13.0	31.2	12,120	4.5	41	3,160	4,170	1,110	1,243	1,279	1,474	410	770
226	Rhododendron, great (<i>Rhododendron maximum</i>)	Sevier County, Tenn.	Green	5	27.9		98.8	.501	.601	62	16.2	6.3	8.7	15,450	23,700	2,480	5.70	16.9	37.4	22,420	11.7	35	9,550	14,050	3,908	3,730	3,178	2,666	718	1,525
226	Sassafras (<i>Sassafras variifolium</i>)	do	Dry	6			63.9	.381	.434	39	13.0	4.0	7.2	5,030	9,830	1,299	1.17	11.6	24.7	15,970	6.8	40	4,260	4,940	1,500	1,734	1,722	2,010	399	863
226	Service berry (<i>Amelanchier canadensis</i>)	do	Green	4			6.8	.462						7,800	13,270	1,292	2.82	10.8												
226	Silverbell (<i>Halesia carolina</i>)	do	Dry	2			6.8	.462						3,160	5,120	407	1.36	5.6	7.2	8,070	3.3	15	1,220	1,600	210	298	298	308	183	353
226	Sourwood (<i>Oxydendrum arboreum</i>)	do	Green	5	19.4	48	67.4	.424	.473	44	10.3	4.0	6.2	5,470	10,850	1,322	1.25	6.4	6.5											
752	Stopper, red (<i>Eugenia confusa</i>)	Dade County, Fla.	Dry	3			61.6	.473	.545	48	12.7	5.0	7.3	2,120	4,040	799	.32	4.2	8.1	6,180	2.3	18	1,310	1,720	157	261	250	523	140	183
368	Sugarberry (<i>Celtis laevigata</i>)	Pemiscot County, Mo.	Green	5	19.4		47.9	.656	.791	61	18.7	6.7	10.8	4,020	6,990	1,033	.98	5.8	7.1	7,750	2.8	14	2,970	3,750	388	494	316	553	204	325
211	Sumach, staghorn (<i>Rhus hirta</i>)	Sauk County, Wis.	Dry	3	8.6	61	44.7	.449		41				2,090	3,680	696	.37	4.2	6.4	5,740	2.2	13	1,130	1,660	178	214	203	486	120	148
111	Sycamore (<i>Platanus occidentalis</i>)	Hendricks County, Ind.	Green	5	19.2		81.1	.454	.526	51	13.5	4.9	7.3	4,930	7,290	1,252	1.16	4.4	6.6	8,610	2.6	14	3,100	4,060	384	383	307	500	204	426
226	do	Sevier County, Tenn.	Dry	1			84.9	.458	.552	53	14.8	5.2	7.9	3,150	5,570	1,207	.48	5.6	9.2	8,050	2.6	17	2,030	2,550	310	418	338	788	250	464
386	Walnut, black (<i>Juglans nigra</i>)	Kentucky	Green	5	12.2		81.1	.513	.582	58	11.3	5.2	7.1	8,360	11,850	1,410	2.52	7.5	10.6	18,620	8.9	22	4,630	7,480	740	590	448	1,174	309	572
463	Walnut, little (<i>Juglans rupestris</i>)	Coconino County, Ariz.	Dry	1			66.8	.532	.613	55	10.7	4.4	4.6	3,150	5,570	1,207	.48	5.6	9.2	8,050	2.6	17	2,030	2,550	310	418	338	788	250	464
211	Willow, black (<i>Salix nigra</i>)	Sauk County, Wis.	Green	5	4.2		148.4	.330	.409	51	13.3	2.2	8.2	8,360	11,850	1,410	2.52	7.5	10.6	18,620	8.9	22	4,630	7,480	740	590	448	1,174	309	572
368	do	Pemiscot County, Mo.	Dry	2			8.2	.582						3,150	5,570	1,207	.48	5.6	9.2	8,050	2.6	17	2,030	2,550	310	418	338	788		

TABLE 21.—Strength and related properties, by localities, of woods grown in the United States—Continued

Ship- ment no.	Species (common and botanical names)	Place of growth of material tested	Moisture condition	Trees tested	Rings per inch	Sum- mer wood	Mois- ture con- tent	Specific gravity, oven dry, based on volume—		Weight per cubic foot	Shrinkage from green to oven- dry condition based on dimen- sions when green			Static bending						Impact bending			Compression parallel to grain		Com- pression perpen- dicular to grain; stress at proportional limit	Hardness; load required to em- bed a 0.444-inch ball to ½ its diameter		Shear parallel to grain; maxi- mum shearing strength	Cleav- age; load to cause splitting	Tension perpen- dicular to grain; maxi- mum tensile strength	
								At test	When oven- dry		Volu- metric	Ra- dial	Tan- gen- tial	Stress at pro- portional limit	Modu- lus of rupture	Modu- lus of elas- ticity	Work			Stress at proportional limit	Work to proportional limit	Height of drop causing complete failure (50-pound hammer)	Stress at proportional limit	Maxi- mum crushing strength		End	Side				
																	Proportional limit	Maxi- mum load	Total												
1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31	
	SOFTWOODS—continued			Num- ber	Num- ber	Per- cent	Per- cent			Pounds	Per- cent	Per- cent	Per- cent	Lb. per sq. in.	Lb. per sq. in.	1,000 lb. per sq. in.	In.-lb. per cu. in.	In.-lb. per cu. in.	In.-lb. per cu. in.	Lb. per sq. in.	In.-lb. per cu. in.	Inches	Lb. per sq. in.	Lb. per sq. in.	Lb. per sq. in.	Lb. per sq. in.	Pounds	Pounds	Lb. per sq. in.	Lb. per in. of width	Lb. per sq. in.
318	Cedar, incense (<i>Libocedrus decurrens</i>)	Lane County, Oreg.	(Green..... Dry..... Green.....	4 2 2	17.4 16.0	30 30 30	135.8 5.1 80.0	0.332 0.365 0.360	0.365	49	7.7	3.3	5.2	3,920 7,370 3,950	6,400 9,410 6,040	926 1,296 754	0.94 2.31	6.4 4.9	8.8 7.9	7,320 11,120	2.4 5.2	17	2,940 6,290	3,270 7,200	393 841	570 1,037	389 521	834 905	160	284 268	
319	do	Weed, Calif.	(Green..... Dry..... Green.....	5 1 9	23.6 22.4	25 39	52.0 9.0 38.2	.411 .454 .392	.470	39	10.7	5.2	8.1	3,920 8,880 3,980	6,800 14,510 5,840	1,497 2,039 1,375	.59 2.25 1.75	7.8 12.1 7.2	24.1 23.6	9,330 17,600 2,650	2.7 7.2 3.2	25	2,980 7,960 2,650	3,280 7,750 3,040	385 1,025 332	555 948 414	475 696 354	881 1,496 808	154 358 78	240 631 142	
532	do	Coos County, Oreg.	(Green..... Dry..... Green.....	9 9 5	12.0	35.1	10.1 44.2 15.3	.400 .442 .464	.492	37	7.8	3.1	4.7	3,980 7,970 3,430	5,840 10,990 7,030	1,375 1,634 649	.68 2.20 1.08	7.2 8.0 15.0	7.2 8.0 34.7	9,080 12,300 6,990	3.2 4.4 2.7	19	2,650 7,240 2,540	3,040 7,240 3,570	332 931 864	414 707 760	354 528 646	808 924 1,008	78 215 176	142 390 331	
904	Cedar, eastern red (<i>Juniperus virginiana</i>)	Bennington County, Vt.	(Green..... Dry..... Green.....	5 3 5	12.0	35.1	15.3 44.2 15.3	.442 .464 .421	.492	37	7.8	3.1	4.7	3,430 3,670 5,050	7,030 8,280 8,350	649 812 929	1.08 1.03 1.57	15.0 9.7 8.8	34.7 8.040 10.7	6,990 8,040 10,540	2.7 4.0 5.4	35	2,540 5,210	3,570 5,210	864 1,046	760 860	646 1,008	1,008	176 234	331 376	
751	Cedar, southern red (<i>Juniperus</i> sp.)	Marion County, Fla.	(Green..... Dry..... Green.....	5 2 5	13.4	25.6	12.9 44.0 25.6	.421 .440 .294	.453	33	7.0	2.2	4.0	5,050 7,290 2,890	8,350 9,450 4,750	929 1,169 886	1.57 1.88 .54	8.8 5.4 4.5	10.7 6.6 7.1	10,540 10,270 6,360	5.4 4.3 2.0	18	3,910 5,080 2,380	4,360 6,370 2,630	911 998 278	809 1,017 394	580 606 246	1,188 758	208 758	398	
224	Cedar, western red (<i>Thuja plicata</i>)	Missoula County, Mont.	(Green..... Dry..... Green.....	5 1 5	20.9	42	32.8 7.3 31.9	.294 .319 .326	.327	24	7.6	2.5	4.6	2,890 5,440 3,620	4,750 8,080 5,730	886 1,220 1,021	.54 1.37 1.74	4.5 4.9 5.6	7.1 7.6 10.3	6,360 9,980 7,820	2.0 3.8 2.9	16	2,380 5,020 2,700	2,630 6,090 3,050	278 554 351	394 620 462	246 327 272	698 848 742	132 99	216 146	
263	Cedar, western (<i>Thuja plicata</i>)	Snohomish County, Wash.	(Green..... Dry..... Green.....	5 1 5	19.5	31	45.2 7.5 31.1	.326 .356 .311	.360	30	8.6	2.5	5.6	3,620 6,670 3,040	5,730 9,520 4,880	1,021 1,281 850	.74 1.98 .62	5.6 7.9 4.9	10.3 11.1 12.9	7,820 9,150 6,550	2.9	18	2,700 5,720 2,330	3,050 6,540 2,560	351 850 376	462 875 430	272 428 289	742 985	131 148	195	
939	do	Near Ketchikan, Alaska.	(Green..... Dry..... Green.....	5 2 5	17.5	34.0	9.8 31.1 34.0	.330 .311 .340	.340	26	7.0	2.2	4.6	6,200 3,040 6,200	8,500 4,880 8,500	1,061 .62 1,061	2.07 5.7	5.7 13.1	8,320 5,290	3.6	17	4,890 2,330 4,890	5,580 2,560 5,580	794 376 794	796 430 796	414 289 414	908 158 908	152 279	303		
185	Cedar, northern white (<i>Thuja occidentalis</i>)	Shawano County, Wis.	(Green..... Dry..... Green.....	5 2 5	23.4	36	55.0 9.8 29.3	.293 .330 .293	.315	28	7.0	2.1	4.7	2,600 5,100 2,600	4,250 6,720 4,250	643 811 643	.60 1.84 .60	5.7 4.7 5.7	8.9 5.9 8.9	5,290 7,200 5,290	2.8	15	1,490 2,730 1,490	1,990 4,140 1,990	288 389 288	321 466 321	226 338 226	616 902 616	144 150	238	
865	Cedar, southern white (<i>Chamaecyparis thyoides</i>)	Rockingham County, N. H.	(Green..... Dry..... Green.....	5 5 5	20.0	37.1	11.6 37.1 11.6	.320 .323 .299	.360	27	7.5	3.2	5.0	2,090 3,920 2,090	4,490 5,830 4,490	641 852 641	.45 1.14 .45	6.3 3.8 6.3	15.6 4.7 15.6	5,220 7,050 5,220	2.8	14	2,160 1,160 2,160	2,220 4,370 2,220	275 607 275	413 567 413	306 372 306	699 758 699	114 136	150	
891	do	Pasquotank County, N. C.	(Green..... Dry..... Green.....	5 5 5	11.8	33.4	13.6 29.9 13.6	.289 .323 .299	.345	25	9.4	2.5	5.4	2,940 3,920 2,940	4,980 5,830 4,980	863 852 863	.57 1.14 .57	5.4 3.8 5.4	11.4 7.050 11.4	2.5	16	2,160 1,160 2,160	2,560 4,370 2,560	326 607 326	394 567 394	278 372 278	690 758 690	131 136	208		
175	Cypress, southern (<i>Taxodium distichum</i>)	(St. John the Baptist Parish, La.	(Green..... Dry..... Green.....	4 1 6	24.8	38	79.4 12.1 92.4	.452 .478 .386	.439	51	11.5	3.8	6.0	4,430 8,400 3,770	7,110 11,340 6,600	1,378 1,725 1,061	.96 2.73 .79	5.1 7.8 10.7	14.3 18.0 7.8	8,290 11,070 7,800	2.7	23	3,440 5,160 3,440	3,960 7,560 3,960	548 715 548	460 755 460	354 534 354	818 994 818	160 160	246	
368	do	Pemiscot County, Mo.	(Green..... Dry..... Green.....	6 2 6	9.6	26	92.4 6.9 42.5	.386 .425 .443	.439	46	10.1	3.8	6.0	3,770 8,860 3,770	6,600 11,220 6,600	1,061 1,411 1,061	.79 3.22 .79	7.3 7.0	19.7 12.6	7,800 11,230	2.6	25	2,750 6,000	3,170 7,770	424 1,034	474 830	403 564	822 1,134	188 178	310	
553	do	St. Bernard Parish, La.	(Green..... Dry..... Green.....	4 1 6	23.9	52	89.6 6.9 42.5	.443 .443 .443	.510	52	10.7	3.9	6.2	4,230 8,860 4,230	6,420 11,220 6,420	1,181 1,411 .93	.93 3.22 .93	12.8 12.6 12.8	9,340 11,230 9,340	3.8	26	3,710 3,710 3,710	4,448 4,448 3,710	509 564	448 529	448 529	782 1,044	164 166	276		
734	do	Ascension Parish, La.	(Green..... Dry..... Green.....	8 2 5	22.9	26	98.3 12.8 44.1	.415 .494 .441	.457	51	10.0	3.7	6.4	4,470 6,080 4,470	6,750 9,616 6,750	1,182 1,299 1,182	.96 1.64 .96	9.0 7.9 9.0	9.8 8.6 9.8	9,240 9,310	3.5	25	3,510 5,770	513 941	399 563	370 448	848 868	194 164	380		
315	Douglas fir (coast type) (<i>Pseudotsuga taxifolia</i>)	Lewis County, Wash.	(Green..... Dry..... Green.....	8 2 5	12.3	32	36.6 6.6 54.0	.474 .540 .461	.544	40	12.3	5.0	8.3	5,320 12,020 4,860	8,040 15,840 7,860	1,627 2,392 1,679	.98 3.45 .80	6.8 9.4 7.0	21.3 30.2 20.2	2.8	26	3,780 10,620 3,440	4,130 12,100 4,080	558 1,190	558 1,190	558 1,190	558 1,190	558 1,190	558 1,190	558 1,190	
318	do	Lane County, Oreg.	(Green..... Dry..... Green.....	5 2 5	19.8	36	35.2 6.0 52.6	.461 .526 .414	.536	39	13.2	5.7	7.6	4,860 11,370 4,860	7,860 15,250 7,860	1,679 2,180 1,679	.80 3.38 .80	7.0 9.2 6.1	20.4 30.0 15.7	9,850 16,720 8,890	3.1	27	3,440 9,290 2,780	4,080 10,940 3,410	541 1,356	538 904	514 776	882 1,181	102		

TABLE 21.—Strength and related properties, by localities, of woods grown in the United States—Continued

Ship- ment no.	Species (common and botanical names)	Place of growth of material tested	Moisture condition	Trees tested	Rings per inch	Sum- mer wood	Moisture con- tent	Specific gravity, oven dry, based on volume—		Weight per cubic foot	Shrinkage from green to oven- dry condition based on dimen- sions when green			Static bending						Impact bending			Compression parallel to grain		Com- pression perpen- dicular to grain; stress at proportional limit	Hardness; load required to em- bed a 0.444-inch ball to ½ its diameter		Shear parallel to grain; maxi- mum shearing strength	Cleav- age; load to cause splitting	Tension perpen- dicular to grain; maxi- mum tensile strenght
								At test	When oven- dry		Volu- metric	Rad- ial	Tan- gen- tial	Stress at pro- portional limit	Modu- lus of rupture	Modu- lus of elastic- ity	Work			Stress at pro- portional limit	Work to proportional limit	Height of drop causing complete failure (50-pound hammer)	Stress at pro- portional limit	Maxi- mum crushing strength		End	Side			
																	Proportional limit	Maxi- mum load	Total											
1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31
	SOFTWOODS—continued			Num- ber	Num- ber	Per- cent	Per- cent			Pounds	Per- cent	Per- cent	Per- cent	Lb. per sq. in.	Lb. per sq. in.	1,000 lb. per sq. in.	In.-lb. per cu. in.	In.-lb. per cu. in.	In.-lb. per cu. in.	Lb. per sq. in.	In.-lb. per cu. in.	Inches	Lb. per sq. in.	Lb. per sq. in.	Lb. per sq. in.	Pounds	Pounds	Lb. per sq. in.	Lb. per in. of width	Lb. per sq. in.
551	Fir, California red (<i>Abies magnifica</i>)	Plumas County, Calif.	{Green—	5	10.8	39	108.1	0.372	0.421	48	11.8	3.8	6.9	4,140	5,980	1,065	0.95	6.7	12.6	8,650	2.8	22	4,830	2,830	441	387	380	923	191	340
			{Dry—	4			10.0	.391						6,410	9,850	1,461	1.64	8.8	13.9	12,090	4.4	23	4,830	5,870	884	991	514	1,080	190	356
263	Fir, silver (<i>Abies amabilis</i>)	Snohomish County, Wash.	{Green—	6	12.5	26	65.8	.351	.415	36	14.1	4.5	10.0	3,540	5,660	1,257	.60	6.0	12.6	7,830	2.2	21	2,380	2,670	289	362	310	674	146	238
			{Dry—	1			9.3	.392						7,030	10,570	1,605	1.73	10.3	24.6	11,930	5.3	24	5,430	6,540	578	716	471	1,180	210	
142	Fir, white (<i>Abies concolor</i>)	Madera County, Calif.	{Green—	5	9.9	30	156.5	.350	.388	56	10.2	3.4	7.0	3,880	5,970	1,131	.77	5.2	15.7	7,230	2.2	18	2,610	2,800	445	381	328	732	166	258
			{Dry—	1			9.6	.375						7,030	9,800	1,490	1.86	5.5	13.0	8,400	3.2	14	4,070	6,150	720	775	464	1,054	176	258
465	do.	San Miguel County, N. Mex.	{Green—	5	10.6	20	122.8	.312	.360	43	9.0	3.1	6.9	2,940	4,920	.918	.54	5.1	7.6	7,600	2.9	18	1,810	2,210	290	369	278	734	150	252
			{Dry—	2			9.0	.346						6,240	9,440	1,211	1.83	6.0	7.4	10,410	4.3	15		5,660	778	768	463	1,001	226	
571	do.	Plumas County, Calif.	{Green—	10	12.0	37	90.7	.365	.420	43	9.3			4,290	5,950	1,043	1.02	5.1		9,510	3.3	25		2,920	380	381	366	766	181	326
			{Dry—	10			11.4	.388						6,710	9,430	1,425	1.79	7.4		12,220	4.5	23		5,850	559	780	460	913	146	274
165	Hemlock, eastern (<i>Tsuga canadensis</i>)	Marathon County, Wis.	{Green—	5	24.4	31	129.0	.340	.394	49	9.2	2.3	5.0	3,410	5,770	.917	.73	6.6	12.7	6,330	2.2	17	2,140	2,750	420	463	344	802	160	297
			{Dry—	1			9.5	.368						5,380	7,510	1,048	1.65	5.0	8.9	8,220	3.6	13	3,570	5,740	726	810	392	1,148	119	
226	do.	Sevier County, Tenn.	{Green—	5	16.6	36	80.8	.426	.501	48	11.6	3.8	7.8	4,900	7,600	1,330	1.02	6.9	23.4	9,490	3.4	24	3,350	3,790	574	558	468	951	160	217
			{Dry—	1			7.6	.465						9,070	11,930	1,557	2.99	6.7	15.0	16,710	8.3	35	7,370	8,380	1,400	910	584	1,166	102	
865	do.	Strafford County, N. H.	{Green—	5	17.8	36	119.3	.358	.398	49	8.6	3.0	7.1	3,400	5,910	1,014	.64	5.2	14.5	7,240	2.4	20	2,540	2,830	382	485	432	800	142	206
			{Dry—	2			8.8	.380						5,640	8,280	1,115	1.66	7.4	10.2	11,300	5.1	18	3,500	5,400	918	892	515	842		
904	Hemlock, eastern (second growth) (<i>Tsuga candensis</i>)	Bennington County, Vt.	{Green—	5	10.8		114.1	.392	.431	52	9.5	3.0	7.4	3,460	6,390	1,032	0.66	8.2	16.7	8,600	3.4	23	2,350	2,970	396	476	361	838	144	204
			{Dry—	5			12.1	.409						6,010	9,600	1,150	1.79	7.9	9.9	9,270	3.6	19	3,580	5,070	752	912	562	1,234	200	344
224	Hemlock, mountain (<i>Tsuga mertensiana</i>)	Missoula County, Mont.	{Green—	5	22.6	45	70.1	.418	.480	44	10.8	4.4	7.1	3,490	6,030	.936	.78	9.4	30.8	8,770	3.6	36	2,550	2,890	399	579	464	884	200	362
			{Dry—	1			8.0	.460						7,440	11,440	1,180	2.74	9.1	12.0	15,600	7.9	36	4,460	7,510	1,419	1,289	690	1,263	166	336
939	do.	Near Girdwood, Alaska	{Green—	5	29.3		54.1	.450	.531	43	11.9	4.4	7.6	4,080	7,150	1,217	.80	9.8	25.5	9,360	3.4	28	2,530	3,410	545	611	544	936	201	303
			{Dry—	3			12.1	.495						8,500	12,580	1,522	2.70	8.4	13.0	12,820	5.0	28	5,340	7,720	1,078	1,252	852	1,285	156	304
325	Hemlock, western (<i>Tsuga heterophylla</i>)	Chehalis County, Wash.	{Green—	5	10.3	27	71.1	.376	.431	40	11.6	4.5	7.9	3,450	6,070	1,192	.58	6.0	13.5	7,800	2.4	20	2,320	2,890	350	543	432	808	168	256
			{Dry—	2			5.4	.417						8,010	10,800	1,524	2.48	6.1	16.1	13,030	6.0	26	7,730	7,910	833	1,020	621	1,172	170	
939	do.	Near Cordova, Alaska	{Green—	5	21.0		78.6	.360	.417	40	11.4	3.9	7.9	2,970	5,650	1,117	.48	7.0	17.0	7,700	2.7	21	2,460	2,780	388	449	376	769	191	334
			{Dry—	2			11.6	.396						6,030	9,340	1,396	1.56	7.4	13.6	12,160	5.4	23	4,890	5,810	636	916	527	1,178	207	357
939	do.	Near Ketchikan, Alaska	{Green—	6	21.0		65.8	.407	.472	42	12.5	4.5	7.8	3,750	6,580	1,316	.62	7.3	21.0	8,510	3.0	26	2,640	3,240	458	559	484	840	207	331
			{Dry—	2			11.2	.440						8,100	11,980	1,645	2.25	8.9	17.0	13,700	6.2	31	6,120	7,390	806	1,073	667	1,358	202	341
563	do.	Oregon	{Green—	2	11.8	40	91.5	.380	.436	45	11.8			3,760	6,180	1,273	.62	6.6		8,990	3.1	20		3,040	323	507	395	798	168	303
			{Dry—	2			11.4	.414						7,440	9,970	1,535	2.04	7.4		13,150	5.6	24		6,270	662	842	592	1,094	196	370
463	Juniper, alligator (<i>Juniperus pachyphloea</i>)	Coconino County, Ariz.	{Green—	3			39.6	.477	.545	42	7.8	2.7	3.6	3,640	6,560	.450	1.67	13.4	16.4	6,810	3.9	21	2,490	3,730	1,029	963	823	1,281		
			{Dry—	1			9.1	.518						5,740	6,760															

TABLE 21.—Strength and related properties, by localities, of woods grown in the United States—Continued

Ship- ment no.	Species (common and botanical names)	Place of growth of material tested	Moisture condition	Trees tested	Rings per inch	Sum- mer wood	Mois- ture con- tent	Specific gravity, oven dry, based on volume—		Weight per cubic foot	Shrinkage from green to oven- dry condition based on dimen- sions when green			Static bending						Impact bending			Compression parallel to grain		Com- pression perpen- dicular to grain; stress at proportional limit	Hardness; load required to em- bed a 0.444-inch ball to ½ its diameter		Shear parallel to grain; maxi- mum shearing strength	Cleav- age; load to cause splitting	Tension perpen- dicular to grain; maxi- mum tensile strength
								At test	When oven- dry		Volu- metric	Rad- ial	Tan- gen- tial	Stress at pro- portional limit	Modu- lus of rupture	Modu- lus of elas- ticity	Work			Stress at pro- portional limit	Work to propor- tional limit	Height of drop causing complete failure (50-pound hammer)	Stress at pro- portional limit	Maxi- mum crushing strength		End	Side			
																	Proportional limit	Maxi- mum load	Total											
1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31
SOFTWOODS—continued																														
314	Pine, longleaf (<i>Pinus palustris</i>)	Nassau County, Fla.	{Green—	10	23.6	43	41.6	0.574	0.667	51	12.2	5.1	6.9	5,410	8,580	1,615	1.02	7.6	29.3	11,150	3.6	33	3,860	4,340	576	542	602	1,066	177	274
343	do	Washington Parish, La.	{Dry—	4			6.9	.648						13,400	18,540	2,422	4.20	11.8	24.6	18,620	7.5	34	9,030	12,720	1,920	1,350	1,220	1,886	246	418
1059	do	St. Tammany Parish, La.	{Green—	9	14.4	38	63.8	.550	.650	56	12.4	5.5	7.8	5,650	9,400	1,752	1.03	8.9	34.1	11,490	3.8	39	3,880	4,620	707	597	664	1,150	222	324
1063	do		{Dry—	2			7.2	.627						14,120	19,360	2,488	4.47	11.8	25.6	16,310	6.5	35	10,400	13,210	1,740	1,194	1,010	1,618	212	
1065	do	Columbia County, Fla.	{Green—	5	13.0	41	70.8	.564	.661	60	12.3	5.5	7.7	5,360	9,230	1,717	.95	11.2	38.8	10,400	3.1	40	3,390	4,710	714	597	641	1,092	248	428
			{Dry—	5			11.9	.626						10,010	16,670	2,181	2.59	14.3	22.0	17,510	7.0	37	7,100	9,510	1,239	973	992	1,760	348	608
			{Green—	5	5.1	40	117.4	.483	.554	66	10.6	4.1	7.0	4,430	7,770	1,335	.84	9.6	30.0	8,100	2.7	34	2,370	3,640	547	562	562	996	254	448
			{Dry—	5			11.7	.529						7,040	12,990	1,556	1.79	12.5	18.2	13,590	5.4	35	4,770	7,350	1,148	890	812	1,554	336	609
		Charleston County, S. C.	{Green—	10	7.6	40	88.2	.501	.587	59	12.9	4.8	7.6	4,810	8,610	1,549	.85	10.4	34.8	8,600	2.4	36	2,780	4,130	531	548	574	955	238	382
226	Pine, mountain (<i>Pinus pungens</i>)	Sevier County, Tenn.	{Dry—	10			13.2	.557						7,600	14,300	1,920	1.68	13.9	21.5	14,690	5.6	37	4,850	7,420	887	770	798	1,346	334	556
			{Green—	5	15.2	29	74.7	.494	.549	54	10.9	3.4	6.8	4,530	7,520	1,271	.94	8.1	25.2	10,210	3.8	29	2,980	3,540	559	478	494	956	201	321
185	Pine, northern white (<i>Pinus strobus</i>)	Shawano County, Wis.	{Dry—	2			8.0	.532						9,420	13,380	1,653	3.06	8.9	13.6	15,930	7.6	29	4,800	8,500	1,531	832	726	1,290	201	378
			{Green—	5	16.2	31	73.9	.363	.391	39	7.8	2.2	5.9	3,410	5,310	1,073	.62	5.9	13.4	6,490	2.1	18	2,430	2,720	314	304	296	644	152	255
615	do	Near Funkley, Minn.	{Dry—	2			9.9	.385						7,040	9,620	1,417	2.04	6.4	12.6	9,280	3.3	18	5,060	6,360	757	611	469	1,072	200	339
			{Green—		13.0	28	74.7	.343	.371	37	8.7			3,210	4,830	980	.61	4.9	6.7	8,020	3.7	16		2,350	268	296	298	674	154	363
			{Dry—	5			10.0	.357						6,540	9,310	1,298	1.86	6.4	6.5	10,540	5.0	18		5,450	709	479	411	1,034	155	377
865	Pine, northern white (virgin growth) (<i>Pinus strobus</i>)	Strafford County, N. H.	{Green—	5	13.4	28	52.5	.340	.368	32	8.7	2.3	5.8	3,070	5,160	1,064	.52	5.3	11.0	6,810	2.1	16	1,870	2,470	296	303	309	668	141	238
865	Pine, northern white (second growth) (<i>Pinus strobus</i>)	do	{Dry—	2			9.5	.367						6,440	9,760	1,340	1.82	7.0	10.3	10,690	3.9	20	3,650	5,280	696	573	416	834	142	268
			{Green—	5	10.5		75.1	.329	.362	36	7.8	2.4	6.3	2,730	4,760	934	.45	4.7	10.5	5,890	1.5	16		2,380	257	318	322	653	132	158
185	Pine, Norway (<i>Pinus resinosa</i>)	Shawano County, Wis.	{Dry—	2			9.4	.348						6,060	9,300	1,223	1.65	7.9	10.8	9,660	4.1	18		4,860	568	530	390	789	161	330
			{Green—	5	22.1	41	54.0	.440	.507	42	11.5	4.6	7.2	3,740	6,430	1,384	.59	5.8	28.4	7,480	2.2	28	2,410	3,080	358	355	342	776	158	192
226	Pine, pitch (<i>Pinus rigida</i>)	Sevier County, Tenn.	{Dry—	1			12.5	.478						9,170	12,300	1,787	2.68	9.9	17.1	15,090	6.3	25	5,160	7,060	893	696	597	1,262	206	468
			{Green—	5	11.7	30	85.3	.470	.542	54	11.7	4.8	7.4	3,660	6,680	1,118	.75	8.5	30.5	9,120	3.4	29	2,370	3,040	510	458	484	950	220	354
904	Pine, pitch (second growth) (<i>Pinus rigida</i>)	Franklin County, Mass.	{Dry—	2			7.6	.517						7,840	12,430	1,498	2.56	8.7	14.0	17,150	10.7	28	4,310	7,600	1,170	820	690	1,566	288	576
			{Green—	5	11.7	27	72.3	.431	.504	46	10.1	3.3	6.7	3,540	6,980	1,281	.60	9.8	25.3	8,820	3.1	26	1,530	2,860	390	378	459	772	154	202
314	Pine, pond (<i>Pinus rigida serotina</i>)	Naussau County, Fla.	{Dry—	4			10.6	.493						8,040	11,860	1,500	1.56	9.6	12.4	11,200	4.1	35	4,770	6,460	1,255	742	634	1,438	270	488
			{Green—	5	12.8	35	55.7	.501	.580	49	11.2	5.1	7.1	4,540	7,450	1,281	.93	7.5	26.8	9,350	3.2	33	2,940	3,660	544	455	510	936	186	280
			{Dry—	2			6.2	.556						11,300	14,600	2,050	3.43	9.3	12.3	15,700	6.3	26	9,110	10,690	1,578	1,020	889	1,716	275	413
28	Pine, ponderosa (<i>Pinus ponderosa</i>)	Douglas County, Colo.	{Green—	5	31.9	40	92.7	.391	.435	47	9.9	3.8	5.8	3,310	5,460	1,053	.59	6.0	11.5	6,910	2.2	20	2,240	2,600	410	315	331	706	153	
			{Dry—	2			15.8	.411						6,230	9,460	1,263	1.77	7.0	8.5	8,020	2.7	18	4,210	4,920	714	618	454	1,030	200	
31	do	Stevens County, Wash.	{Green—		15.9		37.6	.415		36				3,390	5,660	1,160								2,770	299					
			{Dry—	5			11.2	.435						6,660	10,870	1,534								6,460	701					
140	do	Coconino County, Ariz.	{Green—	5	21.4	26	98.5	.353	.395	44	9.2	4.1	6.4	2,660	4,760	879	.47	4.9	12.8	6,160	2.1	17	1,870	2,220	342	310	314	662	166	301
			{Dry—	1																										

TABLE 21.—Strength and related properties, by localities, of woods grown in the United States—Continued

Ship- ment no.	Species (common and botanical names)	Place of growth of material tested	Moisture condition	Trees tested	Rings per inch	Sum- mer wood	Mois- ture con- tent	Specific gravity, oven dry, based on volume—		Weight per cubic foot	Shrinkage from green to oven- dry condition based on dimen- sions when green			Static bending						Impact bending			Compression parallel to grain		Com- pression perpen- dicular to grain; stress at proportional limit	Hardness; load required to em- bed a 0.444-inch ball to ½ its diameter		Shear parallel to grain; maxi- mum shearing strength	Cleav- age; load to cause splitting	Tension perpen- dicular to grain; maxi- mum tensile strength	
								At test	When oven- dry		Volu- metric	Ra- dial	Tan- gen- tial	Stress at pro- portional limit	Modu- lus of rupture	Modu- lus of elas- ticity	Work			Stress at proportional limit	Work to proportional limit	Height of drop causing complete failure (50-pound hammer)	Stress at proportional limit	Maxi- mum crushing strength		End	Side				
																	Proportional limit	Maxi- mum load	Total												
1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31	
SOFTWOODS—continued																															
463	Piñon (<i>Pinus edulis</i>)	Coconino County, Ariz.	(Green—	3	17.3																										
550, 1265	Redwood (<i>Sequoia sempervirens</i>)	Mendocino County, Calif.	(Dry—	2			8.9	.537						2,610	4,820	649	0.61	7.6	23.0	8,190	4.2	21	1,810	2,590	475	512	596	918			
1267			(Green—	9	33.0	28	104.1	.398	.422	51	6.7	2.7	4.1	5,030	7,640	1,178	1.33	7.5	16.3	9,310	3.4	22	3,780	4,290	524	569	422	798	157	248	
1267	do	Humboldt County, Calif.	(Dry—	7	25.2		119.8	.361	.411	50	6.8	2.6	4.6	4,620	7,350	1,175	1.04	7.4	14.0	8,520	3.0	20	3,630	4,110	523	569	405	809	179	264	
1265	Redwood (second growth openly grown) (<i>Sequoia sempervirens</i>)	Mendocino County, Calif.	(Green—	2	3.4		90.3	.300	.328	36	6.9	2.3	5.0	3,340	5,530	801	.80	6.1	7.2	6,590	2.6	14	2,160	2,740	444	448	304	716	179	288	
1267			(Dry—	4	2.8		173.0	.272	.301	46	6.0				2,460	4,130	563	.62	4.7	5.8	5,500	2.2	14	1,640	2,100	248	357	274	604	144	254
1265	Redwood (second growth closely grown) (<i>Sequoia sempervirens</i>)	Mendocino County, Calif.	(Green—	3	7.3		100.0	.347	.396	43	8.5	2.6	5.4	4,030	6,930	1,208	.78	6.7	10.7	8,060	2.8	19	3,360	3,780	376	522	384	764	178	271	
1267			(Dry—	5	6.2		119.0	.300	.340	41	6.7	2.2	4.7	3,270	5,540	879	.70	5.8	11.0	6,740	2.3	17	2,530	2,980	331	436	330	704	178	302	
865	Spruce, black (<i>Picea mariana</i>)	Rockingham County, N. H.	(Green—	5	14.9		37.5	.376	.428	32	11.3	4.1	6.8	2,900	5,360	1,065	.45	7.4	20.4	6,800	1.8	24	1,540	2,570	175	430	370	662	117	104	
26	Spruce, Engelmann (<i>Picea engelmannii</i>)	Grand County, Colo.	(Dry—	5			9.8	.406						5,740	10,290	1,523	1.33	10.5	21.4	13,400	6.2	23	5,510	6,070	1,086	762	556	1,096	170		
29			(Green—	2	17.1	33	45.0	.325	.359	29	10.5	3.7	6.9	2,740	4,550	866	.50	4.8	6.0	6,300	2.1	13	1,820	2,170	302	272	264	616	122		
1	Spruce, red (<i>Picea rubra</i>)	San Miguel County, Colo.	(Dry—	2			12.8	.342						5,100	7,740	1,074	1.37	5.4	8.2	8,890	3.5	16	3,550	4,560	589	484	334	1,024	166		
226			(Green—	5	11.3	37	155.5	.299	.335	48	10.3	3.0	6.2	2,180	3,850	798	.36	5.0	6.5	5,350	1.8	15	1,530	1,800	279	231	221	569	136		
325	Spruce, Sitka (<i>Picea sitchensis</i>)	Chehalis County, Wash.	(Dry—	2			16.8	.314						3,820	5,860	990	.81	5.4	6.5	7,710	2.8	14	2,480	3,060	447	298	244	802	191		
504			(Green—	6	21.4	27	34.9	.389		33					3,550	5,960	1,166	.63	7.5		7,100	2.3	17		2,700	318	418	368	760	129	212
563	do	Sevier County, Tenn.	(Dry—	5			12.8	.412						6,760	10,260	1,564	1.64	8.7	16.1	11,330	4.4	23	5,120	5,700	523	638	502	1,214	173	348	
654			(Green—	2	13.3	29	52.6	.367	.413	35	11.8	3.8	7.8	3,310	5,600	1,215	.52	6.2	14.6	7,220	2.3	19	2,340	2,600	368	446	346	764	146	223	
939	do	Clatsop County, Oreg.	(Dry—	5	9.0	24	53.0	.342	.373	33	11.2	4.5	7.4	3,020	5,490	1,185	.44	6.4	21.8	7,940	2.5	29	2,270	2,600	326	433	370	777	148	216	
939			(Green—	1	15.3		44.6	.340	.379	31	10.7	3.8	7.0	3,160	4,920	1,092	.53	5.4	15.3	7,810	2.7	20	1,920	2,180	222	350	280	696	108	172	
939	do	Oregon	(Dry—	4			12.6	.379						6,180	8,380	1,366	1.48	7.4		9,860	3.6	21		4,570	553	693	442	1,206	220	428	
300			(Green—	3	13.6	47	36.1	.384	.444	33	12.8				3,680	6,020	1,455	.58	6.2	11.1	9,640	3.5	24		2,840	355	478	350	778	118	162
185, 165	do	do	(Dry—	3			10.6	.419						7,670	11,330	1,738	1.92	11.3		13,850	5.7	28		6,260	796	846	536	1,348	213	415	
263			(Green—	3	9.6	41	34.0	.368	.412	31	11.2	4.4	7.9	3,640	5,880	1,311	.59	6.5		9,270	3.9	23		2,930	353	478	348	748	164	296	
939	do	Near Girdwood, Alaska	(Dry—	3			6.9	.389						6,740	9,980	1,604	1.61	8.4	13.2	12,800	5.0	24		7,470	899	941	499	1,296	210	358	
939			(Green—	5	23.3		39.2	.394	.456	34	11.4	4.4	7.6	3,350	5,830	1,138	.56	6.7	18.1	8,350	3.1	23	2,190	2,710	420	395	334	758	150	326	
1	Spruce, white (<i>Picea glauca</i>)	Near Ketchikan, Alaska	(Dry—	2			11.3	.426						7,040	11,780	1,662	1.69	10.3	16.1	11,100	3.8	21	5,170	6,620	714	664	540	1,160	252	346	
300			(Green—	5	16.8		39.1	.384	.431	33	11.6	4.0	7.7	3,340	5,880	1,295	.51	6.3	20.8	8,270	2.9	23	2,330	2,810	375	465	396	778	190	298	
185, 165	do	Coos County, N. H.	(Dry—	2			10.0	.422						7,700	11,540	1,632	2.08	10.1	19.9	10,250	3.3	30	5,310	6,580	792	1,050	617	1,133	205	439	
263			(Green—		11.2	22	52.4	.354		34					3,290	5,670	1,060	.61	6.7		9,670	3.5	20	4,070	4,890	455	514	424	1,131	190	350
939	do	Near Matanuska, Alaska	(Dry—	5			50.2	.388	.461	36	12.6	5.8	9.1	3,170	5,660	1,149	.51	5.8	17.4	7,580	2.7	24	2,230	2,720	330	371	352	710	170	231	
300			(Green—	2			11.8	.435							6,720	10,640	1,402	1.86	8.0	15.4	11,070	4.2	22	3,320	6,310	752	649	504	1,239	228	390
185, 165	Tamarack (<i>Larix laricina</i>)	Marathon and Shawano Counties, Wis.	(Dry—	1			48.4	.377	.431	35	14.8	3.7	7.3	3,370	5,410	988	.69	5.4	14.2	6,750	2.0	20	2,140	2,550	267	290	278	691	134	198	
263			(Green—	5	17.1	29	48.4	.377	.431	35	14.8	3.7	7.3	3,370	5,410	988	.69	5.4	14.2	6,750	2.0	20	2,140	2,550	267	290	278	691	134	198	
185, 165	Yew, Pacific (<i>Taxus brevifolia</i>)	Snohomish County, Wash.	(Dry—	1			6.5	.432						4,200	7,170	1,236	.84	7.2	28.8	7,750	2.7	28	4,530	7,020	730	984	690	806			
263			(Green—	5	19.9	38	52.0	.491	.558	47	13.6	3.7	7.4	4,400	7,170	1,236	.84	7.2	28.8	7,750	2.7	28	2,930	3,480	480	401	375	863	163	255	
185, 165	do	do	(Dry—	2			11.0	.531						8,400	12,050	1,680	2.35	7.1	14.4	12,950	5.7	23	4,980	7,590	1,080	725	636	1,372	228	414	
263			(Green—	5	26.8		44.1	.601	.673	54	9.7	4.0	5.4	6,520	10,140	989	2.46	20.2	54.3	13,110	6.2	38	3,440								

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